

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-03-2015		Table No. A1				
					Location: Village Road		Chainage (Km): 0.650		Reduced Level: 100.0m assumed		WATER TABLE (m) : 8.00		15.00		Date of Completion: 04-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	99.500	DS1		Light Gray Sandy Silt (ML) (1.50m)	0	33	64	3										
15	1.50	98.500	SPT1		Light Gray Sandy Silt (ML-CL)														
	2.50	97.500	UDS1		(3.00m)	0	29	65	6	24	18	6	2.66	1.69	1.55	9.3	UUT	0.50	10
12	3.00	97.000	SPT2		Light Gray Fine Sand (SP-SM)														
16	4.50	95.500	SPT3																
	5.50	94.500	UDS2			0	93	7	0	N	P	-	2.62	1.73	1.58	9.5	DST	0.00	31
18	6.00	94.000	SPT4																
17	7.50	92.500	SPT5																
	8.50	91.500	UDS3																
20	9.00	91.000	SPT6			0	89	11	0	N	P	-							
39	10.50	89.500	SPT7												1.65*		DST	0	33
	11.50	88.500	UDS4																
44	12.00	88.000	SPT8			0	93	7	0	N	P	-	2.60						
41	13.50	86.500	SPT9																
	14.50	85.500	UDS5																
34	15.00	85.000	SPT10		(15.00m)	0	92	8	0	N	P	-							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 27-02-2015		Table No. A1a					
					Location: MAJOR BRIDGE		Chainage (Km): 1+172		Reduced Level: 204.640		WATER TABLE (m) : 7.00		30.00		Date of Completion: 28-02-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
	0.50	204.140	DS1		Light Gray Sandy Silt (ML-CL)	0	33	61	6											
6	1.50	203.140	SPT1																	
	2.50	202.140	UDS1			0	31	63	6	24	18	6	2.66	1.69	1.52	11.2	UUT	0.45	12	
11	3.00	201.640	SPT2																	
7	4.50	200.140	SPT3																	
	5.50	199.140	UDS2			0	33	59	8	26	20	6	2.67	1.74	1.52	14.2	UUT	0.50	10	
5	6.00	198.640	SPT4																	
14	7.50	197.140	SPT5		Light Gray Silty Fine Sand (SM)															
	8.50	196.140	UDS3																	
21	9.00	195.640	SPT6		0	86	14	0	N	P	-									
22	10.50	194.140	SPT7		Light Gray Fine Sand (SP-SM)										1.63*		DST	0	32	
	11.50	193.140	UDS4																	
28	12.00	192.640	SPT8			0	92	8	0	N	P	-								
31	13.50	191.140	SPT9																	
	14.50	190.140	UDS5																	
37	15.00	189.640	SPT10		0	93	7	0	N	P	-									
44	16.50	188.140	SPT11		Light Gray Clayey Silt (CL)					33	21	12								
	17.50	187.14	UDS6			5	12	70	13				2.68	1.99	1.68	18.7				
46	18.00	186.64	SPT12																	

Contd. On Table No. A1b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)	Date of Start: 27-02-2015	Table No. A1b							
					Location: MAJOR BRIDGE	Chainage (Km): 1+172	Reduced Level: 204.640		WATER TABLE (m) : 7.00		30.00	Date of Completion: 28-02-2015	Job No. 201414							
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
35	19.50	185.140	SPT13	[Symbol]	Light Gray Clayey Silt (CL)															
	20.50	184.140	UDS7		(21.00m)	4	11	73	12	33	22	11		1.99	1.7	18.5	UUT	1.40	10	
61	21.00	183.640	SPT14	[Symbol]	Light Gray Silty Fine Sand (SM)															
62	22.50	182.140	SPT15		0	56	34	0	N	P	-									
	23.50	181.140	UDS8																	
63	24.00	180.640	SPT16																	
64	25.50	179.140	SPT17		0	80	20	0	N	P	-									
	26.50	178.140	UDS9																	
63	27.00	177.640	SPT18										2.64		1.70*		DST	0.00	34	
68	28.50	176.140	SPT19																	
	29.50	175.140	UDS10																	
61	30.00	174.640	SPT20			(30.00m)	0	84	16	0	N	P	-							

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 24-02-2015		Table No. A2a						
					Location: MAJOR BRIDGE		Chainage (Km): 1+172		Reduced Level: 204.640		WATER TABLE (m) : 9.50		30.00		Date of Completion: 25-02-2015		Job No. 201414				
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test				
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction		
13	0.50	204.140	DS1		Light Gray Sandy Silt (ML-CL)	0	33	61	6												
	1.50	203.140	SPT1																		
	2.50	202.140	UDS1				0	31	63	6	24	18	6	2.66	1.69	1.52	11.2	UUT	0.45	12	
18	3.00	201.640	SPT2		Light Gray Fine Sand (SP-SM) (4.50m)																
16	4.50	200.140	SPT3																		
	5.50	199.140	UDS2					0	33	59	8	26	20	6	2.67	1.74	1.52	14.2	UUT	0.50	10
8	6.00	198.640	SPT4																		
18	7.50	197.140	SPT5																		
	8.50	196.140	UDS3																		
21	9.00	195.640	SPT6			0	86	14	0	N	P	-									
19	10.50	194.140	SPT7													1.63*		DST	0	32	
	11.50	193.140	UDS4																		
19	12.00	192.640	SPT8			0	92	8	0	N	P	-									
30	13.50	191.140	SPT9																		
	14.50	190.140	UDS5																		
35	15.00	189.640	SPT10			0	93	7	0	N	P	-									
36	16.50	188.140	SPT11																		
	17.50	187.14	UDS6					5	12	70	13				2.68	1.99	1.68	18.7			
36	18.00	186.64	SPT12		Light Gray Clayey Silt (CL) (18.00m)																

Contd. On Table No. A2b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)	Date of Start: 24-02-2015	Table No. A2b							
Location: MAJOR BRIDGE		Chainage (Km): 1+172		Reduced Level: 204.640		WATER TABLE (m) : 9.50			30.00	Date of Completion: 25-02-2015		Job No. 201414								
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
34	19.50	185.140	SPT13		Light Gray Clayey Silt (CL)															
	20.50	184.140	UDS7		4	11	73	12	33	22	11		1.99	1.7	18.5	UUT	1.40	10		
41	21.00	183.640	SPT14																	
35	22.50	182.140	SPT15		0	56	34	0	N	P	-									
	23.50	181.140	UDS8		(24.00m)															
46	24.00	180.640	SPT16		Light Gray Fine Sand (SP-SM)															
53	25.50	179.140	SPT17		0	80	20	0	N	P	-									
	26.50	178.140	UDS9																	
54	27.00	177.640	SPT18		2.64															
60	28.50	176.140	SPT19																	
	29.50	175.140	UDS10																	
54	30.00	174.640	SPT20	(30.00m)	0	84	16	0	N	P	-									

SOIL PROFILE		Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1			Termination Depth (m)		Date of Start: 13-02-2015		Table No. B1a						
		Location: MAJOR BRIDGE		Chainage (Km): 2+109		Reduced Level: 207.753		WATER TABLE (m) : 13.00			30.00		Date of Completion: 15-02-2015		Job No. 201414				
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	207.253	DS1		Filled Up Soils														
29	1.50	206.253	SPT1		Light Brown Sandy Silt (ML-CL)					26	20	6							
	2.50	205.253	UDS1			0	17	77	6				2.66	1.70	1.55	9.6	UUT	0.40	10
15	3.00	204.753	SPT2																
20	4.50	203.253	SPT3																
	5.50	202.253	UDS2			0	29	66	5	24	18	6	1.75	1.59	10.20				
24	6.00	201.753	SPT4		Light Gray Silty Fine Sand (SM)														
23	7.50	200.253	SPT5																
	8.50	199.253	UDS3			0	82	18	0	N	P	-	2.63	1.79	1.62	10.6	DST	0.00	32
31	9.00	198.753	SPT6																
35	10.50	197.253	SPT7																
	11.50	196.253	UDS4																
37	12.00	195.753	SPT8																
41	13.50	194.253	SPT9			6	55	39	0	N	P	-							
	14.50	193.253	UDS5																
43	15.00	192.753	SPT10																
48	16.50	191.253	SPT11			0	70	30	0	N	P	-							
	17.50	190.25	UDS6																
51	18.00	189.75	SPT12		Light Gray Fine Sand (SP-SM)	0	90	10	0	N	P	-							

Contd. On Table No. B1b

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)	Date of Start: 13-02-2015	Table No. B1b							
					Location: MAJOR BRIDGE	Chainage (Km): 2+109	Reduced Level: 207.753	WATER TABLE (m) : 13.00		30.00	Date of Completion: 15-02-2015	Job No. 201414								
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
52	19.50	188.253	SPT13		Light Gray Fine Sand (SP-SM)								2.62		1.66*		DST	0.0	34	
	20.50	187.253	UDS7																	
52	21.00	186.753	SPT14																	
61	22.50	185.253	SPT15			0	91	9	0	N	P	-								
	23.50	184.253	UDS8																	
					(24.00m)															
26	24.00	183.753	SPT16		Light Brown Sandy Silt (ML-CL)					25	19	6								
31	25.50	182.253	SPT17																	
	26.50	181.253	UDS9			0	37	57	6				2.65	1.99	1.68	18.2	UUT	1.30	11	
37	27.00	180.753	SPT18																	
51	28.50	179.253	SPT19																	
	29.50	178.253	UDS10		0	32	68	6	24	19	5									
					(30.00m)															
53	30.00	177.753	SPT20		Light Brown Silty Fine Sand (SM)	0	85	15	0	N	P	-								

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 17-02-2015		Table No. B2a				
					Location: MAJOR BRIDGE		Chainage (Km): 2+109		Reduced Level: 207.753		WATER TABLE (m) : 12.50		30.00		Date of Completion: 18-02-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	207.253	DS1		Light Brown Sandy Silt (ML)	0	39	58	3										
22	1.50	206.253	SPT1																
	2.50	205.253	UDS1			0	37	60	3	N	P	-	2.65	1.69	1.56	8.6	DST	0.00	29
20	3.00	204.753	SPT2		(4.50m)														
26	4.50	203.253	SPT3		Light Gray Silty Fine Sand (SM)														
	5.50	202.253	UDS2			0	69	31	3	N	P	-	1.73	1.58	9.7	DST	0.00	30	
21	6.00	201.753	SPT4		(7.50m)														
25	7.50	200.253	SPT5		Light Gray Fine Sand (SP-SM)														
	8.50	199.253	UDS3			0	88	12	0	N	P	-	2.63	1.79	1.62	10.3	DST	0.00	32
31	9.00	198.753	SPT6																
30	10.50	197.253	SPT7		(11.50m)														
	11.50	196.253	UDS4		Light Gray Silty Fine Sand (SM)														
22	12.00	195.753	SPT8			0	55	45	0	N	P	-							
25	13.50	194.253	SPT9										2.64		1.65*		DST	0	32
	14.50	193.253	UDS5																
29	15.00	192.753	SPT10																
31	16.50	191.253	SPT11			0	67	33	0	N	P	-							
	17.50	190.25	UDS6		(18.00m)														
28	18.00	189.75	SPT12		Light Gray Fine Sand (SP-SM)														

Contd. On Table No. B2b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)	Date of Start: 17-02-2015	Table No. B2b						
Location: MAJOR BRIDGE		Chainage (Km): 2+109		Reduced Level: 207.753		WATER TABLE (m) : 12.50		30.00	Date of Completion: 18-02-2015	Job No. 201414									
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
29	19.50	188.253	SPT13		Light Gray Fine Sand (SP-SM)	0	92	8	0	N	P	-							
	20.50	187.253	UDS7		Light Brown Sandy Silt (ML-CL) (21.00m)														
32	21.00	186.753	SPT14																
23	22.50	185.253	SPT15		Light Brown Sandy Silt (ML-CL) (28.50m)					26	20	6	2.66	1.98	1.67	18.5	UUT	1.30	12
	23.50	184.253	UDS8			6	17	71	6										
21	24.00	183.753	SPT16																
32	25.50	182.253	SPT17																
	26.50	181.253	UDS9		0	40	54	6	24	19	5	1.99	1.68	18.2	DST	0.00	34		
39	27.00	180.753	SPT18																
	28.50	179.253	SPT19		Light gray Fine Sand (SP-SM)								1.68*						
	29.50	178.253	UDS10		Light gray Fine Sand (SP-SM) (30.00m)														
58	30.00	177.753	SPT20			0	93	7	0	N	P	-							

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 18-02-2015		Table No. C1a					
		Chainage (Km): 2+306		Reduced Level: 206.666		WATER TABLE (m) : 11.50				30.00		Date of Completion: 19-02-2015		Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
	0.50	206.166	DS1		Light Brown Sandy Silt (ML-CL)	0	38	57	5											
16	1.50	205.166	SPT1							26	20	6								
	2.50	204.166	UDS1				1	12	81	6			2.66	1.71	1.55	10.5	UUT	0.40	10	
27	3.00	203.666	SPT2																	
23	4.50	202.166	SPT3																	
	5.50	201.166	UDS2		(5.50m)	Light Brown Sandy Silt (ML)	0	18	79	3	22	18	4	2.64	1.74	1.58	10.2	DST	0.00	30
17	6.00	200.666	SPT4																	
	7.50	199.166	SPT5		(7.50m)	Light Gray Fine Sand (SP-SM)	0	88	12	0	N	P	-							
	8.50	198.166	UDS3																	
31	9.00	197.666	SPT6																	
35	10.50	196.166	SPT7				0	89	11	0	N	P	-							
	11.50	195.166	UDS4																	
38	12.00	194.666	SPT8									2.62		1.65*		DST	0.00	33		
35	13.50	193.166	SPT9																	
	14.50	192.166	UDS5																	
37	15.00	191.666	SPT10																	
32	16.50	190.166	SPT11			0	90	10	0	N	P	-								
	17.50	189.17	UDS6																	
31	18.00	188.67	SPT12																	

Contd. On Table No. C1b

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 18-02-2015		Table No. C1b				
		Chainage (Km): 2+306		Reduced Level: 206.666		WATER TABLE (m) : 11.50				30.00		Date of Completion: 19-02-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
35	19.50	187.166	SPT13	[Symbol]	Light Gray Fine Sand (SP-SM)														
	20.50	186.166	UDS7																
33	21.00	185.666	SPT14	[Symbol]	(22.50m)	0	89	11	0	N	P	-							
28	22.50	184.166	SPT15		Light Gray Clayey Silt (CL)					34	23	11							
	23.50	183.166	UDS8	[Symbol]		0	9	77	14				2.68	2.00	1.67	19.5			
33	24.00	182.666	SPT16																
29	25.50	181.166	SPT17	[Symbol]															
	26.50	180.166	UDS9		(27.00m)	0	8	79	13	35.0	23.0	12.0		2.01	1.69	19.2	UUT	1.40	9
45	27.00	179.666	SPT18	[Symbol]	Light Brown Silty Fine Sand (SM)														
51	28.50	178.166	SPT19			0	85	15	0	N	P	-							
	29.50	177.166	UDS10	[Symbol]															
59	30.00	176.666	SPT20		(30.00m)														

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 21-02-2015		Table No. C2a					
		Chainage (Km): 2+306			Reduced Level: 206.666			WATER TABLE (m) : 11.00		30.00		Date of Completion: 22-02-2015		Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
20	0.50	206.166	DS1		Light Brown Sandy Silt (ML-CL)															
	1.50	205.166	SPT1																	
	2.50	204.166	UDS1			0	23	71	6	26	20	6	2.66	1.71	1.56	9.6	UUT	0.40	10	
14	3.00	203.666	SPT2																	
17	4.50	202.166	SPT3																	
	5.50	201.166	UDS2			(5.50m)	0	21	76	3	23	19	4	1.75	1.58	10.5				
16	6.00	200.666	SPT4																	
20	7.50	199.166	SPT5																	
	8.50	198.166	UDS3			(10.50m)	0	38	59	3	N	P	-	2.64	1.80	1.62	11.2	DST	0.00	31
24	9.00	197.666	SPT6																	
33	10.50	196.166	SPT7			(10.50m)	0	76	24	0	N	P	-							
	11.50	195.166	UDS4																	
37	12.00	194.666	SPT8																	
41	13.50	193.166	SPT9																	
	14.50	192.166	UDS5																	
43	15.00	191.666	SPT10			0	82	18	0	N	P	-								
48	16.50	190.166	SPT11										2.63		1.66*		DST	0.00	33	
	17.50	189.17	UDS6																	
56	18.00	188.67	SPT12																	

Contd. On Table No. C2b

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 21-02-2015		Table No. C2b				
		Chainage (Km): 2+306		Reduced Level: 206.666		WATER TABLE (m) : 11.00		30.00		Date of Completion: 22-02-2015		Job No. 201414							
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
63	19.50	187.166	SPT13	[Symbol]	Light Gray Fine Sand (SP-SM)														
	20.50	186.166	UDS7																
59	21.00	185.666	SPT14	[Symbol]	(22.50m)	0	87	13	0	N	P	-							
40	22.50	184.166	SPT15		Light Gray Sandy Silt (ML-CL)														
	23.50	183.166	UDS8			6	14	74	6	27	20	7	1.98	1.67	18.3	UUT	1.35	10	
38	24.00	182.666	SPT16	[Symbol]															
42	25.50	181.166	SPT17																
	26.50	180.166	UDS9			5	18	71	6	26	20	6	2.66	1.98	17.9				
59	27.00	179.666	SPT18	[Symbol]	(27.00m)														
	28.50	178.166	SPT19		Light Brown Silty Fine Sand (SM)										1.70*		DST	0.00	34
	29.50	177.166	UDS10																
67	30.00	176.666	SPT20		(30.00m)	0	85	15	0	N	P	-							

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 3		Termination Depth (m)		Date of Start: 09-03-2015		Table No. C3a				
		Chainage (Km): 2+306			Reduced Level: 206.666			WATER TABLE (m) : 8.00		30.00		Date of Completion: 10-03-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	206.166	DS1		Light Gray Sandy Silt (ML-CL)	6	19	69	6										
7	1.50	205.166	SPT1		Light Brown Clayey Silt (CL)														
	2.50	204.166	UDS1			6	4	77	13	34	22	12	2.67	1.73	1.53	13.2	UUT	0.50	9
11	3.00	203.666	SPT2																
16	4.50	202.166	SPT3			4	9	75	12	33	22	11		1.81	1.58	14.7			
	5.50	201.166	UDS2																
20	6.00	200.666	SPT4		Light Gray Silty Fine Sand (SM)														
33	7.50	199.166	SPT5			0	61	39	0	N	P	-							
	8.50	198.166	UDS3		Light Gray Fine Sand (SP-SM)														
35	9.00	197.666	SPT6			0	88	12	0	N	P	-							
41	10.50	196.166	SPT7										2.63		1.66*		DST	0	33
	11.50	195.166	UDS4																
47	12.00	194.666	SPT8			0	92	8	0	N	P	-							
55	13.50	193.166	SPT9																
	14.50	192.166	UDS5																
45	15.00	191.666	SPT10										2.62		1.67*		DST	0	34
49	16.50	190.166	SPT11			0	93	7	0	N	P	-							
	17.50	189.17	UDS6																
56	18.00	188.67	SPT12																














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BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Merut Section of DFCC Meerut				BH.No. 3		Termination Depth (m)	Date of Start: 09-03-2015	Table No. C3b						
		Chainage (Km): 2+306		Reduced Level: 206.666		WATER TABLE (m) : 8.00		30.00	Date of Completion: 10-03-2015	Job No. 201414									
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
35	19.50	187.166	SPT13		Light Brown Sandy Silt (ML-CL)														
	20.50	186.166	UDS7		0	32	62	6	24	18	6	2.66	1.97	1.67	17.9	UUT	1.35	10	
25	21.00	185.666	SPT14		(23.50m)														
41	22.50	184.166	SPT15																
	23.50	183.166	UDS8		Light Gray Fine Sand (SP-SM)														
61	24.00	182.666	SPT16		0	93	7	0	N	P	-								
66	25.50	181.166	SPT17		(30.00m)														
	26.50	180.166	UDS9																
70	27.00	179.666	SPT18		(30.00m)														
	28.50	178.166	SPT19			0	94	6	0	N	P	-			1.72*	DST	0.00	35	
71	29.50	177.166	UDS10		(30.00m)														
64	30.00	176.666	SPT20																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 12-03-2015		Table No. B1					
					Location: Minor Bridge		Chainage (Km): 2.873		Reduced Level: 209.001		WATER TABLE (m) : 10.50		12.00		Date of Completion: 12-03-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
	0.50	208.501	DS1		Light Brown Sandy Silt (ML-CL)	0	27	67	6											
5	1.50	207.501	SPT1		(6.00m)															
	2.50	206.501	UDS1			0	30	64	6	24	18	6	2.66	1.74	1.53	13.5	UUT	0.45	10	
7	3.00	206.001	SPT2																	
13	4.50	204.501	SPT3																	
	5.50	203.501	UDS2			0	26	69	5	25	19	6		1.79	1.57	14.2				
17	6.00	203.001	SPT4			Light Gray Silty Fine Sand (SM)														
17	7.50	201.501	SPT5		(12.00m)	0	77	23	0	N	P	-								
	8.50	200.501	UDS3																	
19	9.00	200.001	SPT6																	
23	10.50	198.501	SPT7																	
	11.50	197.501	UDS4			0	85	15	0	N	P	-	2.63	1.90	1.62	17.3	DST	0.00	31	
26	12.00	197.001	SPT8																	

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 13-03-2015		Table No. C1					
					Location: Minor Bridge		Chainage (Km): 3.490		Reduced Level (m): 209.334		WATER TABLE (m) : 11.00		12.00		Date of Completion: 13-03-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
14	0.50	208.834	DS1		Light Brown Sandy Silt (ML-CL)	0	22	73	5											
	1.50	207.834	SPT1																	
	2.50	206.834	UDS1				0	22	72	6	25	19	6	2.66	1.71	1.55	10.6	UUT	0.50	9
15	3.00	206.334	SPT2																	
	4.50	204.834	SPT3		(4.50m) Light Gray Clayey Silt (CL)															
21	5.50	203.834	UDS2			0	10	77	13	31	22	9	2.68	1.81	1.60	13.2	UUT	0.70	10	
	6.00	203.334	SPT4																	
19	7.50	201.834	SPT5		(7.50m) Light Gray Fine Sand (SP-SM)															
	8.50	200.834	UDS3			0	90	10	0	N	P	-		1.85	1.63	13.5	DST	0.00	32	
30	9.00	200.334	SPT6																	
34	10.50	198.834	SPT7																	
	11.50	197.834	UDS4																	
43	12.00	197.334	SPT8		(12.00m)	0	89	11	0	N	P	-	2.63							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 13-03-2015		Table No. D1				
					Location: Minor Bridge		Chainage (Km): 4.252		Reduced Level (m): 209.998		WATER TABLE (m) : 10.20		12.00		Date of Completion: 14-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	209.498	DS1		Light Brown Silty Fine Sand (SM) (1.50m)	0	52	48	0										
5	1.50	208.498	SPT1		Light Gray Sandy Silt (ML)														
	2.50	207.498	UDS1			0	26	71	3	21	17	4	2.65	1.70	1.52	12.1	DST	0.00	30
6	3.00	206.998	SPT2																
10	4.50	205.498	SPT3		Light Gray Sandy Silt (ML-CL) (4.50m)														
	5.50	204.498	UDS2			0	20	74	6	26	20	6		1.77	1.56	13.5			
22	6.00	203.998	SPT4		Light Gray Silty Fine Sand (SM) (6.00m)														
22	7.50	202.498	SPT5																
	8.50	201.498	UDS3			0	79	21	0	N	P	-	2.64	1.81	1.60	13.2	DST	0.00	31
26	9.00	200.998	SPT6																
31	10.50	199.498	SPT7																
	11.50	198.498	UDS4																
45	12.00	197.998	SPT8			0	80	20	0	N	P	-							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 14-03-2015		Table No. E1				
					Location: Minor Bridge		Chainage (Km): 5+163		Reduced Level (m): 209.732		WATER TABLE (m) : NOT MET		12.00		Date of Completion: 14-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	209.232	DS1		Filled Up Soils														
36	1.50	208.232	SPT1		Light Gray Sandy Silt (ML-CL)														
	2.50	207.232	UDS1			0	24	70	6	25	19	6	2.66	1.70	1.55	9.7	UUT	0.45	12
11	3.00	206.732	SPT2																
14	4.50	205.232	SPT3		Light Gray Sandy Silt (ML)														
	5.50	204.232	UDS2			0	29	68	3	21	17	4	2.65	1.77	1.60	10.5	DST	0.00	30
19	6.00	203.732	SPT4																
	7.50	202.232	SPT5		Light Gray Silty Fine Sand (SM)														
	8.50	201.232	UDS3			0	72	28	0	N	P	-	1.79	1.62	10.3	DST	0.00	32	
26	9.00	200.732	SPT6																
	10.50	199.232	SPT7			0	77	23	0	N	P	-	2.63						
	11.50	198.232	UDS4		Light Gray Fine Sand (SP-SM)														
36	12.00	197.732	SPT8			0	92	8	0	N	P	-							

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Muzaffarnagar Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 15-03-2015		Table No. F1				
		Chainage (Km): 6+099			Reduced Level (m): 209.709			WATER TABLE (m) : NOT MET			12.00		Date of Completion: 15-03-2015		Job No. 201414				
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	209.209	DS1		Dark Gray Sandy Silt (ML - CL) (1.50m)	0	27	67	6										
7	1.50	208.209	SPT1		Light Brown Sandy Silt (CL)					33	22	11							
	2.50	207.209	UDS1			0	16	72	12				2.68	1.73	1.53	13.2	UUT	0.45	10
8	3.00	206.709	SPT2																
17	4.50	205.209	SPT3																
	5.50	204.209	UDS2			0	14	75	11	32	22	10		1.82	1.60	13.5			
23	6.00	203.709	SPT4		Light Brown Silty Fine Sand (SM) (7.50m)	0	81	19	0	N	P	-							
27	7.50	202.209	SPT5		Light Gray Fine Sand (SP-SM)														
	8.50	201.209	UDS3			0	90	10	0	N	P	-	2.63	1.81	1.62	11.5	DST	0.00	31
30	9.00	200.709	SPT6																
34	10.50	199.209	SPT7																
	11.50	198.209	UDS4			0	92	8	0	N	P	-	1.84	1.64	12.2				
43	12.00	197.709	SPT8		(12.00m)														

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 15-03-2015		Table No. G1				
					Location: Minor Bridge		Chainage (Km): 7+064		Reduced Level (m): 209.884		WATER TABLE (m) : NOT MET		12.00		Date of Completion: 16-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
31	0.50	209.384	DS1		Light Gray Sandy Silt (ML - CL)	0	23	72	5										
	1.50	208.384	SPT1																
26	2.50	207.384	UDS1		(4.50m)	0	22	72	6	26	19	7	2.66	1.70	1.55	9.5	UUT	0.40	12
	3.00	206.884	SPT2																
19	4.50	205.384	SPT3		Light Gray Silty Fine Sand (SM)														
	5.50	204.384	UDS2			0	70	30	0	N	P	-	1.76	1.60	9.8				
23	6.00	203.884	SPT4																
	7.50	202.384	SPT5																
37	8.50	201.384	UDS3			0	73	27	0	N	P	-	2.63	1.80	1.63	10.4	DST	0.00	32
	9.00	200.884	SPT6																
43	10.50	199.384	SPT7																
	11.50	198.384	UDS4			0	79	21	0	N	P	-	1.85	1.67	10.9				
49	12.00	197.884	SPT8		(12.00m)														




















BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Muzaffarnagar Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 02-04-2015		Table No.H1				
		Chainage (Km): 8+060			Reduced Level (m): 210.385			WATER TABLE (m) : Not Met			12.00		Date of Completion: 02-04-2015		Job No. 201414				
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	209.885	DS1		Light Gray Sandy Silt (ML)	0	36	61	3										
12	1.50	208.885	SPT1																
	2.50	207.885	UDS1			0	38	59	3	N	P	-	2.64	1.70	1.54	10.7	DST	0.00	28
12	3.00	207.385	SPT2		Light Gray Sandy Silt (ML-CL) (3.00m)														
16	4.50	205.885	SPT3			0	17	77	6										
	5.50	204.885	UDS2							26	19	7		1.80	1.59	13.5			
17	6.00	204.385	SPT4		Light Gray Clayey Silt (CL) (6.00m)														
21	7.50	202.885	SPT5																
	8.50	201.885	UDS3			0	9	78	13				2.68	1.85	1.62	14.4	UUT	0.70	9
23	9.00	201.385	SPT6		Light Gray Clayey Silt (CI) (9.00m)					42	23	19							
21	10.50	199.885	SPT7																
	11.50	198.885	UDS4			0	90	10	0	N	P	-		1.87	1.66	12.7	DST	0.00	33
25	12.00	198.385	SPT8		Gray Fine Sand (SP-SM) (12.00m)														

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 16-03-2015		Table No. I1				
					Location: Major Bridge		Chainage (Km): 8+977		Reduced Level (m): 212.888		WATER TABLE (m) : 10.50		12.00		Date of Completion: 16-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	212.388	DS1		Light Gray Sandy Silt (ML - CL) (1.50m)	0	24	70	6										
4	1.50	211.388	SPT1		Light Gray Clayey Silt (CL)					33	22	11							
	2.50	210.388	UDS1			0	10	77	13				2.67	1.70	1.48	14.7	UUT	0.40	9
7	3.00	209.888	SPT2																
8	4.50	208.388	SPT3																
	5.50	207.388	UDS2			0	13	73	14	34	22	12		1.75	1.52	15.2	UUT	0.65	10
13	6.00	206.888	SPT4																
16	7.50	205.388	SPT5																
	8.50	204.388	UDS3			0	13	74	13	33	22	12	2.68	1.84	1.58	16.7			
25	9.00	203.888	SPT6		Light Gray Silty Fine Sand (SM) (9.00m)	0	75	25	0	N	P	-							
	10.50	202.388	SPT7		Light Gray Sandy Silt (CL) (10.50m)														
	11.50	201.388	UDS4			0	14	76	10	30	21	9	2.67	1.97	1.65	19.2	UUT	1.20	9
20	12.00	200.888	SPT8		Light Gray Fine Sand (SP-SM) (12.00m)	0	89	11	0	N	P	-							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 17-03-2015		Table No.J1				
					Location: Minor Bridge		Chainage (Km): 10+030		Reduced Level (m): 212.290		WATER TABLE (m) : Not Met		12.00		Date of Completion: 17-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
4	0.50	211.790	DS1	[Symbol]	Light Gray Clayey Silt (CL)	0	6	81	13										
	1.50	210.790	SPT1																
8	2.50	209.790	UDS1	[Symbol]	(4.50m)	0	7	80	13	34	22	12	2.68	1.70	1.48	14.7	UUT	0.50	9
	3.00	209.290	SPT2																
13	4.50	207.790	SPT3	[Symbol]	Light Gray Sandy Silt (ML)														
	5.50	206.790	UDS2																
16	6.00	206.290	SPT4	[Symbol]	(7.50m)	0	30	67	3	N	P	-	2.64	1.79	1.58	13.2	DST	0.00	30
	7.50	204.790	SPT5																
23	8.50	203.790	UDS3	[Symbol]	(12.00m)	0	79	21	0	N	P	-	2.63	1.86	1.64	13.5	DST	0.00	32
	9.00	203.290	SPT6																
24	10.50	201.790	SPT7	[Symbol]	(12.00m)														
	11.50	200.790	UDS4																
27	12.00	200.290	SPT8	[Symbol]	Light Gray Fine Sand (SP-SM)	0	92	8	0	N	P	-							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 18-03-2015		Table No.K1					
					Location: Minor Bridge		Chainage (Km): 10+973		Reduced Level (m): 212.165		WATER TABLE (m) : 11.00		12.00		Date of Completion: 18-03-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
25	0.50	211.665	DS1		Light Gray Silty Sand (SM)	0	21	73	6											
	1.50	210.665	SPT1		Light Gray Silty Sand (SM)															
	2.50	209.665	UDS1				0	23	72	5	25	19	6	2.65	1.73	1.58	9.5	UUT	0.50	12
20	3.00	209.165	SPT2		(4.50m)	Light Gray Clayey (CL)														
24	4.50	207.665	SPT3																	
	5.50	206.665	UDS2		(7.50m)	Light Gray Silty Fine Sand (SM)	0	5	81	14	34	22	12	2.68	1.82	1.60	13.7	UUT	0.85	10
27	6.00	206.165	SPT4																	
30	7.50	204.665	SPT5		(10.50m)	Light Gray Fine Sand (SP-SM)														
	8.50	203.665	UDS3				0	78	22	0	N	P	-	1.86	1.64	13.5	DST	0.00	32	
33	9.00	203.165	SPT6	(12.00m)	Light Gray Fine Sand (SP-SM)															
37	10.50	201.665	SPT7																	
	11.50	200.665	UDS4			0	88	12	0	N	P	-	2.63							
41	12.00	200.165	SPT8																	

BHOOMI		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Muzaffarnagar Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 19-03-2015		Table No.L1					
		Chainage (Km): 11+987			Reduced Level (m): 212.114			WATER TABLE (m) : Not Met		12.00		Date of Completion: 19-03-2015		Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
11	0.50	211.614	DS1		Light Brown Sandy Silt (ML-CL)	0	19	76	5											
	1.50	210.614	SPT1																	
	2.50	209.614	UDS1																	
15	3.00	209.114	SPT2		(4.50m)															
19	4.50	207.614	SPT3		Light Gray Clayey (CL)															
	5.50	206.614	UDS2		(6.00m)															
21	6.00	206.114	SPT4		Light Gray Fine Sand (SP-SM)															
27	7.50	204.614	SPT5																	
34	8.50	203.614	UDS3																	
34	9.00	203.114	SPT6																	
34	10.50	201.614	SPT7																	
	11.50	200.614	UDS4																	
39	12.00	200.114	SPT8	(12.00m)																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 24-03-2015		Table No. D1a				
					Location: MAJOR BRIDGE		Chainage (Km): 13+841		Reduced Level: 214.107		WATER TABLE (m) : 14.80		30.00		Date of Completion: 26-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	213.607	DS1		Light Brown Sandy Silt (ML-CL)	4	18	72	6										
26	1.50	212.607	SPT1																
	2.50	211.607	UDS1			0	17	77	6	25	19	6	2.65	1.71	1.55	10.4	UUT	0.50	10
21	3.00	211.107	SPT2		Light Gray Clayey Silt (CL)														
22	4.50	209.607	SPT3							33	21	12							
	5.50	208.607	UDS2		Light Gray Clayey Silt (CI)	0	6	73	21				1.78	1.57	13.5	UUT	0.73	12	
18	6.00	208.107	SPT4							41	24	17							
25	7.50	206.607	SPT5																
	8.50	205.607	UDS3			0	5	72	23				2.69	1.86	1.62	14.7			
28	9.00	205.107	SPT6		Light Gray Sandy Silt (ML-CL)					25	18	7							
31	10.50	203.607	SPT7			0	23	72	5										
	11.50	202.607	UDS4		Light Gray Silty Fine Sand (SM)								1.88	1.66	13.5	DST	0.00	32	
37	12.00	202.107	SPT8			0	66	34	0	N	P	-							
40	13.50	200.607	SPT9																
	14.50	199.607	UDS5		Light Gray Fine Sand (SP-SM)														
45	15.00	199.107	SPT10			0	91	9	0	N	P	-							
50	16.50	197.607	SPT11										2.63		1.68*	DST	0	34	
	17.50	196.61	UDS6																
50	18.00	196.11	SPT12																

Contd. On Table No. D1b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1			Termination Depth (m)		Date of Start: 24-03-2015		Table No. D1b			
Location:		Chainage (Km):		Reduced Level:			WATER TABLE (m) :			Termination Depth (m)		Date of Completion: 26-03-2015		Job No. 201414					
MAJOR BRIDGE		13+841		214.107			14.80			30.00									
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
56	19.50	194.607	SPT13		Light Gray Fine Sand (SP-SM)	0	92	8	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
	20.50	193.607	UDS7																
59	21.00	193.107	SPT14		Light Gray Silty Fine Sand (SM) (22.50m)	5	67	28	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
31	22.50	191.607	SPT15																
	23.50	190.607	UDS8		Light Gray Fine Sand (SP-SM) (25.50m)	0	93	7	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
36	24.00	190.107	SPT16																
66	25.50	188.607	SPT17		Light Gray Fine Sand (SP-SM) (30.00m)	0	92	8	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
	26.50	187.607	UDS9																
70	27.00	187.107	SPT18		Light Gray Fine Sand (SP-SM) (30.00m)	0	92	8	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
72	28.50	185.607	SPT19																
	29.50	184.607	UDS10		Light Gray Fine Sand (SP-SM) (30.00m)	0	92	8	0	N	P	-	2.64	1.97	1.68	17.5	DST	0	35
80	30.00	184.107	SPT20																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 27-03-2015		Table No. D2				
					Location: MAJOR BRIDGE		Chainage (Km): 13+841		Reduced Level: 214.107		WATER TABLE (m) : Not Met		12.00		Date of Completion: 27-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
34	0.50	213.607	DS1		Light Gray Sandy Silt (ML-CL)	0	22	73	5										
	1.50	212.607	SPT1																
13	2.50	211.607	UDS1		(3.00m)	0	28	66	6	24	19	5	2.65	1.70	1.55	9.4	UUT	0.45	12
	3.00	211.107	SPT2			Light Gray Clayey Silt (CL)													
16	4.50	209.607	SPT3			2	6	78	14										
	5.50	208.607	UDS2								34	22	12	1.80	1.60	12.5			
17	6.00	208.107	SPT4																
	7.50	206.607	SPT5																
20	8.50	205.607	UDS3			6	8	73	13				2.68	1.83	1.62	13.2	UUT	0.80	9
	9.00	205.107	SPT6								33	22	11						
22	10.50	203.607	SPT7		(11.50m)														
	11.50	202.607	UDS4			Light Gray Silty Fine Sand (SM)	0	72	28	0	N	P	-						
27	12.00	202.107	SPT8																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 3		Termination Depth (m)		Date of Start: 27-03-2015		Table No. D3a				
					Location: MAJOR BRIDGE		Chainage (Km): 13+841		Reduced Level: 214.107		WATER TABLE (m) : 13.60		30.00		Date of Completion: 28-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	213.607	DS1		Light Gray Sandy Silt (ML-CL) (1.50m)	0	17	78	5										
20	1.50	212.607	SPT1		Light Gray Clayey Silt (CL)														
	2.50	211.607	UDS1			0	9	78	13	33	22	11	2.67	1.72	1.55	11.2	UUT	0.50	10
14	3.00	211.107	SPT2		(4.50m)														
21	4.50	209.607	SPT3		Light Gray Sandy Silt (ML-CL)					25	19	6							
	5.50	208.607	UDS2			0	19	75	6				1.80	1.60	12.3	UUT	0.65	11	
29	6.00	208.107	SPT4																
28	7.50	206.607	SPT5																
	8.50	205.607	UDS3		(9.00m)	0	22	73	5	25	18	7	2.65	1.85	1.64	12.5			
21	9.00	205.107	SPT6		Light Gray Sandy Silt (ML)														
24	10.50	203.607	SPT7																
	11.50	202.607	UDS4		(12.00m)	0	33	64	3	N	P	-	1.85	1.64	12.7	DST	0.00	31	
28	12.00	202.107	SPT8		Light Gray Fine Sand (SP-SM)														
37	13.50	200.607	SPT9			0	90	10	0	N	P	-							
	14.50	199.607	UDS5																
39	15.00	199.107	SPT10										2.63	1.66*		DST	0	33	
50	16.50	197.607	SPT11																
	17.50	196.61	UDS6																
59	18.00	196.11	SPT12			0	92	8	0	N	P	-							

Contd. On Table No. D3 b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 3			Termination Depth (m)		Date of Start: 27-03-2015		Table No. D3b			
Location:		Chainage (Km):		Reduced Level:		WATER TABLE (m) :			Termination Depth (m)		Date of Completion: 28-03-2015		Job No. 201414						
MAJOR BRIDGE		13+841		214.107		13.60			30.00										
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
64	19.50	194.607	SPT13	[Symbol]	Light Gray Fine Sand (SP-SM)														
	20.50	193.607	UDS7																
64	21.00	193.107	SPT14	[Symbol]	(22.50m)														
64	22.50	191.607	SPT15		Light Gray Silty Fine Sand (SM)														
	23.50	190.607	UDS8			6	67	27	0	N	P	-	2.64	2.00	1.71	17.2	DST	0	34
69	24.00	190.107	SPT16	[Symbol]	(25.50m)														
70	25.50	188.607	SPT17		Light Gray Fine Sand (SP-SM)														
	26.50	187.607	UDS9																
76	27.00	187.107	SPT18	[Symbol]		0	91	9	0	N	P	-							
75	28.50	185.607	SPT19																
	29.50	184.607	UDS10																
82	30.00	184.107	SPT20	[Symbol]	(30.00m)	0	92	8	0	N	P	-							

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 29-03-2015		Table No. E1a				
					Location: RFO		Chainage (Km): 14+069		Reduced Level: 213.830		WATER TABLE (m) : 13.40		30.00		Date of Completion: 30-03-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	213.330	DS1		Light Gray Sandy Silt (ML-CL) (1.50m)	0	17	78	5										
21	1.50	212.330	SPT1		Light Gray Clayey Silt (CL)														
	2.50	211.330	UDS1			0	9	79	12	33	22	11	2.67	1.76	1.58	11.7	UUT	0.55	10
21	3.00	210.830	SPT2																
	4.50	209.330	SPT3		Light Gray Sandy Silt (ML-CL) (4.50m)														
24	4.50	209.330	SPT3		Light Gray Sandy Silt (ML-CL)														
	5.50	208.330	UDS2			0	16	78	6					1.80	1.60	12.5			
22	6.00	207.830	SPT4							26	20	6							
26	7.50	206.330	SPT5																
	8.50	205.330	UDS3			3	17	75	5				2.65	1.85	1.64	12.8	UUT	0.80	11
32	9.00	204.830	SPT6																
	10.50	203.330	SPT7		Light Gray Sandy Silt (ML) (10.50m)														
36	10.50	203.330	SPT7		Light Gray Sandy Silt (ML)														
	11.50	202.330	UDS4			0	33	64	3	N	P	-		1.88	1.66	13.2			
40	12.00	201.830	SPT8		Light Gray Fine Sand (SP-SM) (12.00m)														
47	13.50	200.330	SPT9			0	91	9	0										
	14.50	199.330	UDS5																
54	15.00	198.830	SPT10																
52	16.50	197.330	SPT11			0	93	7	0	N	P	-	2.63						
	17.50	196.33	UDS6																
60	18.00	195.83	SPT12												1.68*		DST	0	34

Contd. On Table No. E1b

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)	Date of Start: 29-03-2015	Table No. E1b							
Location: RFO		Chainage (Km): 14+069		Reduced Level: 213.830		WATER TABLE (m) : 13.40			30.00	Date of Completion: 30-03-2015		Job No. 201414								
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
61	19.50	194.330	SPT13		Light Gray Fine Sand (SP-SM)															
	20.50	193.330	UDS7																	
60	21.00	192.830	SPT14		(22.50m)	0	92	8	0	N	P	-								
52	22.50	191.330	SPT15		Light Gray Clayey Silt (CI)															
	23.50	190.330	UDS8			0	6	72	22				2.69	2.07	1.72	20.4				
50	24.00	189.830	SPT16							41	24	17								
61	25.50	188.330	SPT17																	
	26.50	187.330	UDS9		(27.00m)	0	9	73	18					2.07	1.73	19.70	UUT	1.65	9	
69	27.00	186.830	SPT18		Light gray Fine Sand (SP-SM)															
75	28.50	185.330	SPT19			0	91	9	0	N	P	-								
	29.50	184.330	UDS10																	
78	30.00	183.830	SPT20		(30.00m)										1.73*		DST	0.0	34	

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start: 31-03-2015		Table No. E2a				
					Location: RFO		Chainage (Km): 14+069		Reduced Level: 213.830		WATER TABLE (m) : 13.30		30.00		Date of Completion: 01-04-2015		Job No. 201414		
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
	0.50	213.330	DS1		Dark Gray Sandy Silt (ML-CL) (1.50m)	0	16	79	5										
20	1.50	212.330	SPT1		Light Gray Clayey Silt (CL)														
	2.50	211.330	UDS1		Light Brown Sandy Silt (ML-CL) (3.00m)	0	8	79	13	33	22	11	2.67	1.75	1.56	12.3	UUT	0.55	10
21	3.00	210.830	SPT2		Light Brown Sandy Silt (ML-CL)														
25	4.50	209.330	SPT3		Light Brown Sandy Silt (ML-CL)														
	5.50	208.330	UDS2		Light Brown Sandy Silt (ML) (6.00m)	0	19	75	6	25	19	6	2.66	1.80	1.60	12.8	UUT	0.75	12
20	6.00	207.830	SPT4		Light Brown Sandy Silt (ML)														
25	7.50	206.330	SPT5		Light Brown Sandy Silt (ML)														
	8.50	205.330	UDS3		Light Gray Silty Fine Sand (SM) (9.00m)	0	39	58	3	N	P	-	1.85	1.64	12.5				
30	9.00	204.830	SPT6		Light Gray Silty Fine Sand (SM)														
29	10.50	203.330	SPT7		Light Gray Silty Fine Sand (SM)														
	11.50	202.330	UDS4		Light Gray Fine Sand (SP-SM) (12.00m)	0	81	19	0	N	P	-	1.87	1.66	12.9	DST	0.00	33	
34	12.00	201.830	SPT8		Light Gray Fine Sand (SP-SM)														
32	13.50	200.330	SPT9		Light Gray Fine Sand (SP-SM)														
	14.50	199.330	UDS5		Light Gray Fine Sand (SP-SM)														
39	15.00	198.830	SPT10		Light Gray Fine Sand (SP-SM)														
45	16.50	197.330	SPT11		Light Gray Fine Sand (SP-SM)	0	90	10	0	N	P	-	2.63						
	17.50	196.33	UDS6		Light Gray Fine Sand (SP-SM)														
49	18.00	195.83	SPT12		Light Gray Fine Sand (SP-SM)										1.68*		DST	0	34

Contd. On Table No. E2b

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)	Date of Start: 31-03-2015	Table No. E2b						
					Location:	Chainage (Km):	Reduced Level:		WATER TABLE (m) :		30.00	Date of Completion: 01-04-2015	Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
53	19.50	194.330	SPT13		Light Gray Fine Sand (SP-SM)														
	20.50	193.330	UDS7																
51	21.00	192.830	SPT14		Light Gray Clayey Silt (CL)	0	92	8	0	N	P	-							
43	22.50	191.330	SPT15																
	23.50	190.330	UDS8			8	9	71	12				2.66	2.04	1.69	20.5			
51	24.00	189.830	SPT16		Light Gray Clayey Silt (CL)														
51	25.50	188.330	SPT17																
	26.50	187.330	UDS9			12	8	70	10	32	21	11		2.06	1.72	19.80	UUT	1.35	10
63	27.00	186.830	SPT18		Light gray Fine Sand (SP-SM)														
68	28.50	185.330	SPT19														1.72*		DST
	29.50	184.330	UDS10																
73	30.00	183.830	SPT20		Light Gray Fine Sand (SP-SM)	0	92	8	0	N	P	-							

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 21-03-2015		Table No.A-1				
		Location: Minor Bridge		Chainage (Km): 13+136		Reduced Level (m): 212.439		WATER TABLE (m) : Not Met		12.00		Date of Completion: 22-03-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
5	0.50	211.939	DS1		Light Gray Sandy Silt (ML-CL)	0	19	75	6				2.65	1.71	1.53	11.5	UUT	0.50	10
	1.50	210.939	SPT1								26	20							
15	2.50	209.939	UDS1		(4.50m) Light Gray sandy silt with gravels (ML-CL)	0	11	83	6				2.63	1.80	1.63	10.7	DST	0.00	31
	3.00	209.439	SPT2																
18	4.50	207.939	SPT3		(6.00m) Light Gray Silty Fine Sand (SP-SM)	6	17	71	6	25	19	6	2.63	1.80	1.63	10.7	DST	0.00	31
	5.50	206.939	UDS2																
21	6.00	206.439	SPT4		(12.00m)	0	55	45	0				2.63	1.80	1.63	10.7	DST	0.00	31
24	7.50	204.939	SPT5																
28	8.50	203.939	UDS3		(12.00m)	0	73	27	0	N	P	-	2.63	1.80	1.63	10.7	DST	0.00	31
	9.00	203.439	SPT6																
31	10.50	201.939	SPT7		(12.00m)	0	73	27	0	N	P	-	2.63	1.80	1.63	10.7	DST	0.00	31
	11.50	200.939	UDS4																
36	12.00	200.439	SPT8		(12.00m)														

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut			BH.No. 1		Termination Depth (m)		Date of Start:		Table No.B-1					
		Location: Minor Bridge		Chainage (Km): 15+227		Reduced Level (m): 214.423		WATER TABLE (m) : Not Met		12.00		Date of Completion: 04-03-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
5	0.50	213.923	DS1		Dark Gray Sandy Silt (ML-CL)	0	16	79	5										
	1.50	212.923	SPT1		(1.50m)														
7	2.50	211.923	UDS1		Light gray clayey silt of low plasticity (CL)	0	8	79	13	33	22	11	2.67	1.70	1.51	12.3	UUT	0.40	10
	3.00	211.423	SPT2																
18	4.50	209.923	SPT3		Light gray clayey silt of low plasticity (CL)	0	7	80	13	33	21	12		1.81	1.60	12.9			
	5.50	208.923	UDS2																
21	6.00	208.423	SPT4		Light gray clayey silt of low plasticity (CL)	0	7	80	13	33	21	12		1.81	1.60	12.9			
	29	7.50	SPT5																
34	8.50	205.923	UDS3		Light gray silty fine sand (SM)	0	54	46	0	N	P	-	2.64	1.82	1.64	11.2	DST	0.00	31
	9.00	205.423	SPT6		(8.50m)														
38	10.50	203.923	SPT7		Light gray silty fine sand (SM)	0	8	78	14	34	22	12	2.68	1.89	1.66	13.7			
	11.50	202.923	UDS4		Light gray clayey silt of low plasticity (CL)														
42	12.00	202.423	SPT8		Light gray clayey silt of low plasticity (CL)														

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start:		Table No.C-1					
		Location: Minor Bridge		Chainage (Km): 16+144		Reduced Level (m): 213.770		WATER TABLE (m) : Not Met		12.00		04-04-2015		Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
16	0.50	213.270	DS1		Light gray clayey silt of low plasticity (CL)	0	7	80	13											
	1.50	212.270	SPT1																	
	2.50	211.270	UDS1								34	22	12	2.68	1.71	1.53	11.9	UUT	0.50	9
8	3.00	210.770	SPT2																	
16	4.50	209.270	SPT3					0	10	76	14									
	5.50	208.270	UDS2									33	22	11	1.79	1.58	13.5			
13	6.00	207.770	SPT4																	
16	7.50	206.270	SPT5				(7.50m)													
	8.50	205.270	UDS3		Light gray sandy silt (ML)	0	43	54	3	N	P	-	2.64							
	9.00	204.770	SPT6																	
29	10.50	203.270	SPT7				0	9	74	17										
33	11.50	202.270	UDS4			Light gray clayey silt of medium plasticity (CI)					44	24	20	2.70	1.91	1.65	15.9	DST	0.00	30
37	12.00	201.770	SPT8		(12.00m)															

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-04-2015		Table No. D-1				
		Location: Minor Bridge		Chainage (Km): 17+338		Reduced Level (m): 214.015		WATER TABLE (m) : Not Met		12.00		Date of Completion 04-05-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
8	0.50	213.515	DS1	[Symbol]	Dark gray sandy silt of low plasticity (CL)	0	16	71	13	33	22	10	2.68	1.67	1.51	10.7	UUT	0.40	10
	1.50	212.515	SPT1																
	2.50	211.515	UDS1																
10	3.00	211.015	SPT2		Light gray clayey silt of low plasticity (CL)	1	12	73	14	34	22	12	1.74	1.56	12.5	UUT	1.05	11	
13	4.50	209.515	SPT3																
	5.50	208.515	UDS2																
25	6.00	208.015	SPT4		Light gray sandy silt (ML-CL)	6	16	72	6	26	19	7	1.86	1.64	13.2	UUT	1.05	11	
26	7.50	206.515	SPT5																
	8.50	205.515	UDS3																
28	9.00	205.015	SPT6																
32	10.50	203.515	SPT7																
	11.50	202.515	UDS4	Light gray sandy silt (ML-CL)	0	19	75	6				2.66	1.88	1.66	13.5	UUT	1.05	11	
28	12.00	202.015	SPT8																


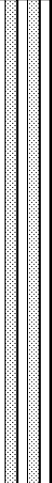
		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-05-2016		Table No. E-1								
		Location: Minor Bridge		Chainage (Km): 18+070		Reduced Level (m): 214.220		WATER TABLE (m) : Not Met			12.00		Date of Completion 04-06-2016		Job No. 201414								
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test						
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction				
8	0.50	213.720	DS1		Light gray sandy silt (ML-CL) (2.50m)	2	17	75	6	25	19	6	2.64	1.72	1.53	12.5	DST	0.00	29				
	1.50	212.720	SPT1																		N	P	-
10	2.50	211.720	UDS1		Light gray sandy silt (ML) (4.50m)	0	46	51	3				2.64	1.72	1.53	12.5	DST	0.00	29				
	3.00	211.220	SPT2																				
16	4.50	209.720	SPT3		Light Gray clayey silt of low plasticity (CL) (11.50m)	4	6	77	13				2.68	1.84	1.60	14.8	UUT	1.04	10				
	5.50	208.720	UDS2																		34	22	12
21	6.00	208.220	SPT4																				
19	7.50	206.720	SPT5																				
	8.50	205.720	UDS3																				
18	9.00	205.220	SPT6																				
23	10.50	203.720	SPT7		Light Gray fine sand (SP-SM) (12.00m)	0	90	10	0				2.68	1.84	1.60	14.8	UUT	1.04	10				
	11.50	202.720	UDS4																		N	P	-
24	12.00	202.220	SPT8																				

SOIL PROFILE					Project :Geotechnical Investigation for Hapur - Muzaffarnagar Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-06-2015		Table No. F-1				
					Chainage (Km): 19+051		Reduced Level (m): 214.650		WATER TABLE (m) : Not Met			12.00		Date of Completion 04-06-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
7	0.50	214.150	DS1		Light gray sandy silt (ML-CL)	1	17	76	6	25	19	6	2.66	1.70	1.52	12.1	UUT	0.40	10
	1.50	213.150	SPT1			0	22	72	6										
	2.50	212.150	UDS1																
8	3.00	211.650	SPT2		Light Gray silty sand (SM) (4.50m)	0	56	44	0	N	P	-	2.63	1.78	1.58	12.5	DST	0.00	30
	15	4.50	210.150			SPT3													
20	5.50	209.150	UDS2		Light gray sandy silt (ML-CL) (6.00m)	2	15	77	6	26	20	6	1.86	1.64	13.3	UUT	0.80	12	
	22	7.50	207.150			SPT5													
24	8.50	206.150	UDS3		Light Gray silty fine sand (SM) (11.50m)	0	22	72	6	N	P	-	2.66						
	22	10.50	204.150			SPT7													
25	11.50	203.150	UDS4		Light Gray silty fine sand (SM) (12.00m)	0	80	20	0	N	P	-							
	12.00	202.650	SPT8																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-07-2015		Table No. G-1				
		Location: Minor Bridge		Chainage (Km): 19+955		Reduced Level (m): 214.234		WATER TABLE (m) : Not Met		12.00		Date of Completion 04-07-2015		Job No. 201414					
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
6	0.50	213.734	DS1		Dark gray sandy silt (ML-CL)	0	19	75	6				2.66	1.72	1.53	12.5	UUT	0.50	10
	1.50	212.734	SPT1																
	2.50	211.734	UDS1																
12	3.00	211.234	SPT2		Light gray silty fine sand with gravels (SM)	3	64	33	0	N	P	-	2.63	1.81	1.63	11.2	DST	0.00	31
16	4.50	209.734	SPT3																
	5.50	208.734	UDS2																
19	6.00	208.234	SPT4																
22	7.50	206.734	SPT5																
	8.50	205.734	UDS3																
23	9.00	205.234	SPT6																
26	10.50	203.734	SPT7																
	11.50	202.734	UDS4																
30	12.00	202.234	SPT8																

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 2		Termination Depth (m)		Date of Start:		Table No. G-2					
		Location: Minor Bridge		Chainage (Km): 19+955		Reduced Level (m): 214.234		WATER TABLE (m) : Not Met		12.00		Date of Completion 04-08-2015		Job No. 201414						
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test			
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction	
6	0.50	213.734	DS1		Light gray sandy silt (ML-CL) (3.00m)	0	38	56	6											
	1.50	212.734	SPT1																	
	2.50	211.734	UDS1				6	30	59	5	25	19	6	2.66	1.72	1.53	12.2	UUT	0.50	10
11	3.00	211.234	SPT2		Light gray silty fine sand (SP-SM) (12.000m)															
13	4.50	209.734	SPT3				1	92	7	0	N	P	-							
	5.50	208.734	UDS2											1.76	1.58	11.3				
18	6.00	208.234	SPT4																	
19	7.50	206.734	SPT5				0	91	9	0	N	P	-							
	8.50	205.734	UDS3											2.62	1.82	1.63	11.9	DST	0.00	32
26	9.00	205.234	SPT6																	
34	10.50	203.734	SPT7																	
	11.50	202.734	UDS4																	
36	12.00	202.234	SPT8			0	90	10	0	N	P	-								

		SOIL PROFILE			Project : Geotechnical Investigation for Hapur - Meerut Section of DFCC Meerut				BH.No. 1			Termination Depth (m)		Date of Start: 04-08-2015		Table No. H-1			
		Location: Minor Bridge		Chainage (Km): 20+935		Reduced Level (m): 215.001		WATER TABLE (m) : Not Met			12.00		Date of Completion 04-08-2015		Job No. 201414				
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
12	0.50	214.501	DS1		Light brown sandy silt (ML-CL)	0	21	73	6										
	1.50	213.501	SPT1																
	2.50	212.501	UDS1							25	19	6	2.66	1.72	1.53	12.2	UUT	0.50	10
10	3.00	212.001	SPT2																
14	4.50	210.501	SPT3																
	5.50	209.501	UDS2											1.78	1.58	12.5			
18	6.00	209.001	SPT4																
18	7.50	207.501	SPT5																
	8.50	206.501	UDS3							26	20	6		1.86	1.63	14.2	UUT	0.80	12
31	9.00	206.001	SPT6			(9.00m)													
28	10.50	204.501	SPT7		Light Gray silty fine sand (SM)	4	72	24	0			2.64							
	11.50	203.501	UDS4											1.87	1.65	13.2	DST	0.00	31
34	12.00	203.001	SPT8			(12.00m)	0	76	24	0	N	P	-						

		SOIL PROFILE			Project :Geotechnical Investigation for Hapur - Muzaffarnagar Section of DFCC Meerut				BH.No. 1		Termination Depth (m)		Date of Start: 04-09-2015		Table No. I-1				
					Chainage (Km): 22+200		Reduced Level (m): 216.485		WATER TABLE (m) : Not Met			12.00		Date of Completion 04-09-2015		Job No. 201414			
N-Value	Depth (m)	Reduced Level (m)	Sample No.	Symbol	SOIL DESCRIPTION	Grain Size Analysis				Atterberg Limits			Specific Gravity	Natural Density gms/cm ³	Dry Density gms/cm ³	Moisture Content %	Triaxial Test		
						Gravel %	Sand %	Silt %	Clay %	Liquid %	Plastic %	Plasticity Index %					Type of Test	Cohesion Intercept Kg/cm ²	Angle of Internal Friction
8	0.50	215.985	DS1		Light gray clayey silt of low plasticity (CL) (3.00m)	3	9	75	13	33	22	11	2.67	1.74	1.54	13.3	UUT	0.50	10
	1.50	214.985	SPT1																
	2.50	213.985	UDS1																
14	3.00	213.485	SPT2		Light gray silty fine sand (SM) (12.00m)	0	86	14	0	N	P	-	2.64	1.78	1.58	12.4	DST	0.00	31
12	4.50	211.985	SPT3																
	5.50	210.985	UDS2																
19	6.00	210.485	SPT4																
16	7.50	208.985	SPT5																
	8.50	207.985	UDS3																
22	9.00	207.485	SPT6																
27	10.50	205.985	SPT7																
	11.50	204.985	UDS4																
32	12.00	204.485	SPT8																

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : 24+920
 Co-ordinate : E:-758845.2131 N:-3189173.561
 Reduce Level: 215.810 m

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : Nil m
 Termination Depth : 12.45 m

Borehole No. 1
 Page 1 of 1



Date : 19/04/2015 to 20/04/2015

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N-value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil																								
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr	c (kPa)	Φ (°)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																					
1		Medium Dense Sandy SILT		ML	0.50-1.00	DS-1																																														
2	1.50-1.95				SPT-1	11	15																																													
3	2.50-2.80				UDS-1			25	NP			0	49	48	3	2.8	1.85	1.8	2.71																																	
4		Medium Dense Silty SAND		SM	3.00-3.45	SPT-2	12	14					0	50	50	0											0			30																						
5	4.50-4.95				SPT-3	16	16																																													
6		Medium to Dense Fine SAND		SP	5.50-5.80	UDS-2							0	96	4	0	12	1.87	1.67	2.7							0			32.4		NIL	139.0	7.69																		
7	6.00-6.45				SPT-4	21	17					0	96	4	0													0			34.0																					
8	7.50-7.95			SPT-5	22	17																																														
9	9.00-9.45			SPT-6	26	18																																														
10				SP-SM			10.50-10.95	SPT-7	28	18					0	91	9	0											0			34.0																				
11	12.00-12.45						SPT-8	32	19																																											
12																																																				
13				Borehole terminated at depth of 12.45m																																																
14																																																				
15																																																				
16																																																				
17																																																				
18																																																				
19																																																				
20																																																				

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Location : 26+530

Borehole dia : 150 mm

Page 1 of 1

Co-ordinate: E:-758555.4818 N:-3190758.323

Ground Water Table : 8.00 m

Termination Depth : 12.45 m

Date : 22/04/2015 to 22/04/2015

Reduce Level: 216.395 m



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil										
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _s	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)							
1		Very Loose to Loose Sandy SILT			0.50-1.00	DS-1																																
2						1.50-1.95	SPT-1	2	3																													
3					ML	2.50-2.80	UDS-1			23	NP			0	41	59	0	12.3	1.92	1.71	2.72																	
4					ML	3.00-3.45	SPT-2	8	9					0	39	61	0									0		29.0										
5		Medium Dense Silty SAND			4.50-4.95	SPT-3	12	12																														
6				SP-SM	5.50-5.80	UDS-2							0	89	11	0	10.4	1.99	1.8	2.69																		
7				SM	6.00-6.45	SPT-4	20	17					0	74	26	0									0		31.5											
8						7.50-7.95	SPT-5	21	17																0		32.0											
9																																						
10		Medium to Dense Fine SAND			9.00-9.45	SPT-6	26	18																														
11				SP-SM	10.50-10.95	SPT-7	30	19				0	92	8	0									0		32.4												
12						12.00-12.45	SPT-8	34	20																													
13		Borehole terminated at depth of 12.45m																																				
14																																						
15																																						
16																																						
17																																						
18																																						
19																																						
20																																						

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
Location : 27+290
Co-ordinate : E:- 758421.4841 N:- 3191491.2589
Reduce Level: 219.350

Method of Boring : Rotary
Borehole dia : 150 mm
Ground Water Table : 9.00 m
Termination Depth : 30.45 m

Borehole No. 1
 Page 1 of 2



Date : 24/04/2015 to 24/04/2015

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear				Chemical Analysis of soil																							
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _s	Cc	Pc	Cr	c (kPa)	Φ (°)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																						
1		Loose Silty SAND		SM	0.50-1.00	DS-1																																															
2																																																					
3								2.50-2.80	UDS-1			24	NP		0	55	45	0	10.7	1.88	1.7								0	31																							
4							SM	3.00-3.45	SPT-2	7	8				1	59	40	0											0	31																							
5								4.50-4.95	SPT-3	12	12																																										
6		Medium Dense Sandy SILT		ML-CL	5.50-5.80	UDS-2			34	24	10	9	5	58	28	17.6	1.96	1.67	2.67						103	0																											
7																																																					
8								7.50-7.95	SPT-5	16	14																																										
9								9.00-9.45	SPT-6	22	16				3	6	84	7											0	29.7																							
10					10.50-10.95	SPT-7	26	17																																													
11		Very Stiff Silty CLAY		CL	11.50-11.80	UDS-3			33	19	14	0	4	65	31	22	2.01	1.65	2.68					120	0																												
12								12.00-12.45	SPT-8	33	19																																										
13		Dense to Vey Dense Fine SAND		SP-SM	13.50-13.95	SPT-9	29	17																																													
14																																																					
15								15.00-15.45	SPT-10	39	20																																										
16								16.50-16.80	SPT-11	42	20				0	95	5	0											0	33.0																							
17								18.00-18.45	SPT-12	48	21																																										
18								19.50-19.95	SPT-13	53	22																																										
19																																																					
20																																																					

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : 28+660
 Co-ordinate: E:- 758099.3974 N:-3192835.787
 Reduce Level: 217.975 m

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 12.45 m

Borehole No. 1
 Page 1 of 1



Date : 25/04/2015 to 25/04/2015

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil																									
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _s	Cc	Pc	Cr	c (kPa)	Φ (°)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																						
1		Firm Silty CLAY		CL	0.50-1.00	DS-1																																															
2					1.50-1.95	SPT-1	7																																														
3		Medium Dense Sandy SILT		ML	2.50-2.80	UDS-1			25	17	8	0	12	79	9	14.9	1.93	1.68	2.69	0.518	0.174	180	0.018	42	0	0	32																										
4					3.00-3.45	SPT-2	10	12						9	20	71	0																																				
5				4.50-4.95	SPT-3	13	13																																														
6				5.50-5.80	UDS-2					27	NP		21	9	67	3	17.5	2.0	1.7	2.62					80	0																											
7				6.00-6.45	SPT-4	16	15																																														
8				7.50-7.95	SPT-5	16	14			23	NP		14	16	70	0																																					
9				9.00-9.45	SPT-6	14	11																																														
10				Medium to Dense Fine SAND		SP-SM	10.50-10.95	SPT-7						0	92	8	0												0	33																							
11		12.00-12.45	SPT-8				34	20																																													
12		Borehole terminated at depth of 12.45m																																																			
13																																																					
14																																																					
15																																																					
16																																																					
17																																																					
18																																																					
19																																																					
20																																																					

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : 28+880
 Co-ordinate: E:- 758061 N:- 3192970
 Reduce Level: 218.115

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 1 of 2



Date : 16/01/2016

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil										
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)							
					0.00-0.50	DS-1																																
1																																						
2					1.50-1.95	SPT-1	18	22																														
				ML	2.00-2.50	UDS-1			29		NP		13	27	47	13	15.8	1.99	1.72	2.61					56	0					Nil	168.79	7.61					
3					3.00-3.45	SPT-2	20	22																														
4																																						
5					4.50-4.95	SPT-3	17	18																														
				ML	5.00-5.50	UDS-2							1	14	77	8																						
6					6.00-6.45	SPT-4	19	19																														
7																																						
8					7.50-7.95	SPT-5	20	18																														
9																																						
10					9.00-9.45	SPT-6	23	19																														
11					10.50-10.95	SPT-7	25	20																														
				ML	11.00-11.50	UDS-4			31		NP		7	8	71	14				2.60					53	0												
12					12.00-12.45	SPT-8	22	18																														
13					13.50-13.95	SPT-9	25	19																														
14																																						
15																																						
16					15.00-15.45	SPT-10	28	20																														
17					16.50-16.95	SPT-11	30	20					0	98		2																						
				SP	18.00-18.45	SPT-12	28	19																														
18																																						
19																																						
20					19.50-19.95	SPT-13	33	20																														

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : 28+880
 Co-ordinate: E:- 758061 N:- 3192970
 Reduce Level: 218.115

Method of Boring : Shell & Auger
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 2 of 2
 Date : 16/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil											
					Depth (m)	Type	(Obs)	(Corr)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (°)	c (kPa)	φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)								
21		Dense Fine SAND		SP	21.00-21.45	SPT-14	38	22				0	99	1				2.68							0	33.3													
22																																							
23								22.50-22.95	SPT-15	43	23																												
24								24.00-24.45	SPT-16	42	22																												
25								25.50-26.00	SPT-17	49	24																												
26		Dense To Very Dense Fine SAND		SP	27.00-27.45	SPT-18	53	25				0	98	2											0	34.0													
27																																							
28								28.50-28.95	SPT-19	49	23																												
29								30.00-30.45	SPT-20	55	25																												
30																																							
31		Borehole terminated at depth of 30.45m																																					
32																																							
33																																							
34																																							
35																																							
36																																							
37																																							
38																																							
39																																							
40																																							

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : 33+050
 Co-ordinate: E:- 756863 N:- 3197017
 Reduce Level: 221.451

Method of Boring : Shell & Auger
 Borehole dia : 150 mm
 Ground Water Table : 6.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 2 of 2
 Date : 16/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	(Obs)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)									
21		Dense Fine SAND		SW-SM	21.00-21.45	SPT-14	33	20				0	95	5																										
22																																								
23								22.50-22.95	SPT-15	28	18																													
24								24.00-24.45	SPT-16	39	21																													
25								25.50-26.00	SPT-17	46	23																													
26																																								
27		Very Dense Fine SAND		SP	27.00-27.45	SPT-18	44	22				0	99	1													0		35.0											
28																																								
29								28.50-28.95	SPT-19	52	24																													
30								30.00-30.45	SPT-20	53	24																													
31		Borehole terminated at depth of 30.45m																																						
32																																								
33																																								
34																																								
35																																								
36																																								
37																																								
38																																								
39																																								
40																																								

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Location : CH 34+360
Co-ordinate: E:- 756485.87 N:- 3198289.30

Borehole dia : 150 mm

Page 2 of 2

Ground Water Table : 6.00 m

Termination Depth : 30.45 m

Date : 21/2/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c	φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)													
																																φ	σ											
21		Hard Silty CLAY		CI	20.00-20.50	UDS-7			45	23	22	0	7	67	26	21	1.98	1.64	2.64																									
22		Medium Dense to Very Dense Fine SAND		SW-SM	21.00-21.45	SPT-14																																						
23					22.50-22.95	SPT-15																																						
24					24.00-24.45	SPT-16																																						
25					25.50-25.95	SPT-17																																						
26					27.00-27.45	SPT-18									0	94		6												0														
27					28.50-28.95	SPT-19																																						
28					30.00-30.45	SPT-20																																						
30		Borehole terminated at depth of 30.45m																																										
31																																												
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36																																												
37																																												
38																																												
39																																												
40																																												

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 2

Page 2 of 2

Location : 34+986
Co-ordinate: E:- 756198 N:- 3198847

Borehole dia : 150 mm

Ground Water Table : 3.50 m

Termination Depth : 30.45 m

Date : 18/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil													
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (°)	c (kPa)	φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)										
21		Dense To Very Dense Fine SAND		SP	21.00-21.45	SPT-14	38	22				0	98	2																											
22																																									
23								22.50-22.95	SPT-15	38	21																														
24								24.00-24.45	SPT-16	43	23																														
25								25.50-26.00	SPT-17	47	24																														
26								27.00-27.45	SPT-18	46	23				0	98	2												0		35										
27								28.50-28.95	SPT-19	49	23																														
28								30.00-30.45	SPT-20	57	26																														
29					Borehole terminated at depth of 30.45m																																				
30																																									
31																																									
32																																									
33																																									
34																																									
35																																									
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37																																									
38																																									
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Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 2 of 2



Location : 35+549
Co-ordinate: E:- 755865 N:- 3199300

Borehole dia : 150 mm
Ground Water Table : 3.75 m

Reduce Level: 221.929

Termination Depth : 30.45 m

Date : 19/01/2016

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (°)	c (kPa)	φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)									
21																																								
22																																								
23						22.50-22.95	SPT-15	42	23																															
24																																								
25		Dense Fine SAND		SP	24.00-24.45	SPT-16	45	23				0	99																											
26					25.50-26.00	SPT-17	52	25																																
27					27.00-27.45	SPT-18	49	24																																
28					28.50-28.95	SPT-19	58	26						0	98																									
29				SP	30.00-30.45	SPT-20	61	27																																
30		Borehole terminated at depth of 30.45m																																						
31																																								
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Prepared By: _____

Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 1 of 1

Location : 37+360
Co-ordinate: E:- 754806 N:- 3200772

Borehole dia : 150 mm

Ground Water Table : 2.00 m

Termination Depth : 12.45 m

Date : 16/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (°)	c (kPa)	φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)									
1		Loose Silty SAND			0.00-0.50	DS-1																																		
2					1.50-1.95	SPT-1	9	15																																
3		Medium Dense Silty SAND			2.00-2.50	UDS-1						0	58	31	11	21.9	1.98	1.62							0	31.3			Nil	139	7.2									
4					3.00-3.45	SPT-2	17	19																																
5					4.50-4.95	SPT-3	19	20																																
6		Medium Dense Fine SAND			6.00-6.45	SPT-4	16	17				1	87		12										0	33.7														
7					7.50-7.95	SPT-5	19	18																																
8					9.00-9.45	SPT-6	21	18						2	80		18										0	33.3												
9					10.50-10.95	SPT-7	23	19																																
10					12.00-12.45	SPT-8	25	19				0	95		5									0	34.3															
11		Borehole terminated at depth of 12.45m																																						
12																																								
13																																								
14																																								
15																																								
16																																								
17																																								
18																																								
19																																								
20																																								

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 1 of 2

Location : 38+580
Co-ordinate: E:- 754609 N:- 3201967

Borehole dia : 150 mm

Ground Water Table : 2.00 m

Termination Depth : 30.45 m

Date : 20/01/2016

Reduce Level: 220.980



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil													
					Depth (m)	Type	(Obs)	(Corr)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)										
0.00-0.50		Loose Silty SAND		SM	DS-1																																				
1.50-1.95	SPT-1				9	15																																			
2.00-2.50	UDS-1										0	68	29	3	22.3	1.95	1.59									0	29.8	NII	129.07	7.26											
3.00-3.45	SPT-2																																								
4.50-4.95	SPT-3																																								
6.00-6.45	SPT-4																																								
7.50-7.95	SPT-5			SP													2.63							0	32.0																
9.00-9.45	SPT-6																																								
10.50-10.95	SPT-7																																								
12.00-12.45	SPT-8																																								
13.50-13.95	SPT-9																								0	34															
15.00-15.45	SPT-10																																								
16.50-16.95	SPT-11																																								
18.00-18.45	SPT-12			ML					28		NP	3	14	67	16																										
19.50-19.95	SPT-13																																								

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 2 of 2

Location : 38+580
Co-ordinate: E:- 754609 N:- 3201967

Borehole dia : 150 mm

Ground Water Table : 2.00 m

Termination Depth : 30.45 m

Date : 20/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	(Obs)	(Corr)	LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)													
21		Dense Fine SAND		SP	21.00-21.45	SPT-14	39	22																																				
22																																												
23								22.50-22.95	SPT-15	42	23																																	
24								24.00-24.45	SPT-16	42	22				0	96		4										0				33.3												
25								25.50-26.00	SPT-17	50	25																																	
26																																												
27																																												
28																																												
29		Very Dense Fine SAND		SP	27.00-27.45	SPT-18	49	24																																				
30					28.50-28.95	SPT-19	57	26																																				
31		Borehole terminated at depth of 30.45m					30.00-30.45	SPT-20	60	27			0	96		4																												
32																																												
33																																												
34																																												
35																																												
36																																												
37																																												
38																																												
39																																												
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Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 1 of 2

Location : 39+120
Co-ordinate: E:- 754572 N:- 3202506

Borehole dia : 150 mm

Ground Water Table : 0.60 m

Termination Depth : 30.45 m

Date : 20/01/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				Triaxial Test		Direct Shear		Chemical Analysis of soil								
					Depth (m)	Type	N-value		LL	PL	PI	Gravel	Sand	Silt	Clay					e _p	Cc	Pc	Cr	c (kPa)	φ (o)	c (kPa)	φ (o)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)					
					(Obs)	(Corr)																														
		Loose To Medium Dense Silty SAND		SP	0.00-0.50	DS-1																														
1																																				
2								1.50-1.95	SPT-1	10	16				0	96									0											
3								3.00-3.45	SPT-2	14	20																									
4								4.50-4.95	SPT-3	22	21																									
5								6.00-6.45	SPT-4	27	23	29		NP	3	6	79	12			2.59					0		0		33.5		NII	129.07	7.43		
6		Medium Dense Sandy SILT		ML	7.50-7.95	SPT-5	32	25																												
7								9.00-9.45	SPT-6	26	21																									
8								10.50-10.95	SPT-7	30	22																									
9								12.00-12.45	SPT-8	33	23				0	97		3										0		33.0						
10								13.50-13.95	SPT-9	36	24																									
11								15.00-15.45	SPT-10	39	24																									
12		Dense Fine SAND		SP	16.50-16.95	SPT-11	42	25																												
13								18.00-18.45	SPT-12	35	22	33		20	13	0	7	72	21																	
14								19.50-19.95	SPT-13	38	22																									
15																																				
16		Hard Silty CLAY		CL																																
17																																				
18																																				
19																																				
20																																				

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 41+916
 Co-ordinate : E:- 754838.12 N:-3205227.62
 Reduce Level: 224.585

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 5.00 m
 Termination Depth : 30.00 m

Borehole No. 1
 Page 1 of 2
 Date : 21-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value (Obs.) (Corr.)	Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil									
					Depth (m)	Type		LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)						
1		Firm Silty CLAY		CL	0.50-1.00	DS-1																														
2					1.50-1.95	SPT-1	6																													
3					2.00-2.50	UDS-1	33	18	15	0	18	70	12	21.7	1.92	1.58	2.61								57											
4					Stiff to Very Stiff Silty CLAY		CL	3.00-3.45	SPT-2	14																										
5								4.50-4.95	SPT-3	15																										
6		6.00-6.45	SPT-4	20																																
7		7.50-7.95	SPT-5	24																																
8		8.00-8.50	UDS-3	27				20	7	0	12	74	14	18.4	2.02	1.71									143			NIL	139	8.02						
9		Hard Silty CLAY		CL	9.00-9.45	SPT-6	31																													
10					10.50-10.95	SPT-7	33																													
11					11.00-11.50	UDS-4	26	19	7	1	5	82	12	22.1	2	1.64									133											
12					12.00-12.45	SPT-8	38																													
13					Dense Sandy SILT		ML	13.50-13.95	SPT-9	33																										
14		14.00-14.50	UDS-5	32				NP		3	10	81	6	37.1	1.98	1.44																				
15		15.00-15.45	SPT-10	38				24																												
16		16.50-16.95	SPT-11	41				25																												
17		18.00-18.45	SPT-12	34				21																												
18		Dense Sandy SILT		ML	18.00-18.45	SPT-12	34	21																												
19					19.50-19.95	SPT-13	37	22																												
20																																				

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 2 of 2

Location : CH 41+916

Borehole dia : 150 mm

Co-ordinate: E:- 754838.12 N:-3205227.62

Ground Water Table : 5.00 m



Reduce Level: 224.585

Termination Depth : 30.00 m

Date : 21-01-2016

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil											
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	C _c	P _c	Cr			c (kPa)	Φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)								
21		Hard Silty CLAY		CL	20.00-20.50	UDS-7			27	19	8	2	15	68	15	20.8	1.98	1.61	2.64																				
22							21.00-21.45	SPT-14		42																													
23		Dense to Very Dense Sandy SILT			22.50-22.95	SPT-15		44																															
24				ML	23.00-23.50	UDS-8			28		NP		0	15	77	8	19.1	1.99	1.67							0		32.1											
25							24.00-24.45	SPT-16		48	25																												
26							25.50-25.95	SPT-17		52	25																												
27							27.00-27.45	SPT-18		49	24																												
28							28.50-28.95	SPT-19		56	26																												
29					30.00-30.45	SPT-20		61	27																														
30		Borehole terminated at depth of 30.45m																																					
31																																							
32																																							
33																																							
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35																																							
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Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 43+900
 Co-ordinate : E:-755233.19 N:- 3207147.35
 Reduce Level: 223.979

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.50 m
 Termination Depth : 30.00 m

Borehole No. 1
 Page 1 of 2
 Date : 22-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																		
					Depth (m)	Type	N - value	LL	PL	PI	Gravel	Sand	Silt	Clay	e ₀					Cc	Pc	Cr	c (kPa)			φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																
																															(Obs.)	(Corr.)														
1		Very Stiff Silty CLAY		CL	0.50-1.00	DS-1																																								
2					1.50-1.95	SPT-1	21																																							
3					2.00-2.50	UDS-1		30	18	12	1	14	74	11	13.6	1.72	1.51																													
4					3.00-3.45	SPT-2	25																																							
5					Medium Dense Sandy SILT		ML	4.50-4.95	SPT-3	22																																				
6								5.00-5.50	UDS-2		30	NP		0	18	76	6	11.3	1.68	1.51	2.64																									
7								6.00-6.45	SPT-4	18	18																																			
8								7.50-7.95	SPT-5	14	15																																			
9					Very Stiff Silty CLAY		CL	8.00-8.50	UDS-3		33	20	13	0	16	73	11	19.1	1.9	1.6																										
10								9.00-9.45	SPT-6	21																																				
11								10.50-10.95	SPT-7	24																																				
12		11.00-11.50	UDS-4					32	21	11	4	4	68	24	21.8	2.06	1.69																													
13		12.00-12.45	SPT-8	25																																										
14		13.50-13.95	SPT-9	27																																										
15		15.00-15.45	SPT-10	29																																										
16		16.50-16.95	SPT-11	34																																										
17		Dense Fine SAND		SP				18.00-18.45	SPT-12	37	22			1	96						2.66																									
18								19.50-19.95	SPT-13	40	23																																			
19																																														
20																																														

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 2 of 2

Location : CH 43+900

Borehole dia : 150 mm

Co-ordinate: E:-755233.19 N:- 3207147.35

Ground Water Table : 9.50 m

Reduce Level: 223.979

Termination Depth : 30.00 m

Date : 22-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	C _c	P _c	Cr			c (kPa)	φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)									
21		Dense to Vey Dense Fine SAND		SP	20.00-20.50	UDS-7																																		
22					21.00-21.45	SPT-14	42	23																																
23					22.50-22.95	SPT-15	40	22																																
24					24.00-24.45	SPT-16	43	23																																
25					25.50-25.95	SPT-17	47	24							0	98	2																							
26					27.00-27.45	SPT-18	52	25																																
27					28.50-28.95	SPT-19	57	26																																
28					30.00-30.45	SPT-20	64	28																																
29					Borehole terminated at depth of 30.45m																																			
30																																								
31																																								
32																																								
33																																								
34																																								
35																																								
36																																								
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Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 46+362
 Co-ordinate : E:- 755356.32 N:- 3209599.42
 Reduce Level: 226.246

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 10.00 m
 Termination Depth : 30.00 m
 Borehole No. 1
 Page 2 of 2
 Date : 24-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	Φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)									
21		Hard Silty CLAY		CL	20.00-20.50	UDS-7			30	19	11	0	11	75	14	19.4	2.04	1.71	2.65																					
22					21.00-21.45	SPT-14	59																																	
23					22.50-22.95	SPT-15	56																																	
24					23.00-23.50	UDS-8		29	20	9	0	12	76	12	23.7	2.02	1.63									230														
25					24.00-24.45	SPT-16	58																																	
26					25.50-25.95	SPT-17	52																																	
27					Very Dense Fine SAND		SP	27.00-27.45	SPT-18	58	27				0	97	3										0	34.7												
28		28.50-28.95	SPT-19	62				28																																
29		30.00-30.45	SPT-20	76				32																																
30		Borehole terminated at depth of 30.45m																																						
31																																								
32																																								
33																																								
34																																								
35																																								
36																																								
37																																								
38																																								
39																																								
40																																								

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 1 of 2

Location : CH 48+122

Borehole dia : 150 mm

Co-ordinate: E:- 755031.57 N:- 3211329.88

Ground Water Table : 9.00 m



Reduce Level: 224.318

Termination Depth : 30.00 m

Date : 23-01-2016

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	N - value	LL	PL	PI	Gravel	Sand	Silt	Clay	e ₀					C _c	P _c	Cr	c (kPa)			φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)														
					(Obs.)	(Corr.)																																						
1		Stiff Silty CLAY		CI	0.50-1.00	DS-1																																						
2					1.50-1.95	SPT-1	10																																					
3					2.00-2.50	UDS-1					37	21	16	0	5	70	25	20.4	1.96	1.63																								
4					3.00-3.45	SPT-2	12																																					
5					4.50-4.95	SPT-3	15																																					
6					Medium Dense Sandy SILT		ML	6.00-6.45	SPT-4	18	18				0	9	86	5				2.60																						
7		7.50-7.95	SPT-5	20				18																																				
8		9.00-9.45	SPT-6	23				19																																				
9		10.50-10.95	SPT-7	25				20																																				
10		11.00-11.50	UDS-4						29				NP	1	5	85	9	22.1	1.99	1.63										0		30												
11		12.00-12.45	SPT-8	22				18																																				
12		13.50-13.95	SPT-9	25				19																																				
13		14.00-14.50	UDS-5						28				NP	5	9	80	6	16.9	2	1.71																								
14		15.00-15.45	SPT-10	29				20																																				
15		16.50-16.95	SPT-11	30				20																																				
16		Hard Silty CLAY		CL				17.00-17.50	UDS-6			30	20	10	7	9	72	12	21.8	2.06	1.69	2.66																						
17					18.00-18.45	SPT-12	36																																					
18					19.50-19.95	SPT-13	32																																					
19																																												
20																																												

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 48+122
 Co-ordinate : E:- 755031.57 N:- 3211329.88
 Reduce Level: 224.318

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 30.00 m

Borehole No. 1
 Page 2 of 2
 Date : 23-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification (mm)	Sample		SPT N - value		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil											
					Depth (m)	Type	(Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	C _c	P _c	Cr			c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)								
21		Hard Silty CLAY		CL	20.00-20.50	UDS-7			28	18	10	1	2	64	33	18.6	2.03	1.71							129														
22					21.00-21.45	SPT-14	35																																
23		Dense to Very Dense Sandy SILT		ML	22.50-22.95	SPT-15																																	
24					23.00-23.50	UDS-8			28	21	7	3	13	78	6	22.3	2.14	1.75																					
25					24.00-24.45	SPT-16	34	20																															
26					25.50-25.95	SPT-17	32	19																															
27					27.00-27.45	SPT-18	43	22																															
28																																							
29																																							
30					30.00-30.45	SPT-20	65	28																															
		Borehole terminated at depth of 30.45m																																					
31																																							
32																																							
33																																							
34																																							
35																																							
36																																							
37																																							
38																																							
39																																							
40																																							

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 1 of 2



Location : CH 48+400
Co-ordinate: E:- 754976.25 N:- 3211606.75

Borehole dia : 150 mm

Ground Water Table : 4.00 m

Termination Depth : 30.45 m

Date : 21/2/2016

Reduce Level: 225.969

Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT N-value (Obs.)	SPT N-value (Corr.)	Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil													
					Depth (m)	Type			LL	PL	PI	Gravel	Sand	Silt	Clay					e _g	Cc	Pc	Cr			c	φ (o)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)										
1		Firm Silty CLAY		CL	0.50-1.00	DS-1																																			
2	1.50-1.95				SPT-1	5																																			
3	2.00-2.50				UDS-1					34	22	12	0	12	76	12	16.3	1.7	1.46	2.63								51								Nil	89.36	7.15			
4		Stiff to Very Stiff Silty CLAY		CL	3.00-3.45	SPT-2																																			
5					4.50-4.95	SPT-3	11																																		
6					5.00-5.50	UDS-2																																			
7					6.00-6.45	SPT-4	16																																		
8					7.50-7.95	SPT-5	12																																		
9					8.00-8.50	UDS-3											17.5	1.85	1.58																						
10		Very Stiff Silty CLAY		CL	9.00-9.45	SPT-6																																			
11					10.50-10.95	SPT-7	28																																		
12					11.00-11.50	UDS-4																																			
13					12.00-12.45	SPT-8	31																																		
14					13.50-13.95	SPT-9	35																																		
15		Very Stiff to Hard Silty CLAY		CI	14.00-14.50	UDS-5					2	7	74	17	26	2.04	1.62									155															
16					15.00-15.45	SPT-10	26																																		
17					16.50-16.95	SPT-11	32																																		
18					17.00-17.50	UDS-6				43	21	22	5	7	68	20	28.5	1.94	1.51																						
19					18.00-18.45	SPT-12	45																																		
20					19.50-19.95	SPT-13	31																																		

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Shell & Auger

Borehole No. 1

Page 2 of 2

Location : CH 48+400
 Co-ordinate : E:- 754976.25 N:- 3211606.75
 Reduce Level: 225.969

Borehole dia : 150 mm
 Ground Water Table : 4.00 m
 Termination Depth : 30.45 m

Date : 21/2/2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)			Water Content (%)	Bulk Density (Mg/m ³)	Dry Density (Mg/m ³)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	N - value (Obs.)	N - value (Corr.)	LL	PL	PI	Gravel	Sand	Silt					Clay	e ₀	Cc	Pc			Cr	c (kPa)	φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)												
																																Gravel	Sand	Silt	Clay	c	φ						
21		Hard Silty CLAY		CI	20.00-20.50	UDS-7									20.7	2.02	1.68																										
22					21.00-21.45	SPT-14	38																																				
23					22.50-22.95	SPT-15	39																																				
24					23.00-23.50	UDS-8		39	20	19	0	6	77	17	30.5	1.99	1.53																										
25					24.00-24.45	SPT-16	41																																				
26					25.50-25.95	SPT-17	36	20																																			
27					Dense to Very Dense Fine SAND		SW-SM	25.00-25.50	UDS-9																																		
28								27.00-27.45	SPT-18	43	22																																
29								28.50-28.95	SPT-19	49	23			1	93	6												0	34.1														
30								30.00-30.45	SPT-20	58	26																																
31		Borehole terminated at depth of 30.45m																																									
32																																											
33																																											
34																																											
35																																											
36																																											
37																																											
38																																											
39																																											
40																																											

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 48+510
 Co-ordinate : E:-754951.06 N:- 3211709.45
 Reduce Level: 226.193

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 30.00 m

Borehole No. 1
 Page 2 of 2
 Date : 23-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil							
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	C _c	P _c	Cr			c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)				
21		Dense to Very Dense Sandy SILT		ML	20.00-20.50	UDS-7			28		NP	1	16	74	9	22.4	2.01	1.64	2.65							0	32								
22					21.00-21.45	SPT-14	49	26																											
23					22.50-22.95	SPT-15	54	27																											
24					23.00-23.50	UDS-8			27		NP			3	17	74	6	21.8	2.04	1.67															
25					24.00-24.45	SPT-16	47	24																											
26		25.50-25.95	SPT-17	52	25																														
27		Very Dense Fine SAND		SP	27.00-27.45	SPT-18	51	25				0	98	2											0	33.3									
28					28.50-28.95	SPT-19	55	25																											
29					30.00-30.45	SPT-20	63	27																											
30					Borehole terminated at depth of 30.45m																														
31																																			
32																																			
33																																			
34																																			
35																																			
36																																			
37																																			
38																																			
39																																			
40																																			

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 1 of 2

Location : CH 55+850

Borehole dia : 150 mm

Co-ordinate : E:- 757566.43 N:- 3218233.90

Ground Water Table : 12.00 m

Reduce Level: 225.699

Termination Depth : 30.45 m

Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																								
					Depth (m)	Type	N - value	LL	PL	PI	Gravel	Sand	Silt	Clay	e ₀					Cc	Pc	Cr	c (kPa)			φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)																						
																															(Obs.)	(Corr.)	(Obs.)	(Corr.)																		
1					0.50-1.00	DS-1																																														
2				ML	1.50-1.95	SPT-1	16	21																																												
3				ML	2.00-2.50	UDS-1			31		NP		0	25	63	12	11.4	1.83	1.65							0		30																								
4		Medium Dense Sandy SILT			3.00-3.45	SPT-2	12	17																																												
5					4.50-4.95	SPT-3	18	19																																												
6					6.00-6.45	SPT-4	21	20																																												
7																																																				
8				SW-SM	7.50-7.95	SPT-5	23	20				0	92		8			2.63																																		
9					9.00-9.45	SPT-6	29	23																																												
10		Medium Dense Fine SAND			10.50-10.95	SPT-7	23	19																																												
11					12.00-12.45	SPT-8	30	22																																												
12																																																				
13																																																				
14				SP	13.50-13.95	SPT-9	35	23				0	95		5												0		35.6																							
15					15.00-15.45	SPT-10	45	27																																												
16																																																				
17		Dense Fine SAND			16.50-16.95	SPT-11	30	20																																												
18					18.00-18.45	SPT-12	34	21																																												
19																																																				
20				SP	19.50-19.95	SPT-13	38	22				0	96		4																																					

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 55+850
 Co-ordinate : E:- 757566.43 N:- 3218233.90
 Reduce Level: 225.699

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 12.00 m
 Termination Depth : 30.45 m
 Borehole No. 1
 Page 2 of 2
 Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																									
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																						
																																Direct Shear		Chemical Analysis of soil																			
21		Dense to Very Dense Fine SAND			21.00-21.45	SPT-14	39	22																																													
22					22.50-22.95	SPT-15	41	22																																													
23					24.00-24.45	SPT-16	44	23																																													
24					25.50-25.95	SPT-17	56	27																																													
25					27.00-27.45	SPT-18	54	26						0	96		4										0	36.5																									
26					28.50-28.95	SPT-19	60	27																																													
27					30.00-30.45	SPT-20	68	29																																													
28					Borehole terminated at depth of 30.45m																																																
29																																																					
30																																																					
31																																																					
32																																																					
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36																																																					
37																																																					
38																																																					
39																																																					
40																																																					

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 1 of 2

Location : CH 56+780

Borehole dia : 150 mm

Co-ordinate: E:- 758055.09 N:- 3219023.09

Ground Water Table : 7.00 m

Reduce Level: 228.358

Termination Depth : 30.45 m

Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification Code	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil														
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	c (kPa)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)									
1						0.50-1.00	DS-1																																			
2				ML		1.50-1.95	SPT-1	6	10				11	14	64	11																										
3						3.00-3.45	SPT-2	13	18																																	
4		Loose to Medium Dense Sandy SILT				4.50-4.95	SPT-3	17	18																																	
5						6.00-6.45	SPT-4	19	19																																	
6																																										
7																																										
8				SP		7.50-7.95	SPT-5	20	18				0	97	3											0		32.7														
9						9.00-9.45	SPT-6	22	19																																	
10																																										
11		Medium Dense Fine SAND				10.50-10.95	SPT-7	26	20																																	
12						12.00-12.45	SPT-8	27	20																																	
13																																										
14						13.50-13.95	SPT-9	30	21																																	
15				ML		14.00-14.50	UDS-5			29		NP	3	6	82	9	23.6	2.18	1.76																							
16						15.00-15.45	SPT-10	34	22																																	
17						16.50-16.95	SPT-11	37	23																																	
18		Dense to Very Dense Sandy SILT				18.00-18.45	SPT-12	42	24																																	
19																																										
20						19.50-19.95	SPT-13	52	28																																	

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 56+780
 Co-ordinate : E:- 758055.09 N:- 3219023.09
 Reduce Level: 228.358

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 7.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 2 of 2
 Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)													
21		Vey Dense Fine SAND		SW-SM	21.00-21.45	SPT-14	57	29				1	92	7																														
22																																												
23								22.50-22.95	SPT-15	62	30																																	
24								24.00-24.45	SPT-16	49	25																																	
25								25.50-25.95	SPT-17	65	30																																	
26								27.00-27.45	SPT-18	62	28				1	98	1												0	33.3														
27								28.50-28.95	SPT-19	69	30																																	
28								30.00-30.45	SPT-20	75	31																																	
29					Borehole terminated at depth of 30.45m																																							
30																																												
31																																												
32																																												
33																																												
34																																												
35																																												
36																																												
37																																												
38																																												
39																																												
40																																												

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

Method of Boring : Rotary

Borehole No. 1

Page 1 of 2

Location : CH 57+555

Borehole dia : 150 mm

Co-ordinate: E:- 758457.09 N:- 3219680.76

Ground Water Table : 9.00 m

Reduce Level: 228.935

Termination Depth : 30.45 m

Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification (mm)	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																					
					Depth (m)	Type	N - value (Obs.)	N - value (Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	c (kPa)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)																
																																		0	0	0	0												
1					0.50-1.00	DS-1																																											
2					1.50-1.95	SPT-1	12	20																																									
				ML	2.00-2.50	UDS-1			29		NP		2	17	72	9	16.3	2.1	1.81																														
3					3.00-3.45	SPT-2	13	18																																									
4																																																	
5		Medium Dense Sandy SILT			4.50-4.95	SPT-3	17	18																																									
6				ML	5.00-5.50	UDS-2			28		NP		1	12	86	1	15.6	1.74	1.51								0																						
7					6.00-6.45	SPT-4	13	15																																									
8					7.50-7.95	SPT-5	18	20																																									
9																																																	
10				SW-SM	9.00-9.45	SPT-6	33	25				0	92		8				2.64																														
11					10.50-10.95	SPT-7	38	26																																									
12					12.00-12.45	SPT-8	38	25																																									
13					13.50-13.95	SPT-9	41	26																																									
14																																																	
15		Medium Dense to Dense Fine SAND			15.00-15.45	SPT-10	42	26				0	95		5																																		
16				SW-SM	16.50-16.95	SPT-11	41	25																																									
17					18.00-18.45	SPT-12	52	28																																									
18					19.50-19.95	SPT-13	54	29																																									

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 57+555
 Co-ordinate: E:- 758457.09 N:- 3219680.76
 Reduce Level: 228.935

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 9.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 2 of 2
 Date : 26-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)									
21		Dense to Very Dense Fine SAND	[Pattern]	SW-SM	21.00-21.45	SPT-14	55	28				1	93	6			2.66																							
22																																								
23								22.50-22.95	SPT-15	59	29																													
24								24.00-24.45	SPT-16	57	28																													
25								25.50-25.95	SPT-17	52	25																													
26																																								
27																																								
28				SW-SM	27.00-27.45	SPT-18	59	27				0	95	5											0	32.5														
29					28.50-28.95	SPT-19	63	28																																
30					30.00-30.45	SPT-20	76	32																																
Borehole terminated at depth of 30.45m																																								
31																																								
32																																								
33																																								
34																																								
35																																								
36																																								
37																																								
38																																								
39																																								
40																																								

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 58+400
 Co-ordinate: E:- 758903.29 N:- 3220405.84
 Reduce Level: 226.841

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 10.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 1 of 2
 Date : 27-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification (code)	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil												
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)									
1		Very Stiff Silty CLAY		CL	0.50-1.00	DS-1																																		
2					1.50-1.95	SPT-1	15																																	
3					2.00-2.50	UDS-1				32		20	12	2	8	74	16	17.7	2.18	1.85																				
4					3.00-3.45	SPT-2	17																																	
5					4.50-4.95	SPT-3	16																																	
6					6.00-6.45	SPT-4	18																																	
7																																								
8		Medium Dense Silty SAND		SM	7.50-7.95	SPT-5	21	19				0	64	36				2.62																						
9					9.00-9.45	SPT-6	22	19																																
10																																								
11		Medium Dense to Dense Sandy SILT		ML	10.50-10.95	SPT-7	26	20																																
12					11.00-11.50	UDS-4			28		NP		1	15	76	8	19.7	2.09	1.75							0	31.1													
13					12.00-12.45	SPT-8	28	21																																
14					13.50-13.95	SPT-9	30	21																																
15					14.00-14.50	UDS-5			33		NP		2	20	66	12	18.7	2.07	1.74																					
16					15.00-15.45	SPT-10	32	21																																
17					16.50-16.95	SPT-11	34	22																																
18		18.00-18.45	SPT-12	35	22																																			
19																																								
20																																								

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 58+400
 Co-ordinate : E:- 758903.29 N:- 3220405.84
 Reduce Level: 226.841

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 10.00 m
 Termination Depth : 30.45 m

Borehole No. 1
 Page 2 of 2
 Date : 27-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil																
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	Φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)													
21		Dense to Very Dense Fine SAND		SP	21.00-21.45	SPT-14	40	23				0	96	4				2.65																										
22																																												
23								22.50-22.95	SPT-15	42	23																																	
24																																												
25								24.00-24.45	SPT-16	44	23																																	
26																																												
27																																												
28							SP	27.00-27.45	SPT-18	55	26				0	96	4											0	35.7															
29								28.50-28.95	SPT-19	65	29																																	
30								30.00-30.45	SPT-20	65	28																																	
Borehole terminated at depth of 30.45m																																												
31																																												
32																																												
33																																												
34																																												
35																																												
36																																												
37																																												
38																																												
39																																												
40																																												

Prepared By: _____ Checked By: _____

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 59+305
 Co-ordinate : E:- 759335.53 N:- 3221198.37
 Reduce Level: 227.556

Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 10.00 m
 Termination Depth : 12.00 m
 Borehole No. 1
 Page 1 of 1
 Date : 29-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil														
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	Cc	Pc	Cr			c (kPa)	c (kPa)	c (kPa)	Φ (°)	Sulphates mg/l	Chlorides mg/l	pH Value	Organic (%)									
1		Stiff Silty CLAY		CL	0.50-1.00	DS-1																																				
2					1.50-1.95	SPT-1	8																																			
3					2.00-2.50	UDS-1				28	19	9	0	17	71	12	21.7	2.06	1.69																							
4					3.00-3.45	SPT-2	12																																			
5		Medium Dense Silty SAND		SM	4.50-4.95	SPT-3	18	19			0	71	29																													
6					6.00-6.45	SPT-4	24	22																																		
7					7.50-7.95	SPT-5	31	25																																		
8		Hard Silty CLAY		CI	8.00-8.50	UDS-3			48	28	20	0	2	65	33	38.8	1.88	1.35	2.63																							
9					9.00-9.45	SPT-6	33																																			
10					10.50-10.95	SPT-7	31																																			
11					11.00-11.50	UDS-4				31	20	11	0	35	56	9	12.7	1.91	1.7																							
12		12.00-12.45	SPT-8	26																																						
13		Borehole terminated at depth of 12.45m																																								
14																																										
15																																										
16																																										
17																																										
18																																										
19																																										
20																																										

Prepared By:

Checked By:

BORE LOG

Project : Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut
 Location : CH 63+570
 Co-ordinate: E:- 761700.97 N:- 3224570.96
 Reduce Level: 228.732

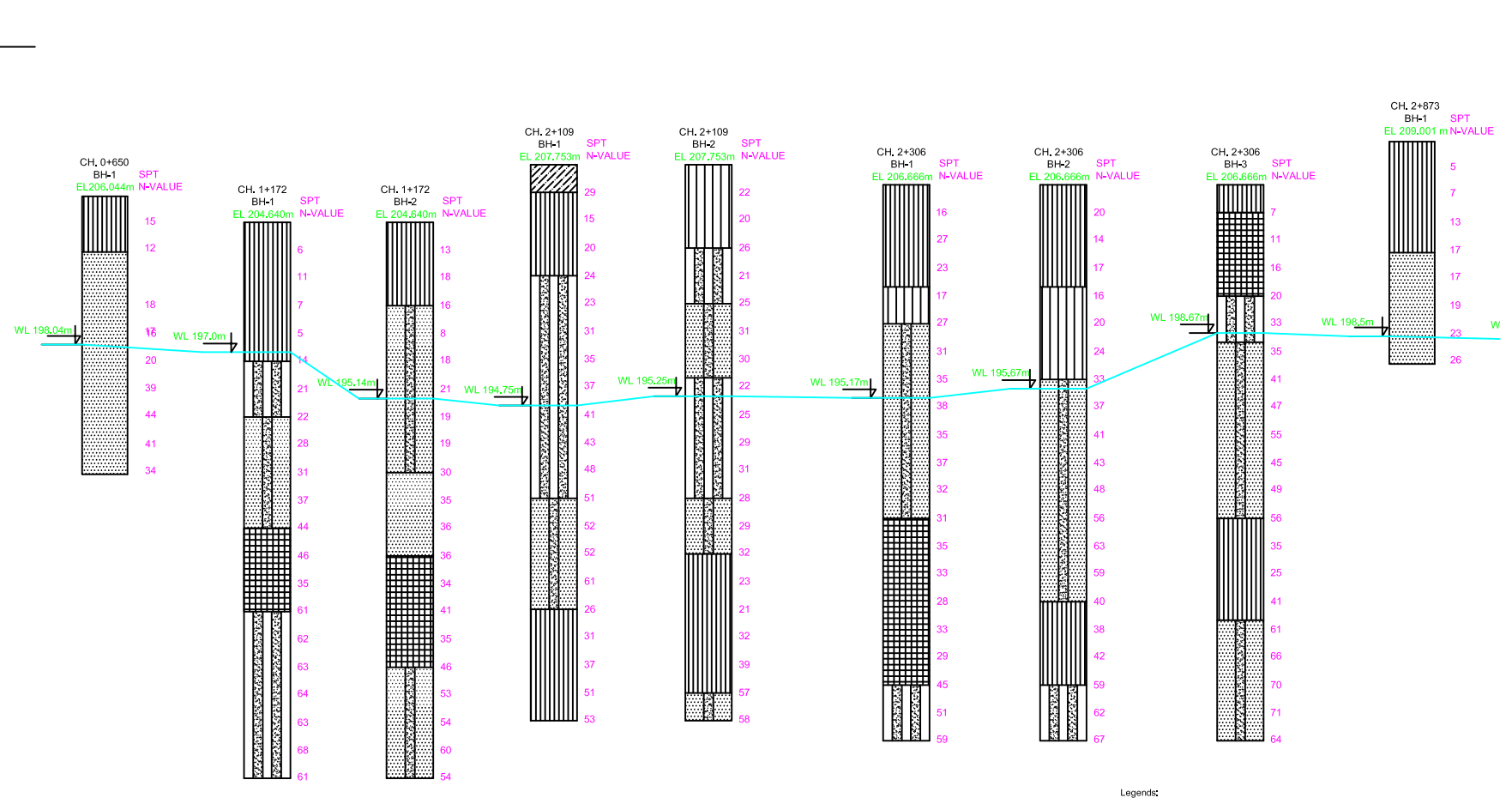
Method of Boring : Rotary
 Borehole dia : 150 mm
 Ground Water Table : 7.00 m
 Termination Depth : 30.45 m
 Borehole No. 1
 Page 1 of 2
 Date : 29-01-2016



Depth (m)	R.L. (m)	Strata Description	Legend	IS Classification (ref)	Sample		SPT		Atterberg Limit (%)			Grain size Analysis (%)				Water Content (%)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Specific Gravity	Consolidation Test				UU c (kPa)	UC c (kPa)	Direct Shear		Chemical Analysis of soil														
					Depth (m)	Type	N - value (Obs.)	(Corr.)	LL	PL	PI	Gravel	Sand	Silt	Clay					e ₀	C _c	P _c	Cr			c (kPa)	φ (°)	Sulphates (mg/l)	Chlorides (mg/l)	pH Value	Organic (%)											
1		Firm Silty CLAY	[Cross-hatched pattern]	CL	0.50-1.00	DS-1																																				
2					1.50-1.95	SPT-1	5																																			
3					2.00-2.50	UDS-1					28	18	10	0	36	49	15	21	2.06	1.7						46																
4					3.00-3.45	SPT-2	6																																			
5		Very Stiff Silty CLAY	[Cross-hatched pattern]	CL	4.50-4.95	SPT-3	18																																			
6					6.00-6.45	SPT-4	17																																			
7					7.50-7.95	SPT-5	21	19						0	97	3				2.65						0	32.0															
8		Medium Dense Fine SAND	[Dotted pattern]	SP	9.00-9.45	SPT-6	25	20																																		
9					10.50-10.95	SPT-7	26	20																																		
10					12.00-12.45	SPT-8	28	21																																		
11					13.50-13.95	SPT-9	29	21						0	93	4																										
12					15.00-15.45	SPT-10	31	21																																		
13		Dense Fine SAND	[Dotted pattern]	SP	16.50-16.95	SPT-11	33	21																																		
14					18.00-18.45	SPT-12	35	22																																		
15					19.50-19.95	SPT-13	40	23						0	99	1				2.61																						
16																																										
17																																										
18																																										
19																																										
20																																										

Prepared By: _____ Checked By: _____

Reduced Level (m)



Legends:

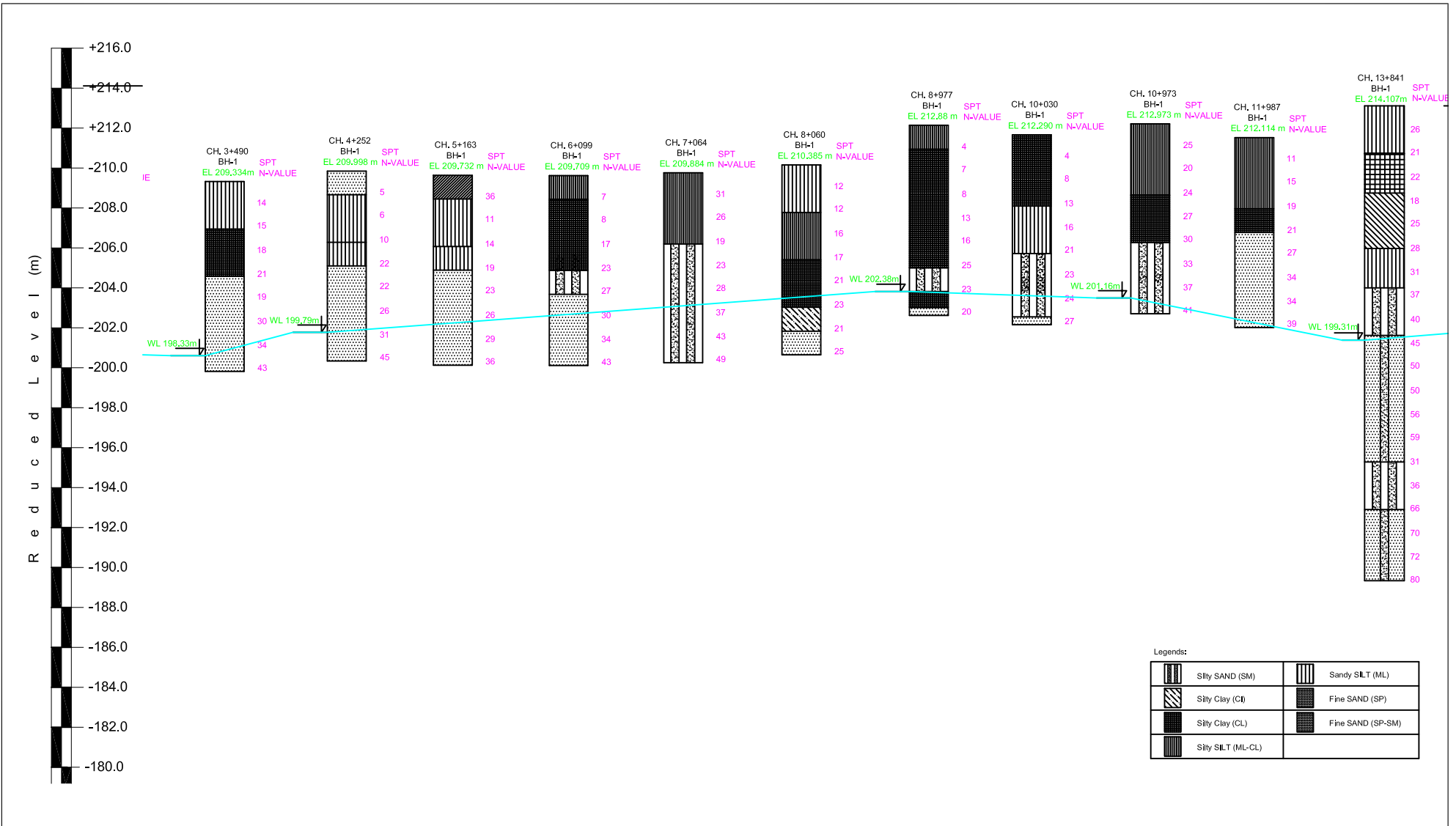
CLIENT: 



PROJECT: Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

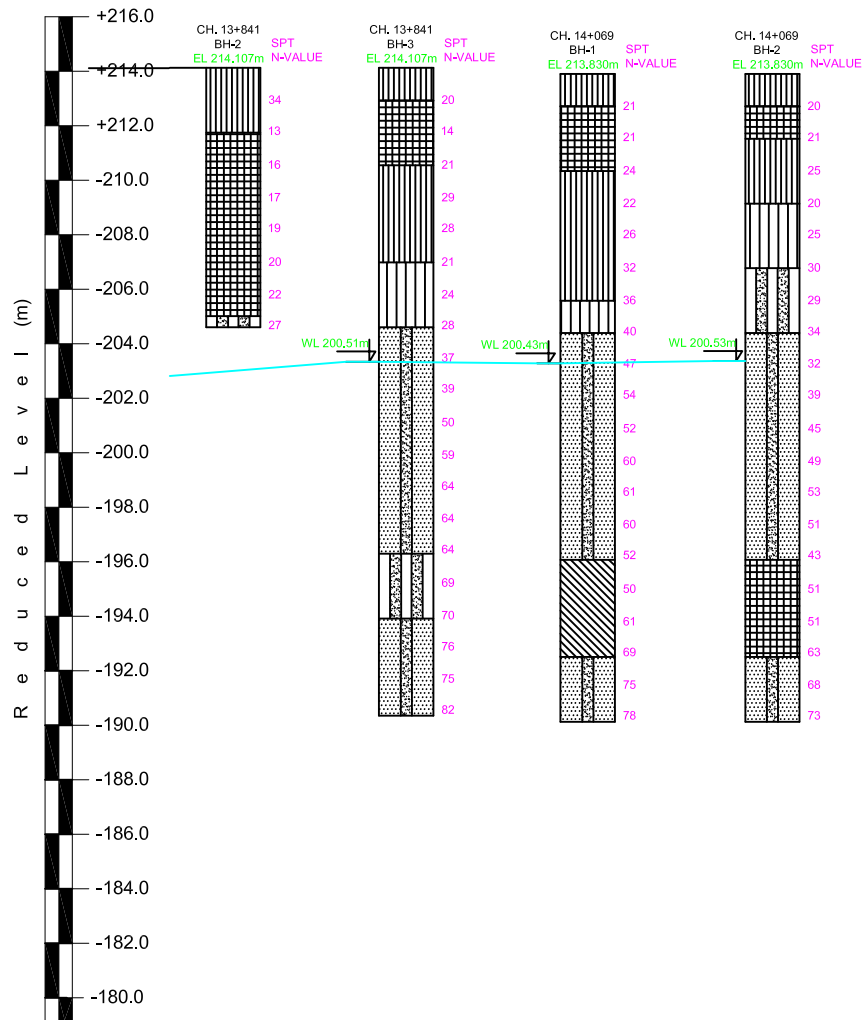
DRAWING TITLE: Soil Profile

AGENCY:  Xplorer Consultancy Services Pvt. Ltd. Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau, Gurgaon-122001, Haryana, India Tel: +91-124-4388659, Fax: +91-124-4241962 Email: xplorer@xplorer.in, Website: www.xplorer.in

DRG. NO. PWD/PROFILE/01
January, 2016



CLIENT: 	PROJECT: Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut	DRAWING TITLE: Soil Profile	AGENCY:  Xplorer Consultancy Services Pvt.Ltd. Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau, Gurgaon-122001, Haryana, India Tel: +91-124-4388659, Fax: +91-124-4241962 Email: xplorer@xplorer.in, Website: www.xplorer.in	DRG. NO. PWD/PROFILE/01 January, 2016
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Legends:

	Silty SAND (SM)		Sandy SILT (ML)
	Silty Clay (CI)		Fine SAND (SP)
	Silty Clay (CL)		Fine SAND (SP-SM)
	Silty SILT (ML-CL)		

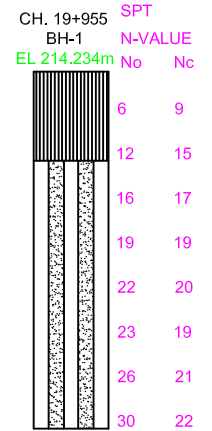
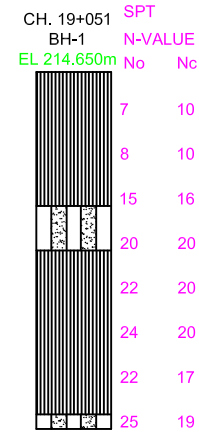
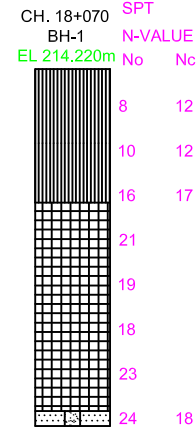
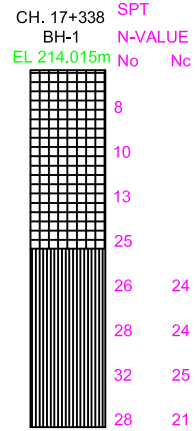
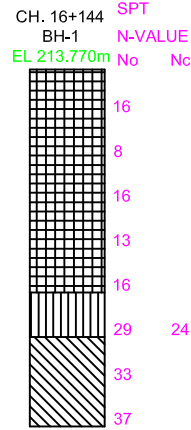
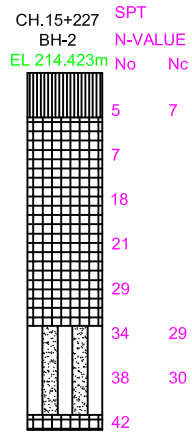
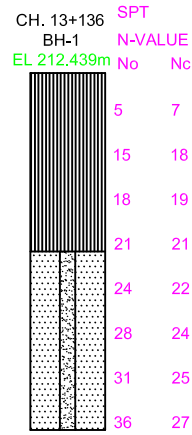
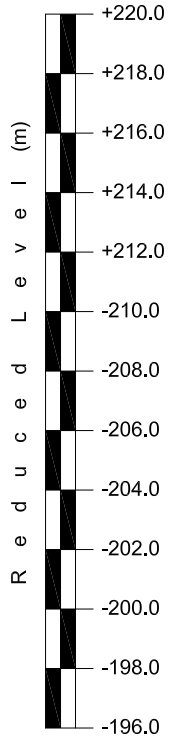
CLIENT: 

PROJECT: Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut

DRAWING TITLE: Soil Profile

AGENCY:  Xplorer Consultancy Services Pvt. Ltd.
 Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau, Gurgaon-122001, Haryana, India
 Tel: +91-124-4388659, Fax: +91-124-4241962
 Email: xplorer@xplorer.in, Website: www.xplorer.in

DRG. NO. PWD/PROFILE/01
 January, 2016



Legends:

	Silty SAND (SM)		Sandy SILT (ML)
	Silty Clay (CI)		Fine SAND (SP)
	Silty Clay (CL)		Fine SAND (SP-SM)
	Silty SILT (ML-CL)		

CLIENT:



PROJECT:

Geotechnical Investigation Works at Hapur-Meerut
Section of DFCC Meerut

DRAWING TITLE:

Soil Profile

AGENCY:

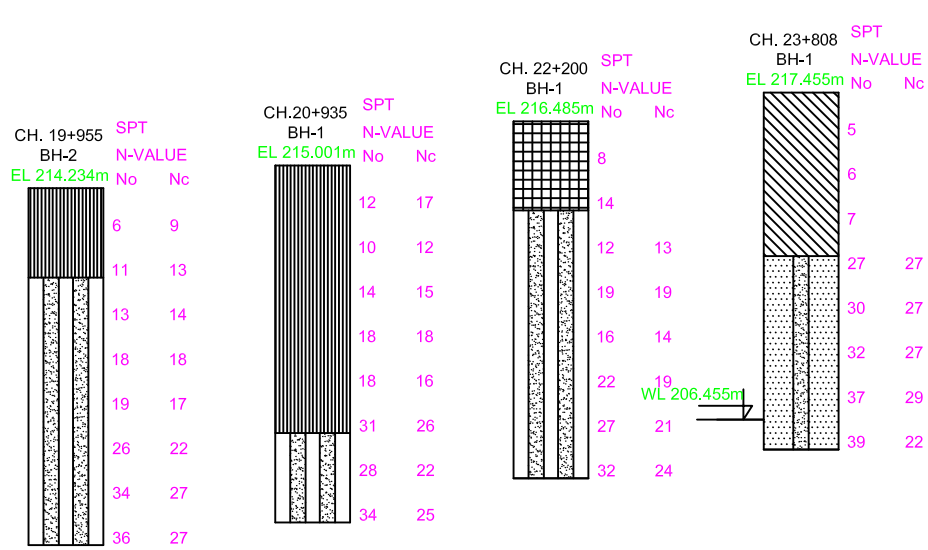


Xplorer Consultancy Services Pvt. Ltd.
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Email: xplorer@xplorer.in, Website: www.xplorer.in

DRG. NO. PWD/PROFILE/01

March, 2016

R e d u c e d L e v e l (m)



Legends:

	Silty SAND (SM)		Sandy SILT (ML)
	Silty Clay (CI)		Fine SAND (SP)
	Silty Clay (CL)		Fine SAND (SP-SM)
	Silty SILT (ML-CL)		

CLIENT:



PROJECT:

Geotechnical Investigation Works at Hapur-Meerut
 Section of DFCC Meerut

DRAWING TITLE:

Soil Profile

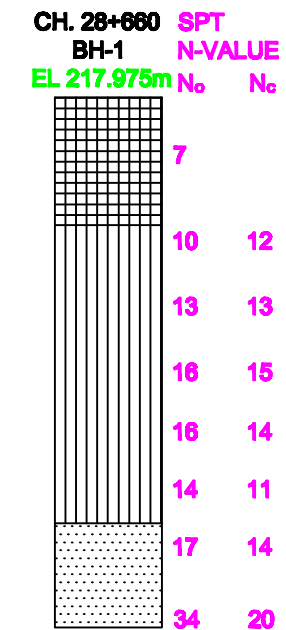
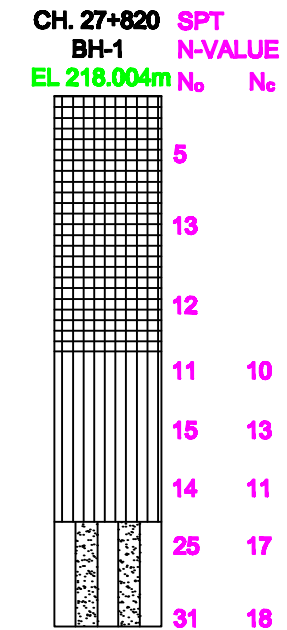
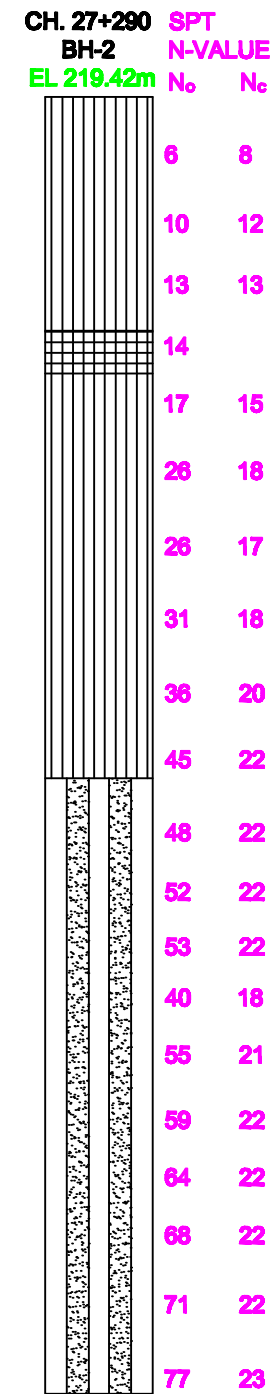
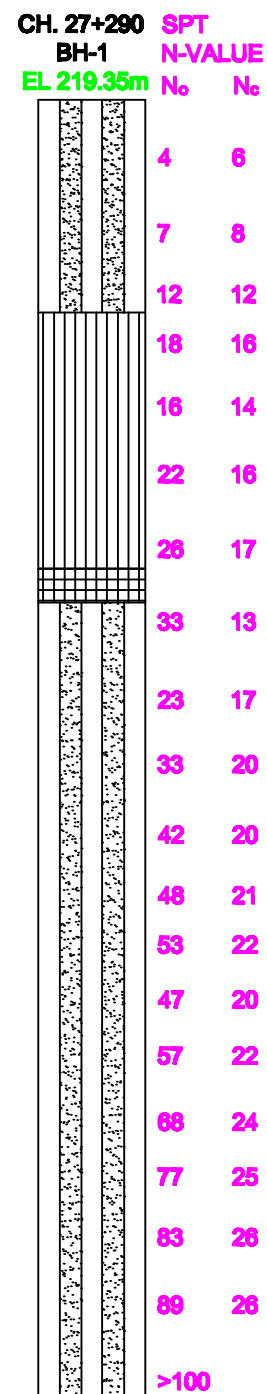
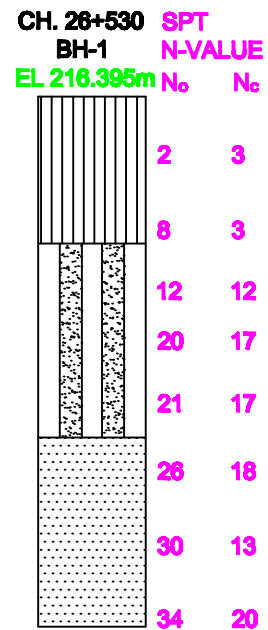
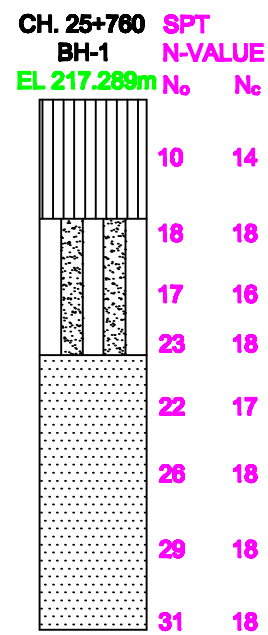
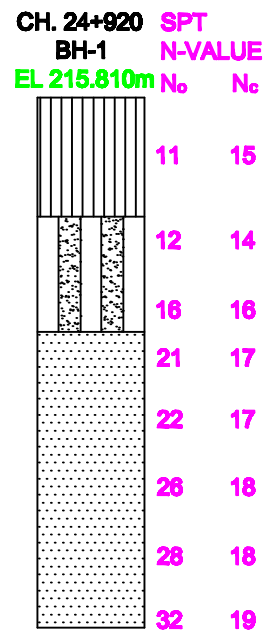
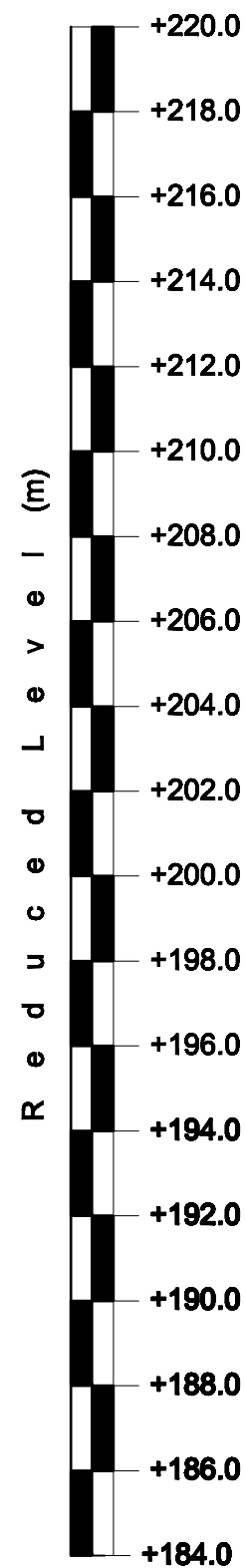
AGENCY:



Xplorer Consultancy Services Pvt. Ltd.
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DRG. NO. PWD/PROFILE/01

March, 2016



Legends:

	Silty SAND (SM)
	Poorly Graded Sand (SP)
	Poorly Graded Gravel (GP)
	Silty Clay (CI)
	Silty Clay (CL)
	Silty SILT (ML)

CLIENT:



PROJECT:

Geotechnical Investigation Works at Khurja-Dadri
Section of DFCC Meerut

DRAWING TITLE:

Soil Profile

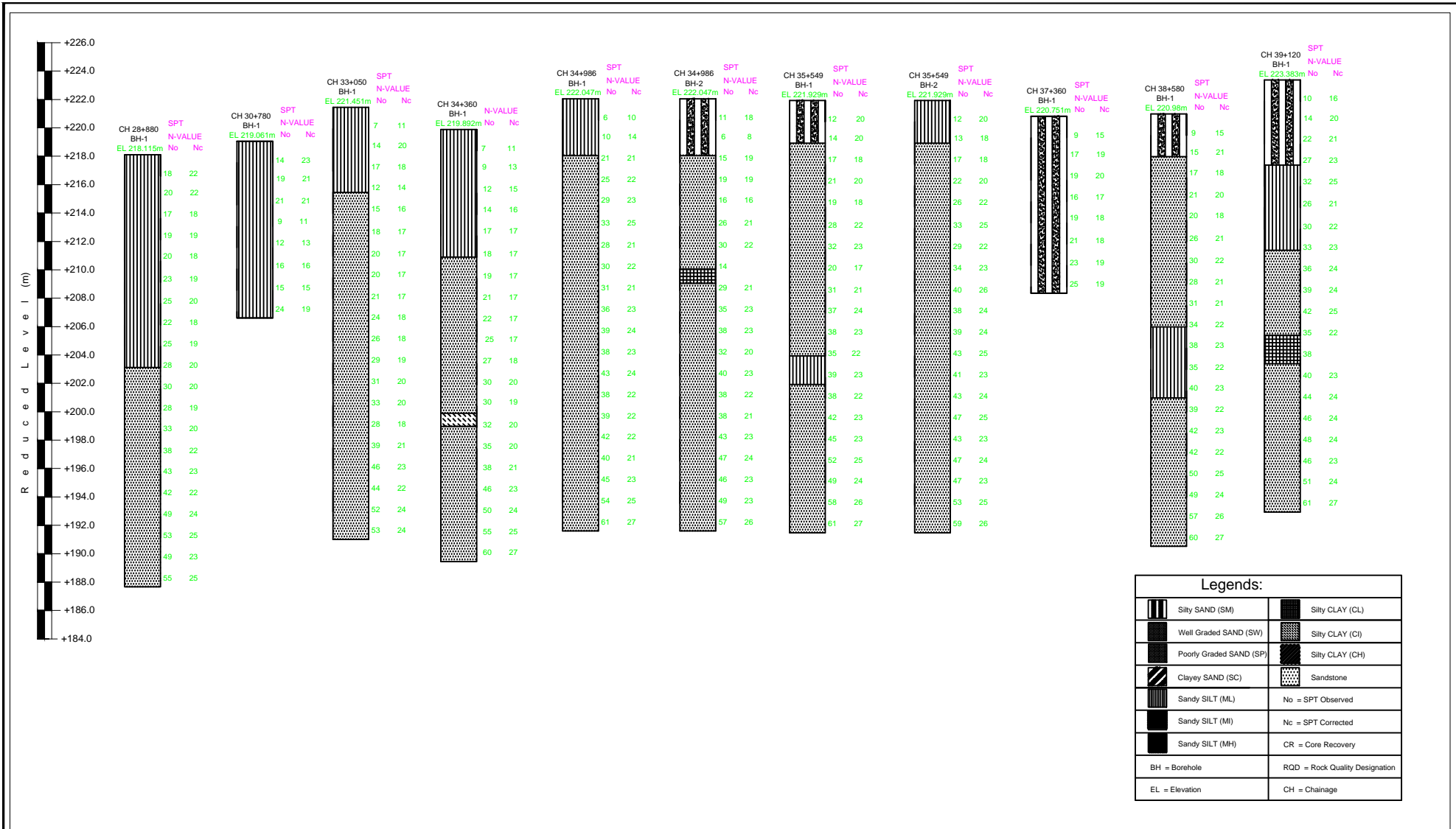
AGENCY:



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Email: xplorer@xplorer.in, Website: www.xplorer.in

DRG. NO. PWD/PROFILE/01

July, 2015



Legends:

	Silty SAND (SM)		Silty CLAY (CL)
	Well Graded SAND (SW)		Silty CLAY (CI)
	Poorly Graded SAND (SP)		Silty CLAY (CH)
	Clayey SAND (SC)		Sandstone
	Sandy SILT (ML)	No = SPT Observed	No = SPT Observed
	Sandy SILT (MI)	Nc = SPT Corrected	Nc = SPT Corrected
	Sandy SILT (MH)	CR = Core Recovery	CR = Core Recovery
BH = Borehole		RQD = Rock Quality Designation	RQD = Rock Quality Designation
EL = Elevation		CH = Chainage	CH = Chainage

CLIENT:




skylark
Designer and Engineers (P) Limited

PROJECT:
Geotechnical Investigation Works at Hapur-Meerut
Section of DFCC Meerut

DRAWING TITLE:
Soil Profile

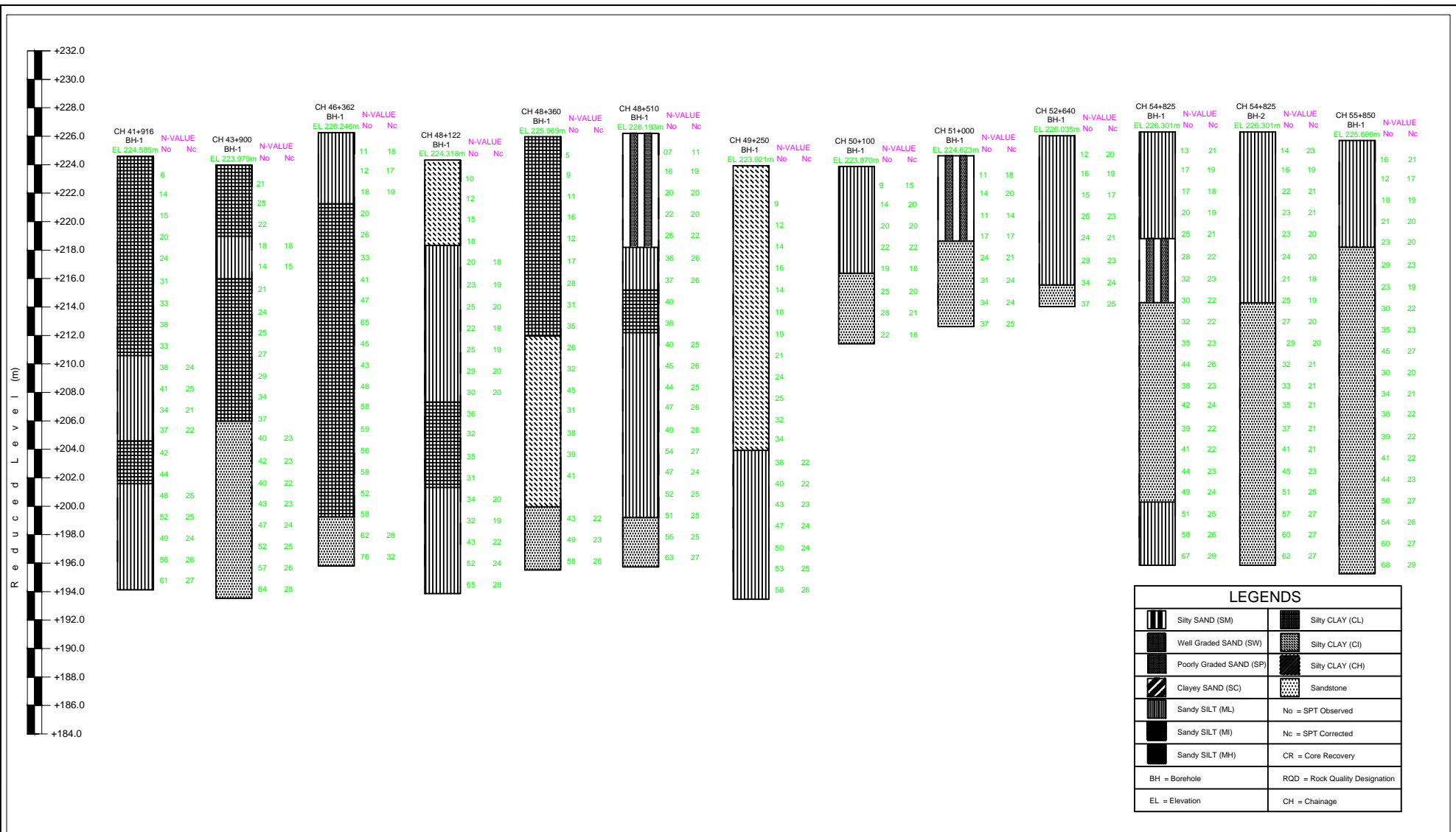
AGENCY:



XPLOER
CONSULTANCY SERVICES PVT. LTD.
INDIAN ENGINEERING DESIGNER

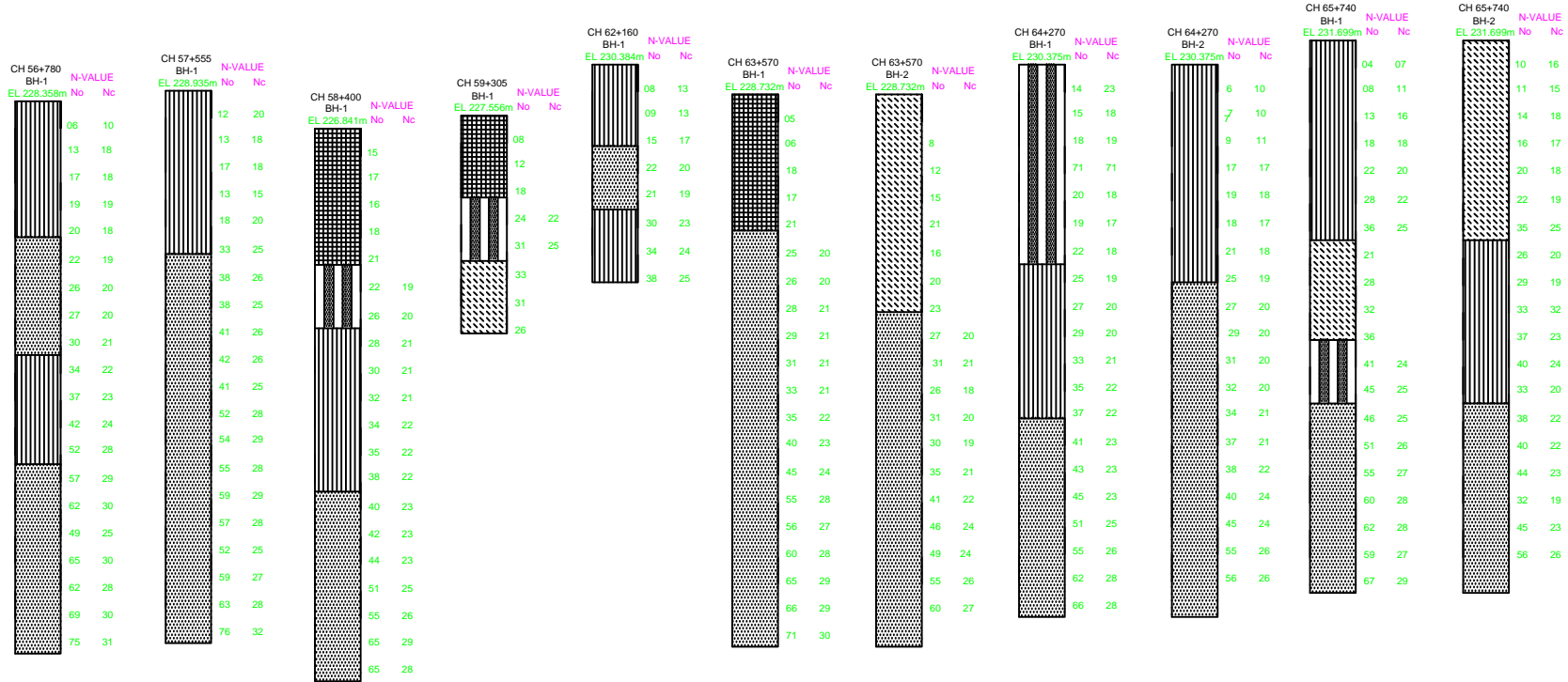
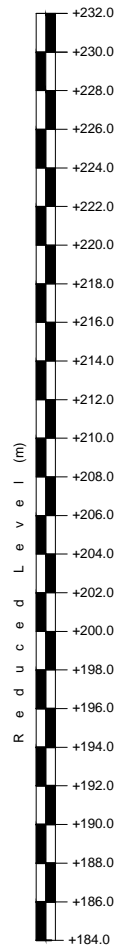
Xplorer Consultancy Services Pvt. Ltd.
Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau,
Gurgaon-122001, Haryana, India
Tel: +91-124-4388659, Fax: +91-124-4241962
Email: xplorer@xplorer.in, Website: www.xplorer.in

DRG. NO. SKYLARK/PROFILE/01
February, 2016



	Silty SAND (SM)		Silty CLAY (CL)
	Well Graded SAND (SW)		Silty CLAY (CI)
	Poorly Graded SAND (SP)		Silty CLAY (CH)
	Clayey SAND (SC)		Sandstone
	Sandy SILT (ML)	No = SPT Observed	No = SPT Observed
	Sandy SILT (MI)	Nc = SPT Corrected	Nc = SPT Corrected
	Sandy SILT (MH)	CR = Core Recovery	CR = Core Recovery
BH = Borehole		RQD = Rock Quality Designation	RQD = Rock Quality Designation
EL = Elevation		CH = Chainage	CH = Chainage

CLIENT: 	PROJECT: Geotechnical Investigation Works at Hapur-Meerut Section of DFCC Meerut	DRAWING TITLE: Soil Profile	AGENCY: Xplorer Consultancy Services Pvt. Ltd. Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau, Gurgaon-122001, Haryana, India Tel: +91-124-4388659, Fax: +91-124-4241962 Email: xplorer@xplorer.in, Website: www.xplorer.in	DRG. NO. SKYLARK/PROFILE/01 March, 2016
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LEGENDS	
	Silty SAND (SM)
	Well Graded SAND (SW)
	Poorly Graded SAND (SP)
	Clayey SAND (SC)
	Sandy SILT (ML)
	Sandy SILT (MI)
	Sandy SILT (MH)
	Silty CLAY (CL)
	Silty CLAY (CI)
	Silty CLAY (CH)
	Sandstone
	No = SPT Observed
	Nc = SPT Corrected
	CR = Core Recovery
	RQD = Rock Quality Designation
	CH = Chainage

CLIENT:

PROJECT:
Geotechnical Investigation Works at Hapur-Meerut
Section of DFCC Meerut

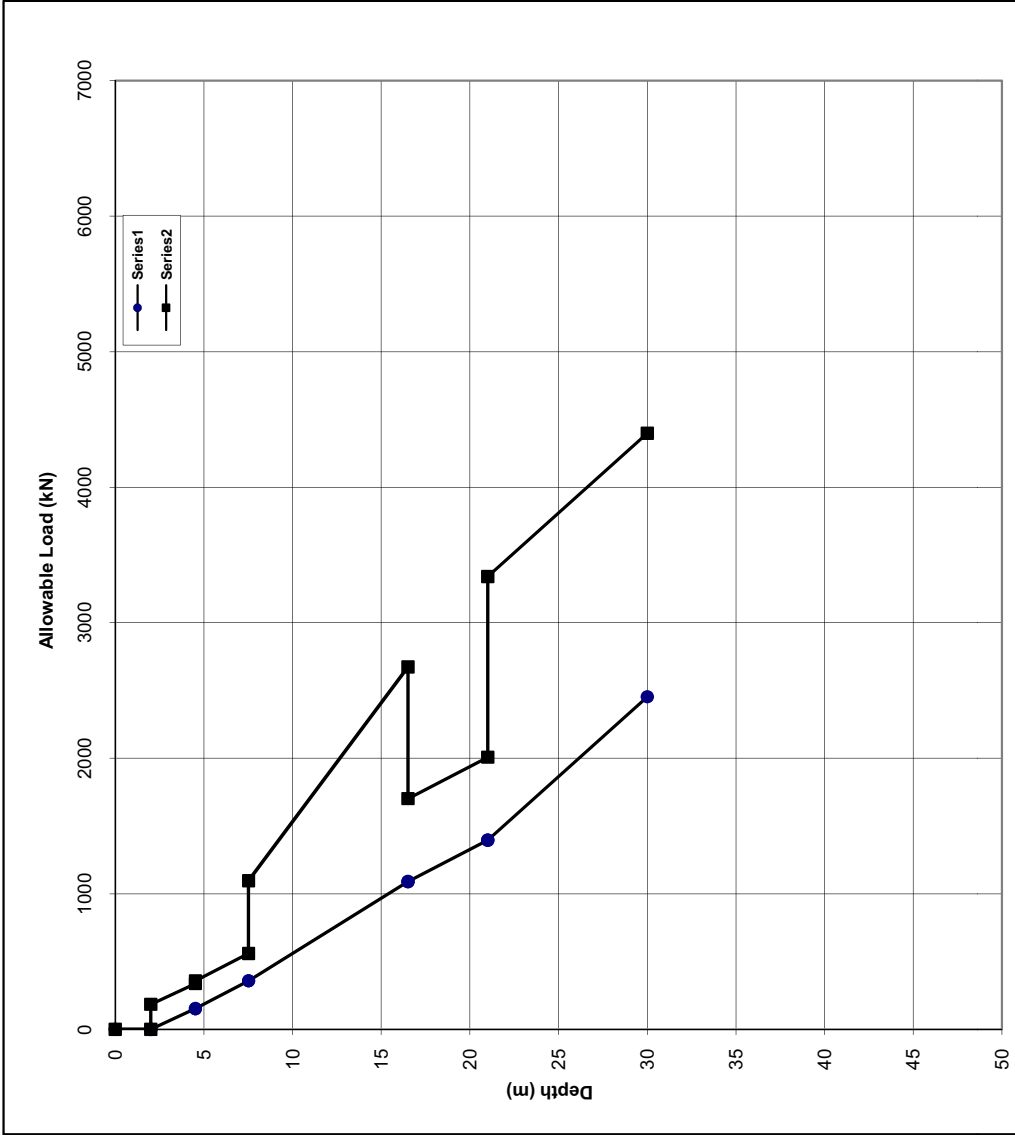
DRAWING TITLE:
Soil Profile

AGENCY:

Xplorer Consultancy Services Pvt. Ltd.
Plot No. 3, First Floor, Sector- 18, Opp. HIPA, Sarhau,
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DRG. NO. SKYLARK/PROFILE/02
March, 2016

ANNEXURE B- BEARING CAPACITY AND PILE CAPACITY CALCULATIONS



Allowable Pile Capacity at Major Bridge Ch. 1+172 km

Pile Type = Bored
 Pile Dia (mm) = 1200
 Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT Pier (Major Bridge Ch.1+172km) (IS: 2911 - PART-1/SEC-2-2010)

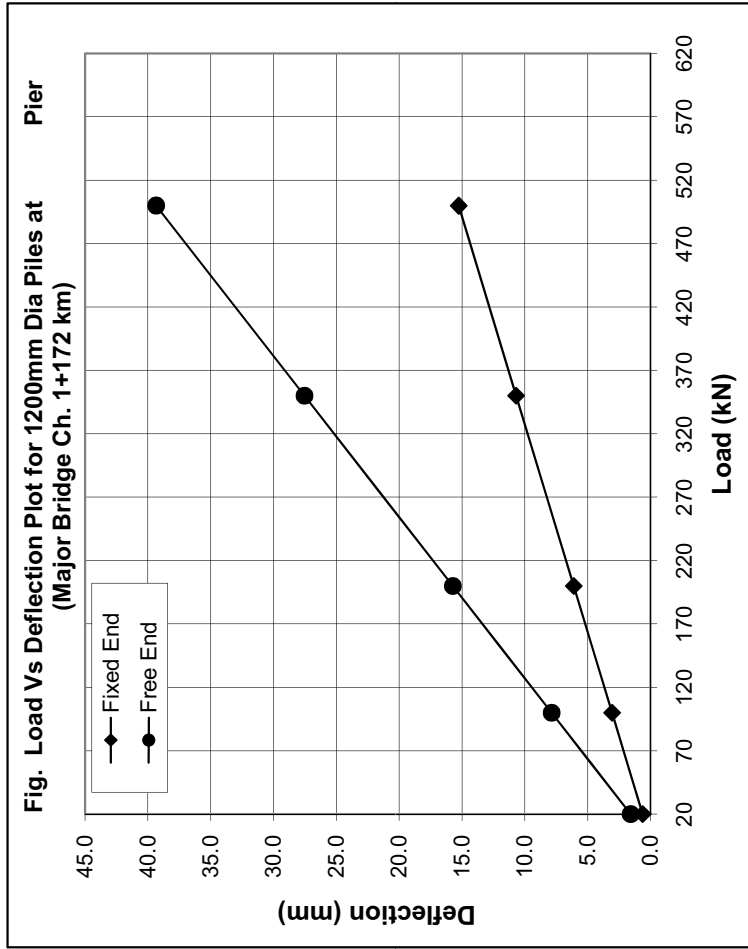
D= 120 cm
 $\eta_p = 0.140 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

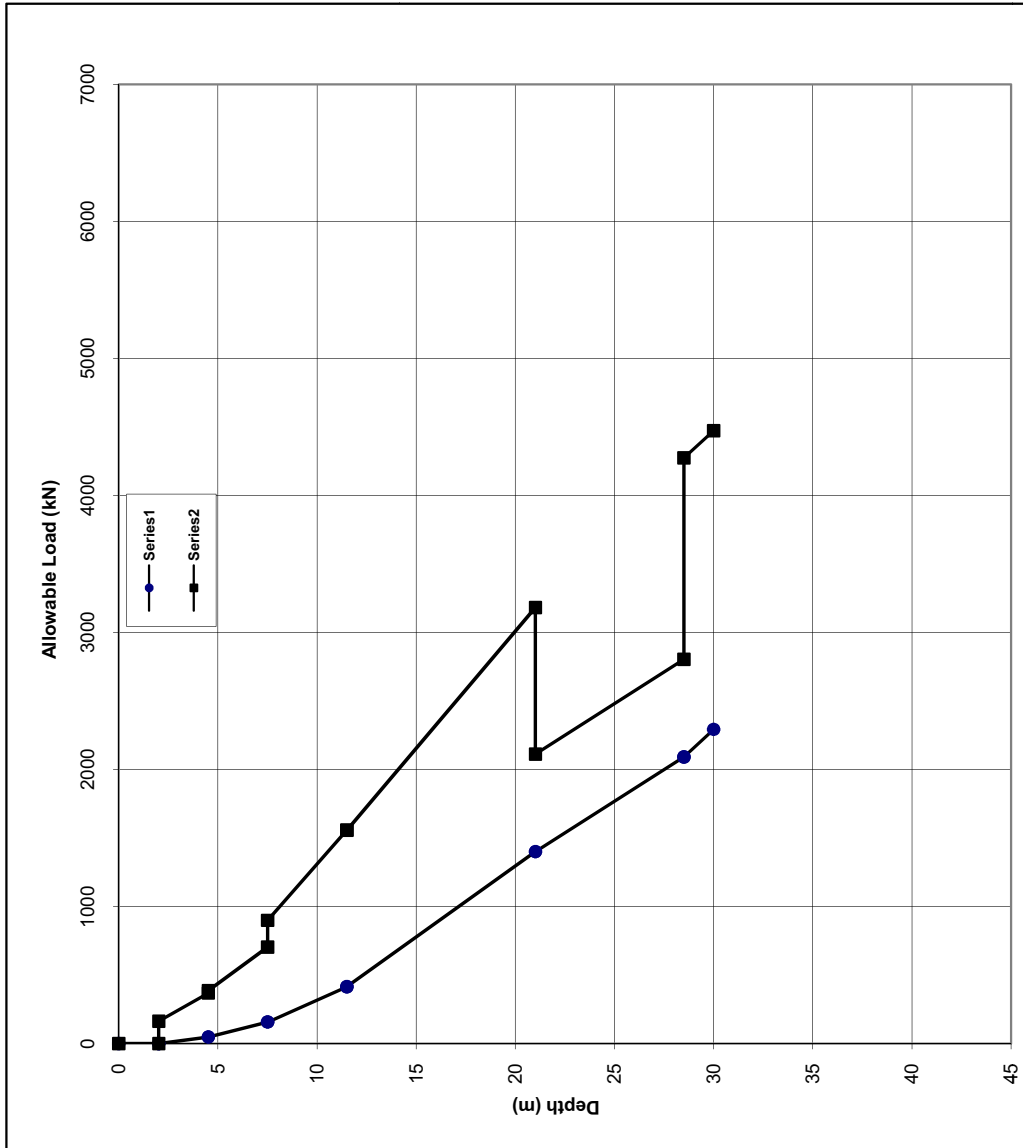
$T = (EI/\eta h)^{0.2} = 455.60$ Fixed
 $L_f/T = 2.2$ cm
 $L_f(\text{Fixed}) = 1002.33$ Free
 $L_f/T = 1.9$ cm
 $L_f(\text{Free}) = 865.64$ cm
 $L_f = 0$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
20	0.61	1.57
100	3.05	7.87
200	6.11	15.74
350	10.69	27.54
500	15.27	39.34

Hence lateral capacity (load corresponding =12mm deflection)

= 380kN (for fixed head condition)
 = 150kN (for free head condition)





Allowable Pile Capacity for Major Bridge, Chainage-2+109 km

Pile Type = Bored
 Pile Dia (mm) = 1200
 Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT Pier (Major Bridge Ch.2+109km) (IS: 2911 - PART-1/SEC-2-2010)

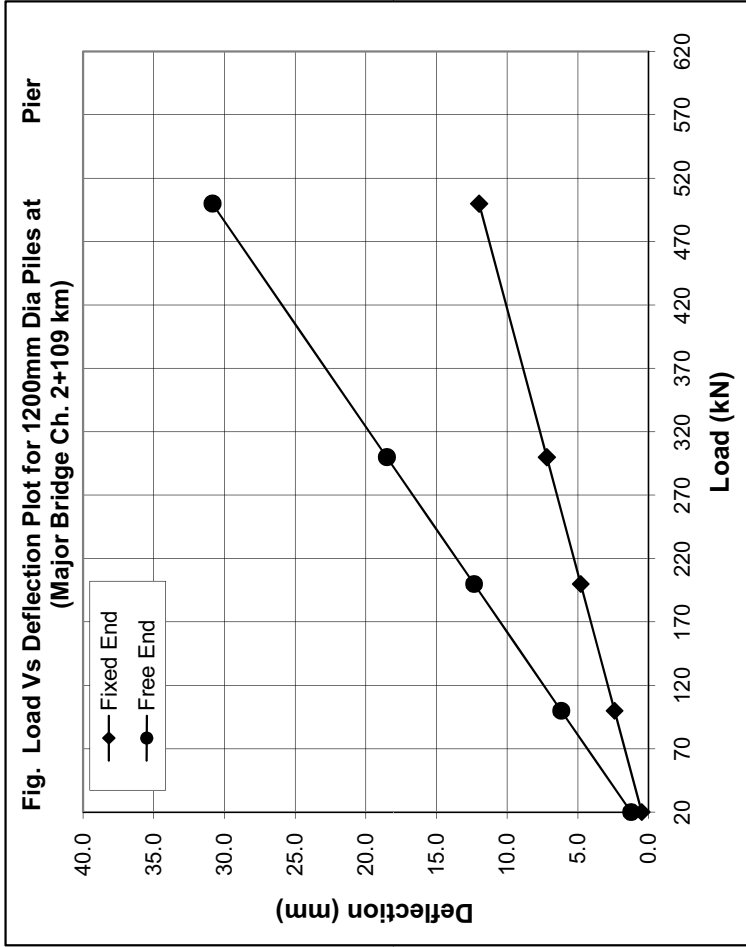
D= 120 cm
 $\eta_b = 0.210 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

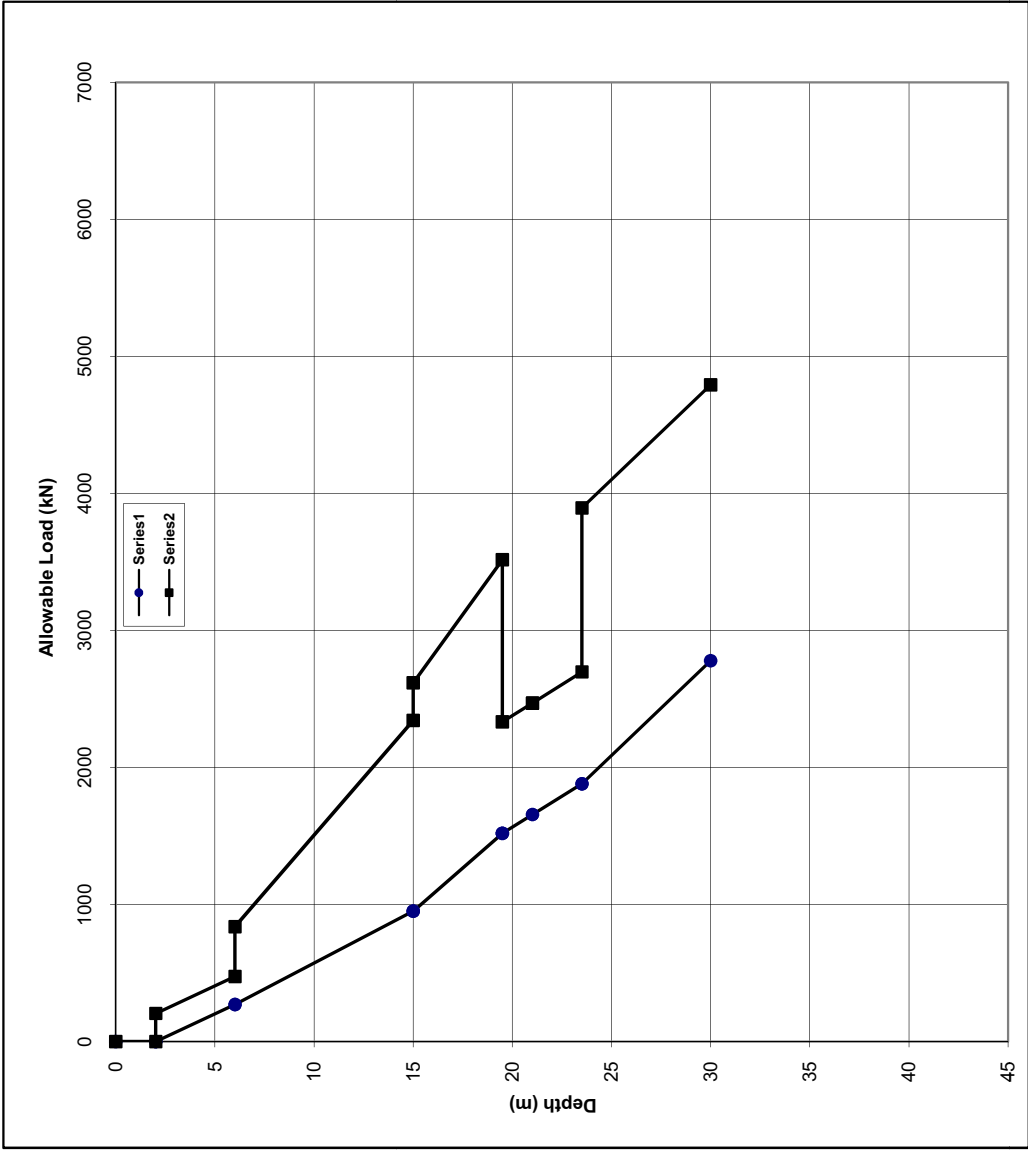
$T = (EI/\eta h)^{0.2}$
 420.11
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 924.25$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 798.22$ cm
 $L_f = 0$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
20	0.48	1.23
100	2.39	6.17
200	4.79	12.34
300	7.18	18.51
500	11.97	30.84

Hence lateral capacity (load corresponding =12mm deflection)

= 500kN (for fixed head condition)
 = 190kN (for free head condition)





Allowable Pile Capacity for Major Bridge, Chainage-2+306 km

Pile Type = Bored
 Pile Dia (mm) = 1200
 Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT Major Bridge Ch.2+306km (IS: 2911 - PART-1/SEC-2-2010)

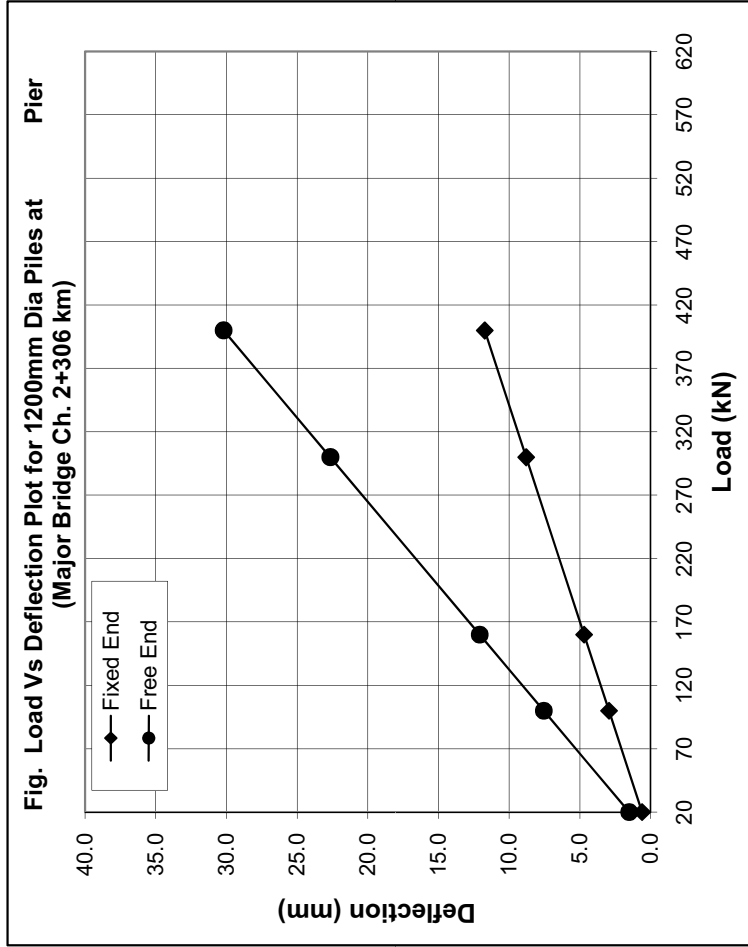
D= 120 cm
 $\eta_p = 0.150 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2}$
 449.36
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 988.59$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 853.78$ cm
 $L_f = 0$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
20	0.59	1.51
100	2.93	7.55
160	4.69	12.08
300	8.79	22.65
400	11.72	30.19

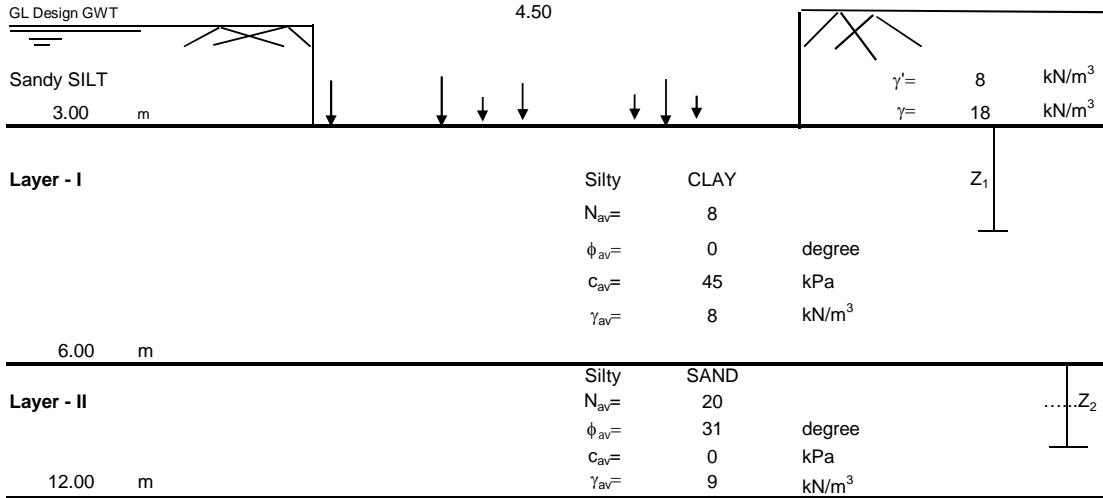
Hence lateral capacity (load corresponding =12mm deflection)

= 400kN (for fixed head condition)
 = 160kN (for free head condition)



Calculation for Bearing Capacity at Minor Bridge CH 2+873 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5		
N_c =	5.14	N_q =	1	N_{γ} =	0.00
S_c =	1.164	S_q =	1.164	S_{γ} =	0.67
dc =	1 + 0.2 * (D/B) * tan(45 + ϕ /2) =		1.13		
dq = d γ =	1 + 0.1 * (D/B) * tan(45 + ϕ /2) =		1.07		
ic = iq =	(1 - α /90) ² =	1.00	ig = (1 - α / ϕ) ² =	1.00	α = 0
Q_{safe-I} =	122.1	kPa			

Design Bearing Capacity = **120 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

m_v = 0.00015 μ_g = 0.7 for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{k0}/p_o) =$ 0.0 $C_{k0}/N =$ for sand

$p_o =$ 15 $p =$ 120.0 kPa 1st layer I = 0.88

Rigidity factor = 0.8 Depth Factor, $d_f =$ 0.82

δ_1 (mm) = 21.82

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ m^2/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) =$ 100.0 $C_{kd}/N =$ 250 kN/m^2 2nd layer I = 0.28
 $p_o =$ 75 $p =$ 120.0 kPa
 Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.82

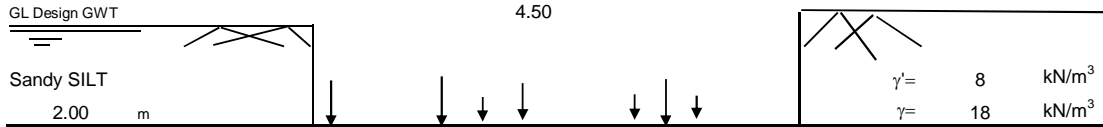
δ_2 (mm) = 14.57

Total settlement = 36.39 mm

Allowable Bearing capacity for 25mm settlement = 83.8 KPa

Calculation for Bearing Capacity at Minor Bridge CH 3+490 km

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Layer - I	Silty CLAY				
	N_{av} =	17			
	ϕ_{av} =	0	degree		
	c_{av} =	60	kPa		
	γ_{av} =	8	kN/m ³		
7.50 m					
Layer - II	Silty SAND				
	N_{av} =	31			
	ϕ_{av} =	33	degree		
	c_{av} =	0	kPa		
	γ_{av} =	9	kN/m ³		
12.00 m					

Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS=	2.5	w=	0.5		
N_c =	5.14	N_q =	1	N_{γ} =	0.00
S_c =	1.164	S_q =	1.164	S_{γ} =	0.67
dc=	1+0.2*(D/B)*tan(45+ ϕ /2)=		1.09		
dq=d γ =	1+0.1*(D/B)*tan(45+ ϕ /2)		1.04		
ic=iq=	(1- α /90) ² =		1.00	ig= (1- α / ϕ) ² =	1.00
$Q_{\text{safe-I}}$ =	156.4	kPa			$\alpha=0$

Design Bearing Capacity= **150 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.0001$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ for sand

$p_o = 24$ $p = 150.0$ kPa $\text{1st layer I} = 0.64$

Rigidity factor= 0.8 Depth Factor, $d_f = 0.88$

δ_1 (mm) = 26.02

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ m^2/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) =$ 144.9 $C_{kd}/N =$ 250 kN/m^2 2nd layer I = 0.2
 $p_o =$ 80.25 $p =$ 150.0 kPa
 Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.88

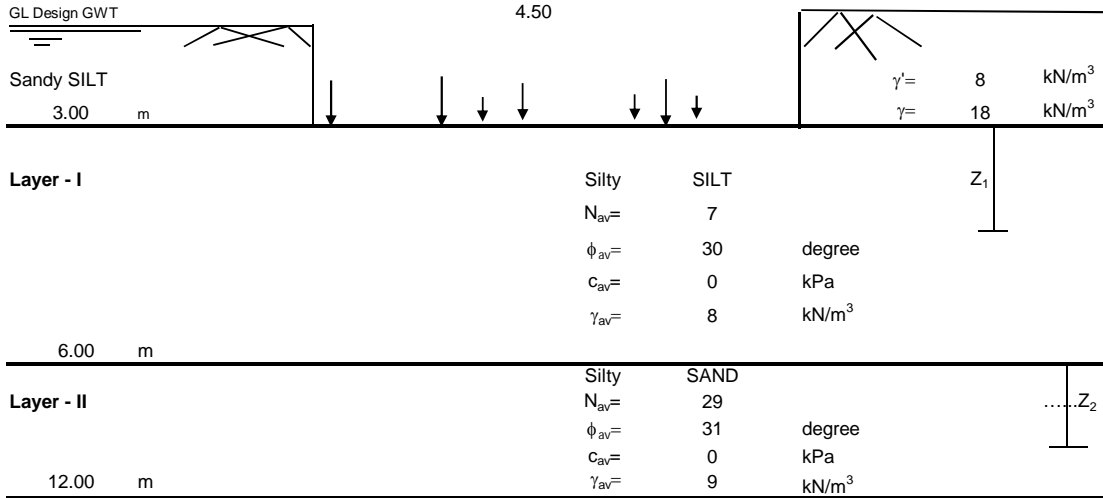
δ_2 (mm) = 6.95

Total settlement = 32.97 mm

Allowable Bearing capacity for 25mm settlement = 118.6 KPa

Calculation for Bearing Capacity at Minor Bridge CH 4+252 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS=	2.5	w=	0.5		
N_c =	16	N_q =	7.25	N_{γ} =	7.59
S_c =	1.164	S_q =	1.164	S_{γ} =	0.67
d_c =	$1 + 0.2 * (D/B) * \tan(45 + \phi/2) =$		1.23		
$d_q = d_{\gamma}$ =	$1 + 0.1 * (D/B) * \tan(45 + \phi/2) =$		1.12		
$i_c = i_q$ =	$(1 - \alpha/90)^2 =$	1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$	124.1	kPa			

Design Bearing Capacity= **120 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor} (0.8)$$

m_v = m²/kN

μ_g =

for clay

$$\delta (\text{mm}) = [2.303 * (H/C) * \log_{10}((p_o + \Delta p) / p_o)] * d_r * \text{Rigidity Factor} (0.8)$$

$C = 1.5 * (C_{k0} / p_o) =$

175.0

$C_{k0} / N =$

250 KN/m²

1st layer I = 0.88

$p_o =$

15

$p =$

120.0

kPa

Rigidity factor= 0.8

Depth Factor, $d_r =$

0.82

$\delta_1 (\text{mm}) = 23.44$

Settlement for Layer-II

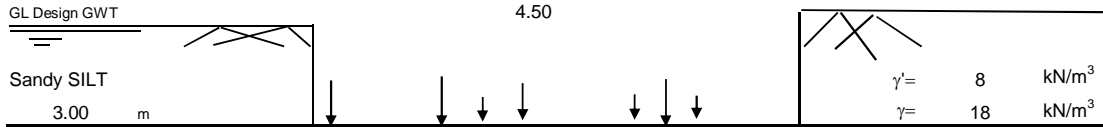
δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$		for clay
$m_v =$	m^2/kN	$\mu_g =$
δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$		for sand IS:8009 (Part I)
$C = 1.5 \cdot (C_{kd}/p_o) =$	174.0	$C_{kd}/N =$
$p_o =$	75	$p =$
Rigidity factor = 0.8	120.0	kPa
	Depth Factor, $d_r =$	0.82
δ_2 (mm) =	8.37	

Total settlement = 31.81 mm

Allowable Bearing capacity for 25mm settlement = 97.5 KPa

Calculation for Bearing Capacity at Minor Bridge CH 5+163 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY	$N_{av} = 12$	$\phi_{av} = 0$ degree	$c_{av} = 45$ kPa	$\gamma_{av} = 8$ kN/m ³	Z_1
4.50 m						
Layer - II	Silty SAND	$N_{av} = 26$	$\phi_{av} = 31$ degree	$c_{av} = 0$ kPa	$\gamma_{av} = 9$ kN/m ³	Z_2
12.00 m						

Safe Bearing Capacity from Shear Failure

Design $\phi = 0$ degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS = 2.5	w = 0.5					
$N_c = 5.14$	$N_q = 1$	$N_{\gamma} = 0.00$				Local shear failure
$S_c = 1.164$	$S_q = 1.164$	$S_{\gamma} = 0.67$				
$d_c = 1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) = 1.13$						
$d_q = d_{\gamma} = 1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) = 1.07$						
$i_c = i_q = (1 - \alpha/90)^2 = 1.00$	$i_{\gamma} = (1 - \alpha/\phi)^2 = 1.00$				$\alpha = 0$	
$Q_{\text{safe-I}} = 122.1$ kPa						

Design Bearing Capacity = **120 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.00014$ m²/kN $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ for sand

$p_o = 9$ $p = 120.0$ kPa $C_{kd}/N =$ KN/m²

Rigidity factor = 0.8 Depth Factor, $d_f = 0.82$ 1st layer I = 1

δ_1 (mm) = 11.57

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ m^2/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) =$ 139.8 $C_{kd}/N =$ 250 kN/m^2 2nd layer I = 0.28
 $p_o =$ 69.75 $p =$ 120.0 kPa
 Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.82

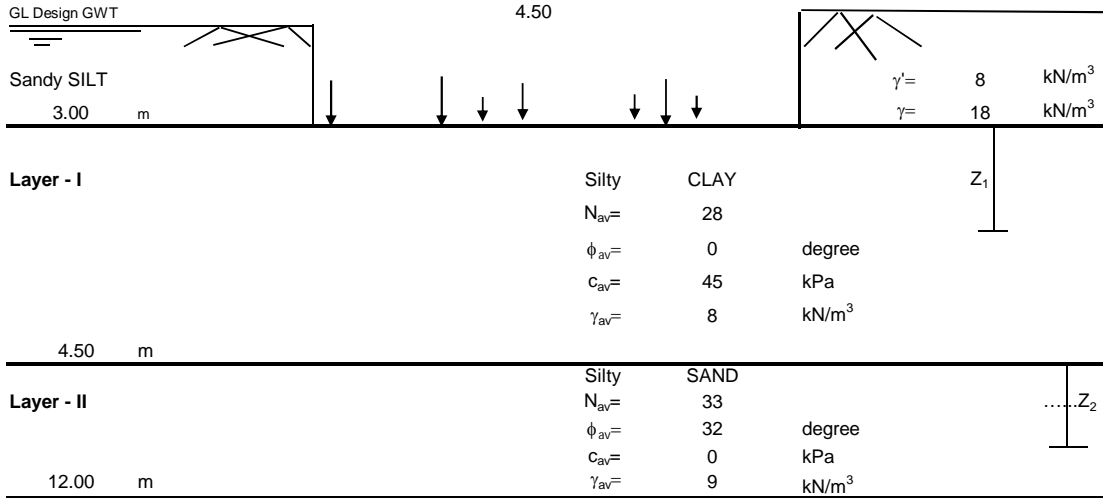
δ_2 (mm) = 13.84

Total settlement = 25.41 mm

Allowable Bearing capacity for 25mm settlement = 120.0 KPa

Calculation for Bearing Capacity at Minor Bridge CH 7+064 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5			
$N_c =$	5.14	$N_q =$	1	$N_{\gamma} =$	0.00	Local shear failure
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67	
$d_c =$	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.13			
$d_q = d_{\gamma} =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.07			
$i_c = i_q =$	$(1 - \alpha/90)^2 =$		1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$	122.1	kPa				

Design Bearing Capacity = **120 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.00006$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{k0}/p_o) = 0.0$ $C_{k0}/N =$ for sand

$p_o = 9$ $p = 120.0$ kPa $\text{1st layer } l = 1$

Rigidity factor = 0.8 Depth Factor, $d_f = 0.82$

δ_1 (mm) = 4.96

Settlement for Layer-II

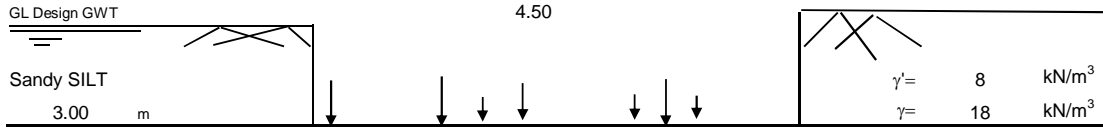
$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ <p style="text-align: center;"> $m_v =$ m^2/kN </p>	for clay $\mu_g =$
$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ <p style="text-align: center;"> $C = 1.5 \cdot (C_{kd}/p_o) =$ 177.4 </p> <p style="text-align: center;"> $p_o =$ 69.75 $p =$ 120.0 </p> <p style="text-align: center;"> $\text{Rigidity factor} = 0.8$ $\text{Depth Factor, } d_r =$ </p>	for sand $IS:8009 \text{ (Part I)}$ 250 KN/m^2 $2nd \text{ layer } I = 0.28$ $C_{kd}/N =$ kPa 0.82
$\delta_2 \text{ (mm)} =$ 10.90	

Total settlement = 15.86 mm

Allowable Bearing capacity for 25mm settlement = 120.0 KPa

Calculation for Bearing Capacity at Major Bridge CH 8+977 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY		Z ₁
N _{av} =	8		
φ _{av} =	0	degree	
c _{av} =	45	kPa	
γ _{av} =	8	kN/m ³	
6.00	m		
Layer - II	Silty CLAY	Z ₂
N _{av} =	19		
φ _{av} =	0	degree	
c _{av} =	65	kPa	
γ _{av} =	9	kN/m ³	
12.00	m		

Safe Bearing Capacity from Shear Failure

Design φ = 0 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5		
N _c =	5.14	N _q =	1	N _γ =	0.00
S _c =	1.164	S _q =	1.164	S _γ =	0.67
dc =	1 + 0.2 * (D/B) * tan(45 + φ/2) =		1.13		
dq = dγ =	1 + 0.1 * (D/B) * tan(45 + φ/2) =		1.07		
ic = iq =	(1 - α/90) ² =		1.00	ig = (1 - α/φ) ² =	1.00
Q_{safe-I} =	122.1	kPa			α = 0

Design Bearing Capacity = **120 kPa**

Settlement for Layer - I

δ (mm) = m_v * H * Δp * μ_g * d_r * Rigidity Factor (0.8)

m_v = 0.0002 m_g = 0.7 for clay

δ (mm) = [2.303 * (H/C) * log₁₀((p_o + Δp)/p_o)] * d_r * Rigidity Factor (0.8)

C = 1.5 * (C_{kd}/p_o) = 0.0 C_{kd}/N = for sand

p_o = 15 p = 120.0 kPa 1st layer I = 0.88

Rigidity factor = 0.8 Depth Factor, d_r = 0.82

δ₁ (mm) = 29.09

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v = 0.00008$ m^2/kN $\mu_g = 0.7$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ kN/m^2 2nd layer I = 0.28
 $p_o = 75$ $p = 120.0$ kPa
 Rigidity factor = 0.8 Depth Factor, $d_r = 0.82$

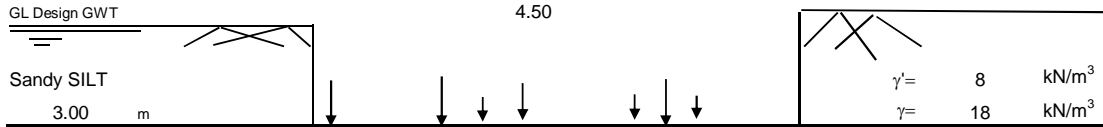
δ_2 (mm) = 7.41

Total settlement = 36.50 mm

Allowable Bearing capacity for 25mm settlement = 83.6 KPa

Calculation for Bearing Capacity at Minor Bridge CH 10+030 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY			Z ₁
	N _{av} =	8		
	φ _{av} =	0	degree	
	c _{av} =	50	kPa	
	γ _{av} =	8	kN/m ³	
5.00 m				
Layer - II	Silty SAND			...Z ₂
	N _{av} =	22		
	φ _{av} =	31	degree	
	c _{av} =	0	kPa	
	γ _{av} =	9	kN/m ³	
12.00 m				

Safe Bearing Capacity from Shear Failure

Design φ = 0 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5		
N _c =	5.14	N _q =	1	N _γ =	0.00
S _c =	1.164	S _q =	1.164	S _γ =	0.67
dc =	1 + 0.2 * (D/B) * tan(45 + φ/2) =		1.13		
dq = dγ =	1 + 0.1 * (D/B) * tan(45 + φ/2) =		1.07		
ic = iq =	(1 - α/90) ² =		1.00	ig = (1 - α/φ) ² =	1.00
Q_{safe-I} =	135.6	kPa			α = 0

Design Bearing Capacity = **135 kPa**

Settlement for Layer - I

δ (mm) = m_v * H * Δp * μ_g * d_r * Rigidity Factor (0.8)

m_v = 0.00015 m²/kN μ_g = 0.7 for clay

δ (mm) = [2.303 * (H/C) * log₁₀((p_o + Δp)/p_o)] * d_r * Rigidity Factor (0.8)

C = 1.5 * (C_{k0}/p_o) = 0.0 C_{k0}/N = for sand

p_o = 11 p = 135.0 kPa 1st layer I = 0.94

Rigidity factor = 0.8 Depth Factor, d_r = 0.82

δ₁ (mm) = 17.48

Settlement for Layer-II

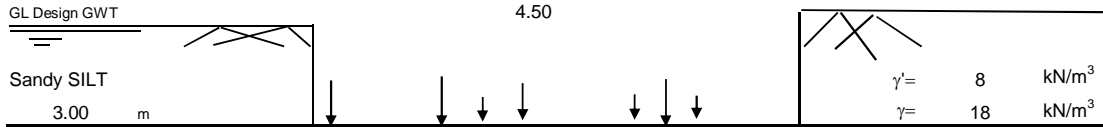
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for clay	
	$m_v =$	m^2/kN		$\mu_g =$		
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for sand	IS:8009 (Part I)
	$C = 1.5 \cdot (C_{kd}/p_o) =$	115.4		$C_{kd}/N =$	250 KN/m ²	2nd layer I = 0.28
	$p_o =$	71.5	$p =$	135.0	kPa	
	Rigidity factor = 0.8			Depth Factor, $d_r =$	0.82	
	δ_2 (mm) =	16.89				

Total settlement = 34.37 mm

Allowable Bearing capacity for 25mm settlement = 98.6 KPa

Calculation for Bearing Capacity at Major Bridge CH 10+973 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY			Z ₁
N _{av} =	24			
φ _{av} =	0	degree		
c _{av} =	65	kPa		
γ _{av} =	8	kN/m ³		
7.50	m			
Layer - II	Silty SAND			Z ₂
N _{av} =	35			
φ _{av} =	32	degree		
c _{av} =	0	kPa		
γ _{av} =	9	kN/m ³		
12.00	m			

Safe Bearing Capacity from Shear Failure

Design φ= 0 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS=	2.5	w=	0.5		
N _c =	5.14	N _q =	1	N _γ =	0.00
S _c =	1.164	S _q =	1.164	S _γ =	0.67
dc=	1+0.2*(D/B)*tan(45+φ/2)=		1.13		
dq=dγ=	1+0.1*(D/B)*tan(45+φ/2)		1.07		
ic=iq=	(1-α/90) ² =	1.00	ig= (1-α/φ) ² =	1.00	α=0
Q_{safe-I}=		176.3	kPa		

Design Bearing Capacity= **175 kPa**

Settlement for Layer - I

δ (mm) = m_v * H * Δp * μ_g * d_r * Rigidity Factor (0.8)

m_v = 0.00007 m²/kN μ_g = 0.7 for clay

δ (mm) = [2.303 * (H/C) * log₁₀((p_o + Δp) / p_o)] * d_r * Rigidity Factor (0.8)

C = 1.5 * (C_{ku} / p_o) = 0.0 C_{ku} / N = for sand

p_o = 21 p = 175.0 kPa 1st layer I = 0.98

Rigidity factor = 0.8 Depth Factor, d_r = 0.82

δ₁ (mm) = 24.81

Settlement for Layer-II

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

 $m_v =$ m^2/kN $\mu_g =$

for clay

$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$$C = 1.5 \cdot (C_{kd}/p_o) =$$

163.6

$$C_{kd}/N =$$

for sand
250 KN/m²IS:8009 (Part I)
2nd layer I = 0.2

$$p_o = 80.25$$

$$p =$$

175.0

kPa

$$\text{Rigidity factor} = 0.8$$

$$\text{Depth Factor, } d_r =$$

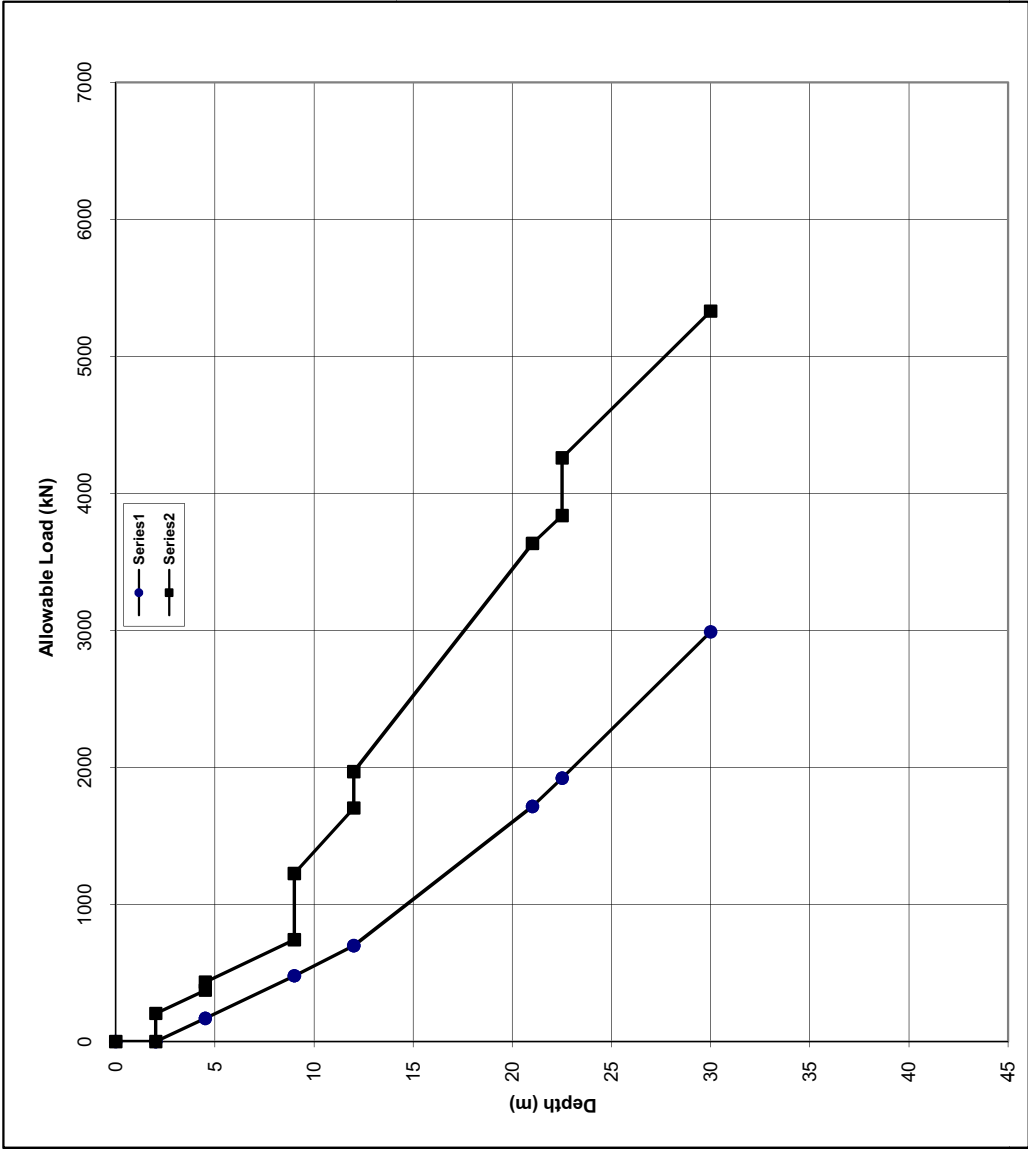
0.82

$$\delta_2 \text{ (mm)} = 6.53$$

$$\text{Total settlement} = 31.34 \text{ mm}$$

$$\text{Allowable Bearing capacity for 25mm settlement} =$$

140.6 KPa



Allowable Pile Capacity for Major Bridge, Chainage-13+841 km

Pile Type = Bored
 Pile Dia (mm) = 1200
 Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT Major Bridge Ch.13+841km (IS: 2911 - PART-1/SEC-2-2010)

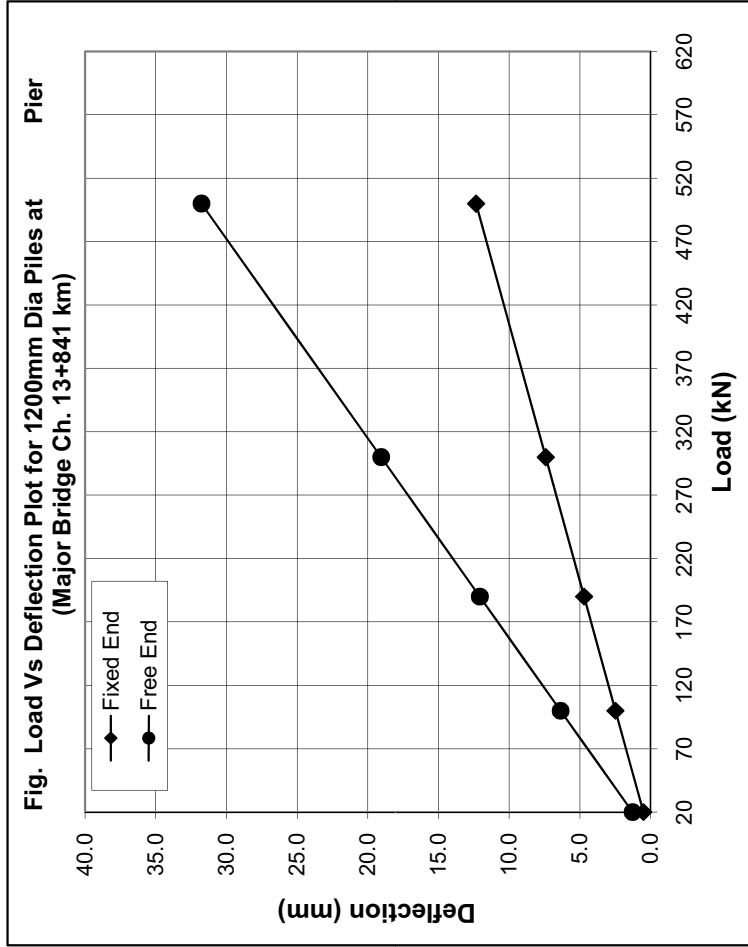
D= 120 cm
 $\eta_p = 0.200 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

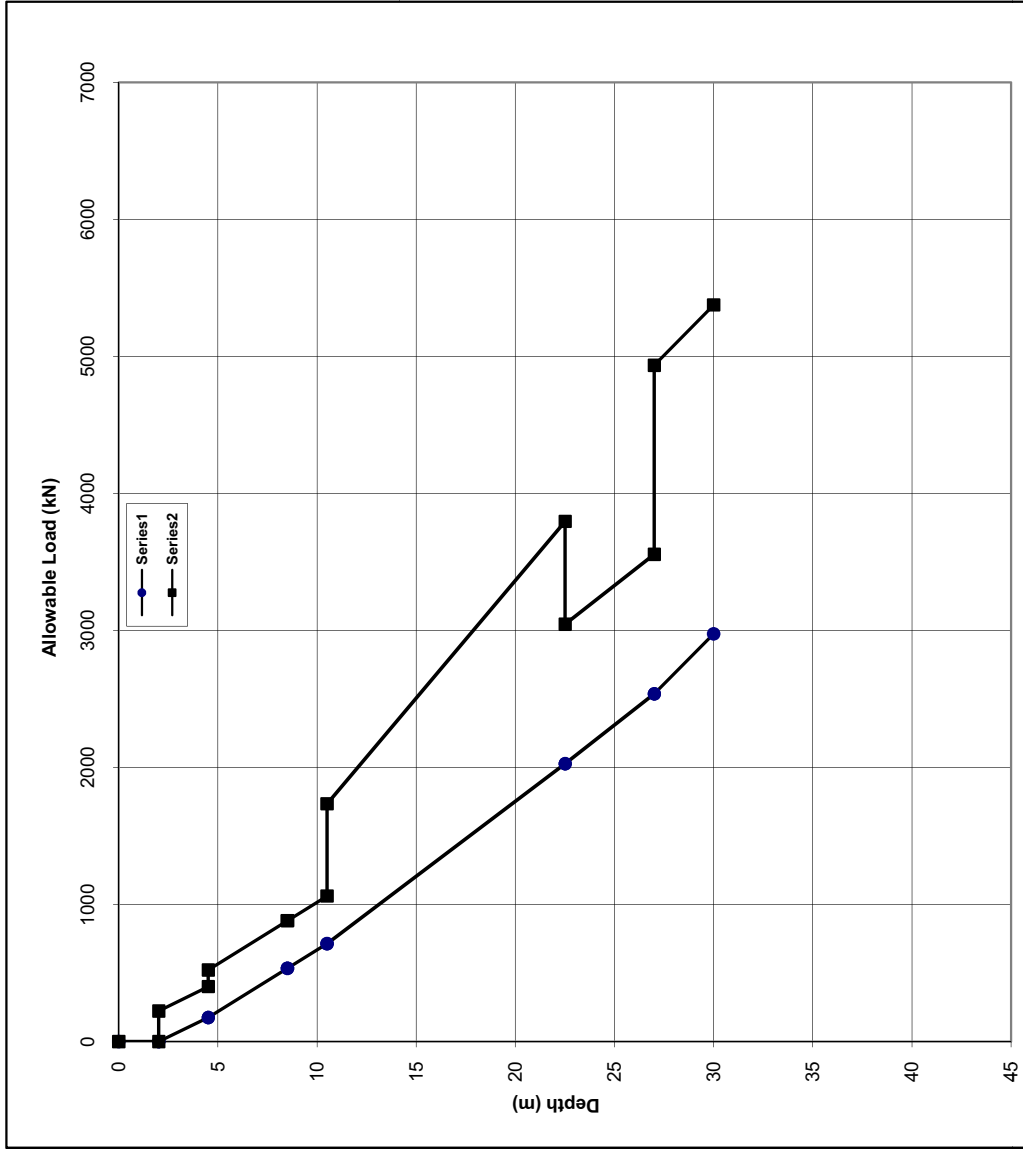
$T = (EI/\eta h)^{0.2}$
 424.23
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 933.32$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 806.05$ cm
 $L_f = 0$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
20	0.49	1.27
100	2.47	6.35
190	4.68	12.07
300	7.40	19.06
500	12.33	31.76

Hence lateral capacity (load corresponding =12mm deflection)

= 500kN (for fixed head condition)
 = 190kN (for free head condition)





Allowable Pile Capacity for Major Bridge, Chainage-13+841 km

Pile Type = Bored
 Pile Dia (mm) = 1200
 Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT Pier (Major Bridge Ch.13+841km) (IS: 2911 - PART-1/SEC-2-2010)

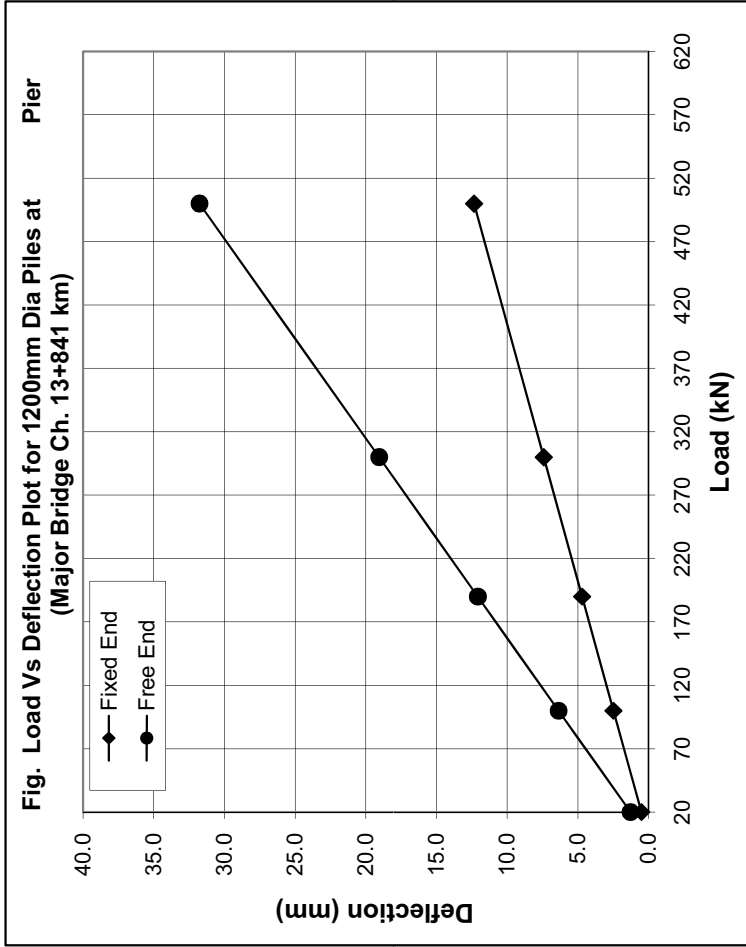
D= 120 cm
 $\eta_p = 0.200 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2}$
 424.23
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 933.32$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 806.05$ cm
 $L_f = 0$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
20	0.49	1.27
100	2.47	6.35
190	4.68	12.07
300	7.40	19.06
500	12.33	31.76

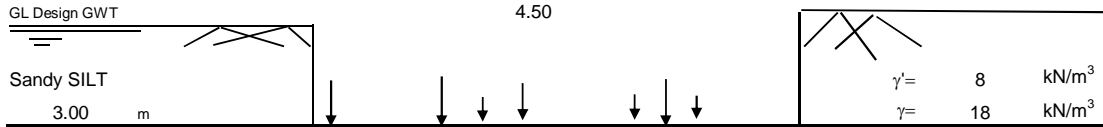
Hence lateral capacity (load corresponding =12mm deflection)

= 500kN (for fixed head condition)
 = 190kN (for free head condition)



Calculation for Bearing Capacity at Minor Bridge CH 13+136 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY		Z ₁
N _{av} =	12		
φ _{av} =	0	degree	
c _{av} =	60	kPa	
γ _{av} =	8	kN/m ³	
6.00 m			
Layer - II	Silty SAND		Z ₂
N _{av} =	28		
φ _{av} =	31	degree	
c _{av} =	0	kPa	
γ _{av} =	8.5	kN/m ³	
12.00 m			

Safe Bearing Capacity from Shear Failure

Design φ = 0 degree

For Layer - I

As φ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5		
N _c =	5.14	N _q =	1	N _γ =	0.00
S _c =	1.164	S _q =	1.164	S _γ =	0.67
dc =	1 + 0.2 * (D/B) * tan(45 + φ/2) =		1.13		
dq = dγ =	1 + 0.1 * (D/B) * tan(45 + φ/2)		1.07		
ic = iq =	(1 - α/90)² =	1.00	ig = (1 - α/φ)² =	1.00	α = 0
Q_{safe-I} =		162.7	kPa		

Design Bearing Capacity = **160 kPa**

Settlement for Layer - I

δ (mm) = m_v * H * Δp * μ_g * d_r * Rigidity Factor (0.8)

m_v = 0.00012 m²/kN μ_g = 0.7 for clay

δ (mm) = [2.303 * (H/C) * log₁₀((p_o + Δp)/p_o)] * d_r * Rigidity Factor (0.8)

C = 1.5 * (C_{k0}/p_o) = 0.0 C_{k0}/N = for sand

p_o = 15 p = 160.0 kPa 1st layer I = 0.9

Rigidity factor = 0.8 Depth Factor, d_r = 0.84

δ₁ (mm) = 24.39

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay

$m_v =$ m^2/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)

$C = 1.5 \cdot (C_{kd}/p_o) =$ 142.9 $C_{kd}/N =$ 250 kN/m^2 2nd layer I = 0.28

$p_o =$ 73.5 $p =$ 160.0 kPa

Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.84

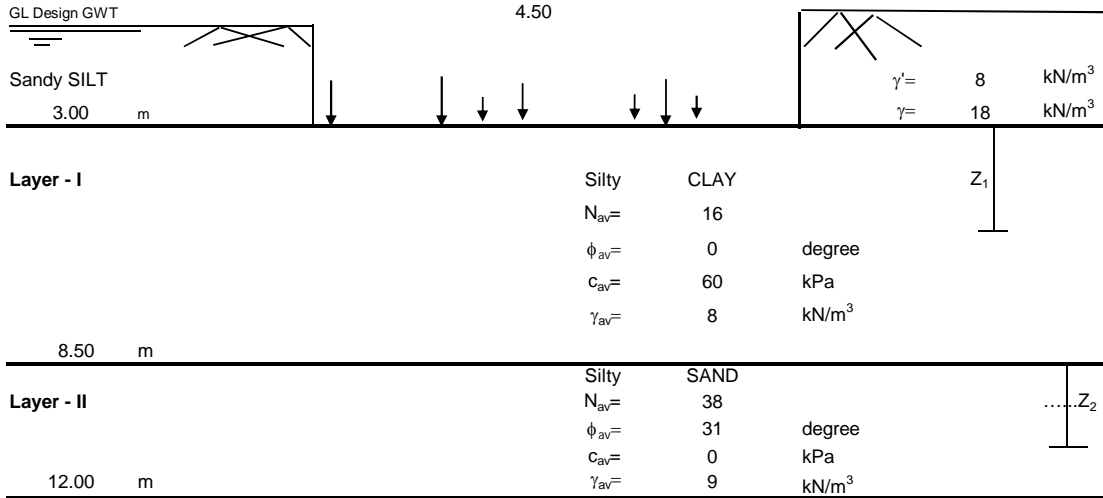
δ_2 (mm) = 13.43

Total settlement = **37.82 mm**

Allowable Bearing capacity for 25mm settlement = **107.6 KPa**

Calculation for Bearing Capacity at Minor Bridge CH 15+227 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5			
$N_c =$	5.14	$N_q =$	1	$N_{\gamma} =$	0.00	Local shear failure
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67	
$dc =$	$1 + 0.2 * (D/B) * \tan(45 + \phi/2) =$		1.13			
$dq = d_{\gamma} =$	$1 + 0.1 * (D/B) * \tan(45 + \phi/2)$		1.07			
$ic = iq =$	$(1 - \alpha/90)^2 =$		1.00	$ig = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$	162.7		kPa			

Design Bearing Capacity = **160 kPa**

Settlement for Layer - I

δ (mm) = $m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor (0.8)}$

$m_v = 0.00008$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 * (H/C) * \log_{10}((p_o + \Delta p)/p_o)] * d_r * \text{Rigidity Factor (0.8)}$

$C = 1.5 * (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ for sand

$p_o = 25$ $p = 160.0$ kPa $\text{1st layer I} = 0.64$

Rigidity factor = 0.8 Depth Factor, $d_r = 0.84$

δ_1 (mm) = 21.19

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ m^2/kN $\mu_g =$
 δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) =$ 170.1 $C_{kd}/N =$ 250 KN/m² 2nd layer I = 0.2
 $p_o =$ 83.75 $p =$ 160.0 kPa
 $\text{Rigidity factor} = 0.8$ $\text{Depth Factor, } d_r =$ 0.84

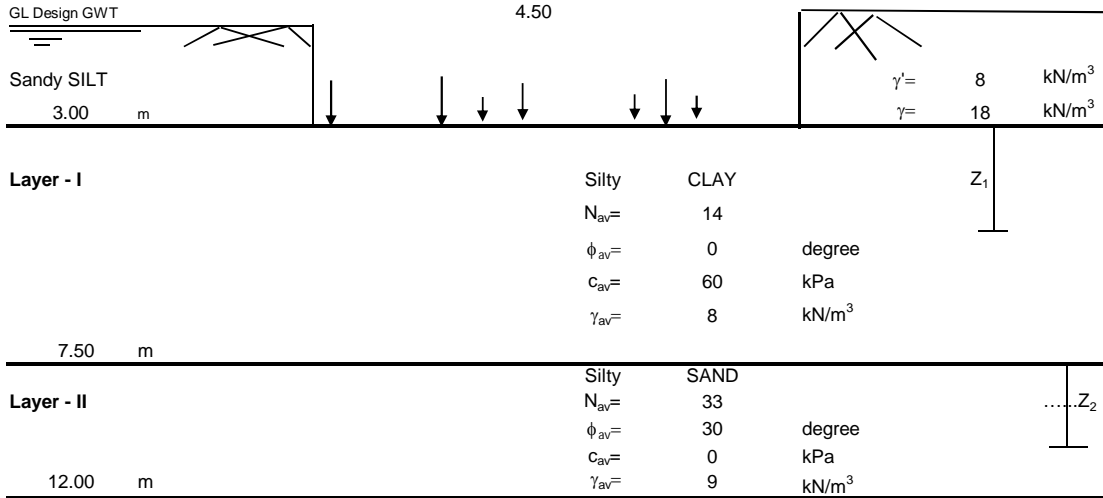
 δ_2 (mm) = **4.47**

Total settlement = **25.67 mm**

Allowable Bearing capacity for 25mm settlement = 158.5 KPa

Calculation for Bearing Capacity at Minor Bridge CH 16+144 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5		
$N_c =$	5.14	$N_q =$	1	$N_{\gamma} =$	0.00
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67
$d_c =$	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.13		
$d_q = d_{\gamma} =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.07		
$i_c = i_q =$	$(1 - \alpha/90)^2 =$		1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00
$\alpha =$					0
$Q_{\text{safe-I}} =$	162.7	kPa			

Design Bearing Capacity = **160 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.00012$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ for sand

$p_o = 21$ $p = 160.0$ kPa

Rigidity factor = 0.8 Depth Factor, $d_f = 0.84$ 1st layer I = 0.74

δ_1 (mm) = 30.08

Settlement for Layer-II

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$$m_v = \quad m^2/kN$$

$$\mu_g =$$

for clay

$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$$C = 1.5 \cdot (C_{kd}/p_o) = \quad 154.2$$

$$C_{kd}/N =$$

for sand
250 KN/m²

IS:8009 (Part I)
2nd layer I = 0.2

$$p_o = 80.25$$

$$p = 160.0$$

$$kPa$$

$$\text{Rigidity factor} = 0.8$$

$$\text{Depth Factor, } d_r =$$

$$0.84$$

$$\delta_2 \text{ (mm)} = 6.58$$

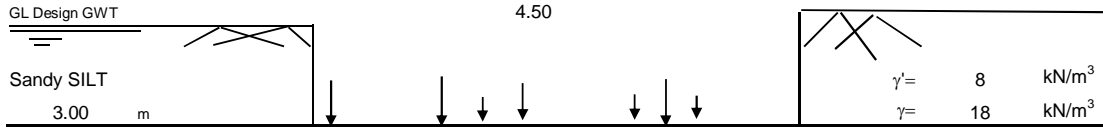
Total settlement = 36.66 mm

Allowable Bearing capacity for 25mm settlement=

111.0 KPa

Calculation for Bearing Capacity at Minor Bridge CH 17+338 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY				
6.00 m	$N_{av} = 10$				Z_1
	$\phi_{av} = 0$	degree			
	$c_{av} = 40$	kPa			
	$\gamma_{av} = 8$	kN/m ³			
Layer - II	Silty SAND				Z_2
12.00 m	$N_{av} = 28$				
	$\phi_{av} = 0$	degree			
	$c_{av} = 105$	kPa			
	$\gamma_{av} = 9$	kN/m ³			

Safe Bearing Capacity from Shear Failure

Design $\phi = 0$ degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma D)(N_q) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS = 2.5	w = 0.5				
$N_c = 5.14$	$N_q = 1$	$N_{\gamma} = 0.00$			Local shear failure
$S_c = 1.164$	$S_q = 1.164$	$S_{\gamma} = 0.67$			
$d_c = 1 + 0.2(D/B) \tan(45 + \phi/2) = 1.13$					
$d_q = d_{\gamma} = 1 + 0.1(D/B) \tan(45 + \phi/2) = 1.07$					
$i_c = i_q = (1 - \alpha/90)^2 = 1.00$	$i_{\gamma} = (1 - \alpha/\phi)^2 = 1.00$			$\alpha = 0$	
$Q_{\text{safe-I}} = 108.5$	kPa				

Design Bearing Capacity = **105.0 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v H \Delta p \mu_g d_f \text{Rigidity Factor (0.8)}$$

$m_v = 0.00014$	$\mu_g = 0.7$			for clay
				} 1st layer I = 0.9
$\delta (\text{mm}) = [2.303(H/C) \log_{10}((p_o + \Delta p)/p_o)] d_f \text{Rigidity Factor (0.8)}$				for sand
$C = 1.5(C_{kd}/p_o) = 0.0$	$C_{kd}/N =$			
$p_o = 15$	$p = 105.0$	kPa		
Rigidity factor = 0.8	Depth Factor, $d_f =$			0.84
$\delta_1 (\text{mm}) = 18.67$				

Settlement for Layer-II

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$m_v =$ m^2/kN $\mu_g =$ for clay

$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$C = 1.5 \cdot (C_{kd}/p_o) =$ 140.0 $C_{kd}/N =$ for sand IS:8009 (Part I)
 $p_o =$ 75 $p =$ 105.0 kPa 250 KN/m^2 2nd layer I = 0.28
 $\text{Rigidity factor} = 0.8$ $\text{Depth Factor, } d_r =$ 0.84

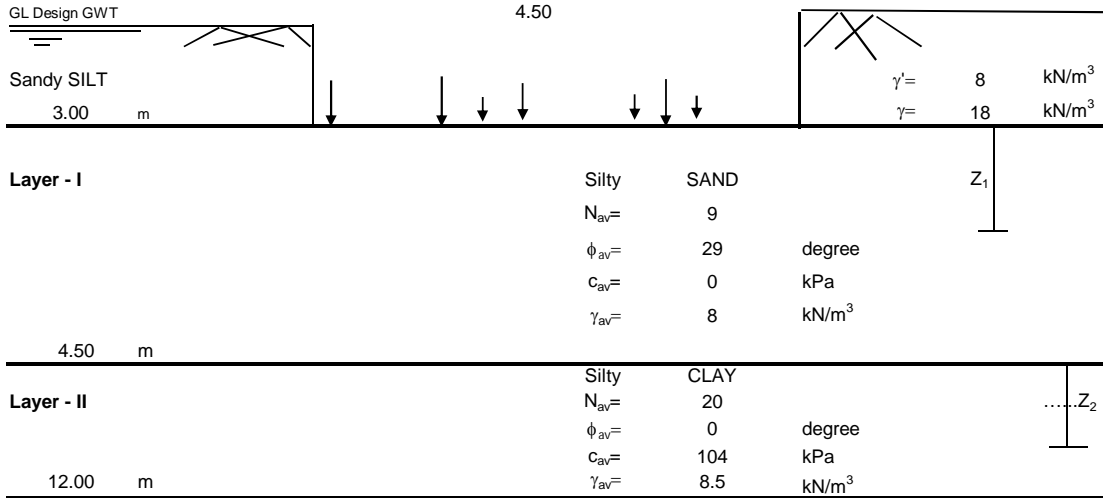
$\delta_2 \text{ (mm)} = 9.53$

Total settlement = 28.20 mm

Allowable Bearing capacity for 25mm settlement= 96.2 KPa

Calculation for Bearing Capacity at Minor Bridge CH 17+338 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 29 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS=	2.5	w=	0.5			
N_c =	15.5	N_q =	6.88	N_{γ} =	6.65	Local shear failure
S_c =	1.164	S_q =	1.164	S_{γ} =	0.67	
d_c =	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.23			
$d_q = d_{\gamma}$ =	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.11			
$i_c = i_q$ =	$(1 - \alpha/90)^2 =$	1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$	
$Q_{\text{safe-I}} =$	113.5	kPa				

Design Bearing Capacity= **113 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$

m_v = m²/kN μ_g = for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{k0}/p_o) =$ 300.0 $C_{k0}/N =$ 200 KN/m² for sand } 1st layer I = 1

$p_o =$ 9 $p =$ 113.0 kPa

Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.84

δ_1 (mm) = 8.76

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v = 0.00009$ m^2/kN $\mu_g = 0.7$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ KN/m^2 2nd layer I = 0.28
 $p_o = 67.875$ $p = 113.0$ kPa
 Rigidity factor = 0.8 Depth Factor, $d_r = 0.84$

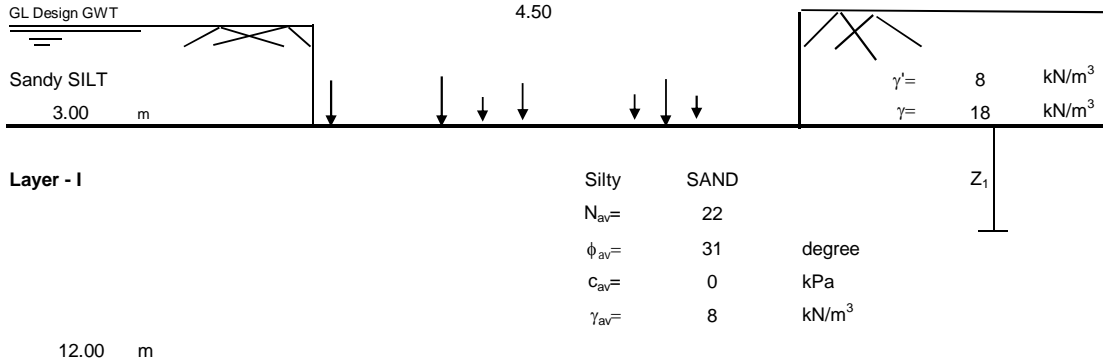
δ_2 (mm) = 10.05

Total settlement = 18.81 mm

Allowable Bearing capacity for 25mm settlement = 113.0 KPa

Calculation for Bearing Capacity at Minor Bridge CH 19+955 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design $\phi = 31$ degree

For Layer - I

As ϕ is 31° Intermediate shear failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q) s_q d_q i_q + 0.5 B \gamma N_y s_y d_y i_{yw}) / FS$$

FS = 2.5		w = 0.5			
$N_c = 33.34$	$N_q = 21.38$		$N_y = 27.54$	General shear failure	
$N_c = 17.19$	$N_q = 8.1$		$N_y = 7.59$	Local shear failure	
$N_c = 19.88$	$N_q = 10.31$		$N_y = 10.92$	Intermediate condition	
$S_c = 1.164$	$S_q = 1.164$		$S_y = 0.67$		
$dc = 1 + 0.2 * (D/B) * \tan(45 + \phi/2) = 1.24$					
$dq = d\gamma = 1 + 0.1 * (D/B) * \tan(45 + \phi/2) = 1.12$					
$ic = iq = (1 - \alpha/90)^2 = 1.00$		$ig = (1 - \alpha/\phi)^2 = 1.00$	$\alpha = 0$		
$Q_{\text{safe-I}} = 182.8$ kPa					

Design Bearing Capacity = **180.0 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor} (0.8)$$

$m_v = \text{m}^2/\text{kN}$ $H_g =$ for clay

$$\delta (\text{mm}) = [2.303 * (H/C) * \log_{10}((p_o + \Delta p)/p_o)] * d_r * \text{Rigidity Factor} (0.8)$$

$C = 1.5 * (C_{kd}/p_o) = 211.5$ $C_{kd}/N = 250 \text{ KN/m}^2$ for sand

$p_o = 39$ $p = 180.0$ kPa

Rigidity factor = 0.8 Depth Factor, $d_f = 0.84$

$\delta_1 (\text{mm}) = 27.99$

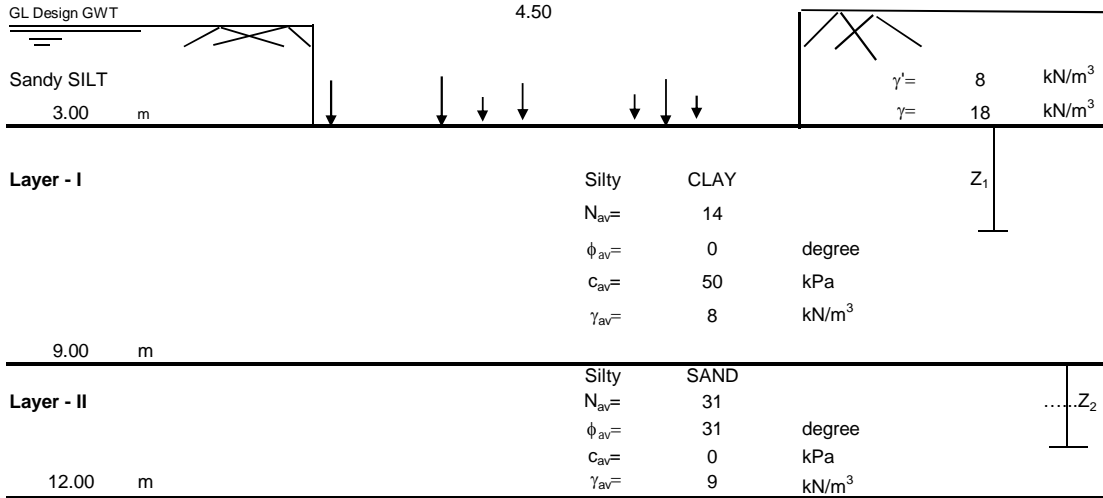
1st layer I = 0.36

Total settlement = 27.99 mm

Allowable Bearing capacity for 25mm settlement= 163.3 KPa

Calculation for Bearing Capacity at Minor Bridge CH 20+935 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 0 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5			
$N_c =$	5.14	$N_q =$	1	$N_{\gamma} =$	0.00	Local shear failure
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67	
$d_c =$	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.13			
$d_q = d_{\gamma} =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.07			
$i_c = i_q =$	$(1 - \alpha/90)^2 =$	1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$	
$Q_{\text{safe-I}} =$	135.6	kPa				

Design Bearing Capacity = **135.0 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.00013$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{k0}/p_o) = 0.0$ $C_{k0}/N =$ for sand

$p_o = 27$ $p = 135.0$ kPa 1st layer I = 0.6

Rigidity factor = 0.8 Depth Factor, $d_f = 0.84$

δ_1 (mm) = 29.72

Settlement for Layer-II

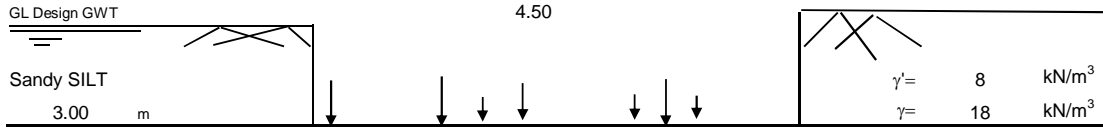
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$	for clay	
$m_v =$	m2/kN	$\mu_g =$	
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$	for sand	IS:8009 (Part I)
$C = 1.5 \cdot (C_{kd}/p_o) =$	136.0	$C_{kd}/N =$	250 KN/m ²
$p_o =$	85.5	$p =$	135.0
Rigidity factor =	0.8	Depth Factor, $d_r =$	0.84
δ_2 (mm) =	4.07		

Total settlement = 33.79 mm

Allowable Bearing capacity for 25mm settlement = 100.3 KPa

Calculation for Bearing Capacity at Minor Bridge CH 23+808 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer - I	Silty CLAY					
5.50 m	$N_{av} = 5$	5				Z_1
	$\phi_{av} = 0$	0	degree			
	$c_{av} = 40$	40	kPa			
	$\gamma_{av} = 8$	8	kN/m ³			
Layer - II	Silty SAND					Z_2
12.00 m	$N_{av} = 33$	33				
	$\phi_{av} = 32$	32	degree			
	$c_{av} = 0$	0	kPa			
	$\gamma_{av} = 8.5$	8.5	kN/m ³			

Safe Bearing Capacity from Shear Failure

Design $\phi = 0$ degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5			
$N_c =$	5.14	$N_q =$	1	$N_{\gamma} =$	0.00	Local shear failure
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67	
$d_c =$	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.13			
$d_q = d_{\gamma} =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.07			
$i_c = i_q =$	$(1 - \alpha/90)^2 =$		1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$	108.5	kPa				

Design Bearing Capacity = **105.0 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$

$m_v = 0.00023$ $\mu_g = 0.7$ for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ for sand

$p_o = 13$ $p = 105.0$ kPa 1st layer I = 0.9

Rigidity factor = 0.8 Depth Factor, $d_r = 0.84$

δ_1 (mm) = 25.56

Settlement for Layer-II

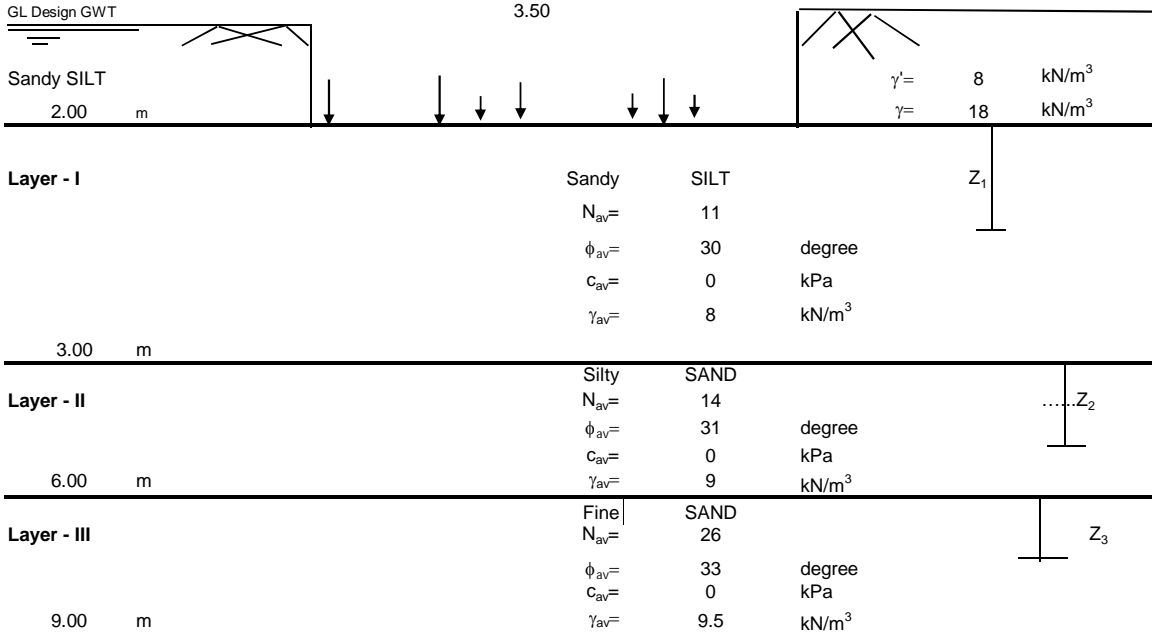
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for clay	
	$m_v =$	m^2/kN		$\mu_g =$		
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for sand	IS:8009 (Part I)
	$C = 1.5 \cdot (C_{kd}/p_o) =$	172.8		$C_{kd}/N =$	250 KN/m ²	2nd layer I = 0.28
	$p_o =$	71.625	$p =$	105.0		
	Rigidity factor = 0.8			Depth Factor, $d_r =$	0.84	
	δ_2 (mm) =	8.69				

Total settlement = 34.26 mm

Allowable Bearing capacity for 25mm settlement = 79.2 KPa

Calculation for Bearing Capacity at Minor Bridge CH 24+920

Footing Size: 5.5x3.5 m
 Depth : 2.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q - 1) s_q d_q i_q + 0.5 B \gamma N_s \gamma' d_v i_{vw}) / FS$$

FS =	2.5	w =	0.5		
N_c =	16	N_q =	7.25	N_v =	7.59 Local shear failure
S_c =	1.127	S_q =	1.127	S_v =	0.75
dc =	$1 + 0.2 * (D/B) * \tan(45 + \phi/2) =$		1.20		
dq = d_v =	$1 + 0.1 * (D/B) * \tan(45 + \phi/2)$		1.10		
ic = iq =	$(1 - \alpha/90)^2 =$	1.00	ig = $(1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$		128.9			kPa

Design Bearing Capacity = **128 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor} (0.8)$$

for clay }
 for sand }
 1st layer I = 1

m_v =		H_g =	
$C = 1.5 * (C_{kd} / p_o) =$	550.0	$C_{kd} / N =$	200 KN/m ²
$p_o =$	6	$p =$	128.0 kPa
Rigidity factor = 0.8		Depth Factor, $d_f =$	0.86
$\delta_1 (\text{mm}) =$	3.89		

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v =$		$\mu_g =$	for clay
	m2/kN		

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) =$	112.0	$C_{kd}/N =$	for sand	IS:8009 (Part I)
			200 KN/m ²	2nd layer I = 0.6
$p_o =$	37.5	$p =$	128.0	kPa
Rigidity factor =	0.8	Depth Factor, $d_f =$		0.86

δ_2 (mm) = 20.54

Settlement for Layer-III

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v =$		$\mu_g =$	for clay
	m2/kN		

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) =$	149.4	$C_{kd}/N =$	for sand	IS:8009 (Part I)
			250 KN/m ²	
$p_o =$	65.25	$p =$	128.0	kPa
Rigidity factor =	0.8	Depth Factor, $d_f =$		0.86

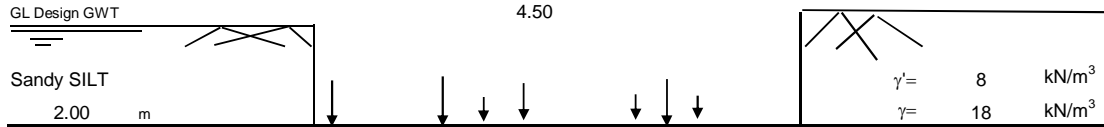
δ_3 (mm) = 4.96

Total settlement = 29.38 mm

Allowable Bearing capacity for 25mm settlement = 109.7 KPa

Calculation for Bearing Capacity at Minor Bridge CH 25+760

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Layer	Soil Type	N_{av}	ϕ_{av}	c_{av}	γ_{av}	Depth
Layer - I	Sandy SILT	10	30 degree	0 kPa	8 kN/m ³	3.00 m
Layer - II	Silty SAND	17	31 degree	0 kPa	9 kN/m ³	6.00 m
Layer - III	Fine SAND	26	33 degree	0 kPa	9.5 kN/m ³	10.00 m

Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = \frac{(cN_c s_c d_c i_c + (\gamma \cdot D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma w})}{FS}$$

FS = 2.5	w = 0.5		
$N_c = 16$	$N_q = 7.25$	$N_{\gamma} = 7.59$	Local shear failure
$S_c = 1.164$	$S_q = 1.164$	$S_{\gamma} = 0.67$	
$d_c = 1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) = 1.15$			
$d_q = d_{\gamma} = 1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) = 1.08$			
$i_c = i_q = (1 - \alpha/90)^2 = 1.00$	$i_{\gamma} = (1 - \alpha/\phi)^2 = 1.00$	$\alpha = 0$	
$Q_{\text{safe-I}} = 132.6$ kPa			

Design Bearing Capacity = **132 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$m_v =$	$\mu_g =$	for clay	} 1st layer I = 1
$C = 1.5 \cdot (C_{kd}/p_o) = 500.0$	$C_{kd}/N = 200$ KN/m ²	for sand	
$p_o = 6$	$p = 132.0$ kPa		
Rigidity factor = 0.8	Depth Factor, $d_r = 0.88$		
$\delta_1 (\text{mm}) = 4.41$			

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v =$		$\mu_g =$	for clay
	m2/kN		

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) =$	136.0	$C_{kd}/N =$	for sand	IS:8009 (Part I)
$p_o =$	37.5	$p =$	200 KN/m ²	2nd layer I = 0.68
Rigidity factor =	0.8	Depth Factor, $d_f =$	0.88	

δ_2 (mm) = 18.98

Settlement for Layer-III

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$m_v =$		$\mu_g =$	for clay
	m2/kN		

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{kd}/p_o) =$	139.3	$C_{kd}/N =$	for sand	IS:8009 (Part I)
$p_o =$	70	$p =$	250 KN/m ²	3RD layer I = 0.28
Rigidity factor =	0.8	Depth Factor, $d_f =$	0.88	

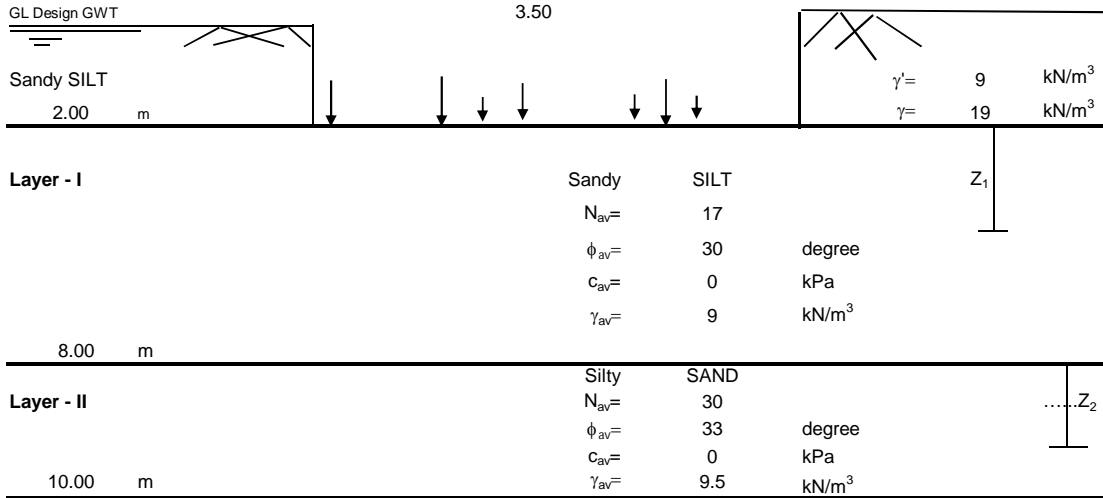
δ_3 (mm) = 8.57

Total settlement = 31.96 mm

Allowable Bearing capacity for 25mm settlement = 103.7 KPa

Calculation for Bearing Capacity at Minor Bridge CH 26+530

Footing Size: 5.5x3.5 m
 Depth : 2.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 29° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma' D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_s s_\gamma d_\gamma i_\gamma) / FS$$

FS =	2.5	w =	0.5		
N_c =	16	N_q =	7.25	N_γ =	7.59
S_c =	1.127	S_q =	1.127	S_γ =	0.75
d_c =	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.20		
$d_q = d_\gamma =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2) =$		1.10		
$i_c = i_q =$	$(1 - \alpha/90)^2 =$		1.00	$\alpha = 0$	
$Q_{\text{safe-I}} =$	137.2	kPa			

Design Bearing Capacity = **137 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor} (0.8)$$

$m_v =$ m²/kN

$\mu_g =$

for clay

$$\delta (\text{mm}) = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor} (0.8)$$

$C = 1.5 \cdot (C_{kd}/p_o) =$

175.9

$C_{kd}/N =$

200 KN/m²

1st layer I = 0.52

$p_o =$ 29

$p =$ 137.0

kPa

Rigidity factor = 0.8

Depth Factor, $d_r =$

0.86

$\delta_1 (\text{mm}) = 29.11$

Settlement for Layer-II

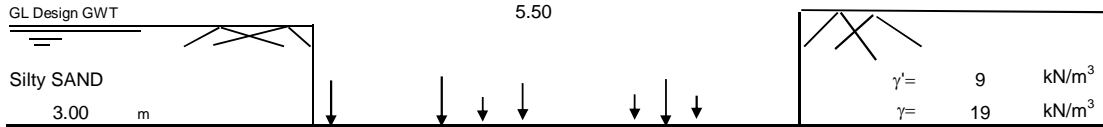
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for clay
	$m_v =$	m^2/kN		$\mu_g =$	
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$				for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	110.4		$C_{kd}/N =$	200 KN/m ²
	$p_o =$	81.5	$p =$	137.0	IS:8009 (Part I)
	Rigidity factor =	0.8		Depth Factor, $d_r =$	2nd layer I = 0.2
					0.86
	δ_2 (mm) =	3.61			

Total settlement = 32.72 mm

Allowable Bearing capacity for 25mm settlement = 104.8 KPa

Calculation for Bearing Capacity at Major Bridge CH 27+290

Footing Size: 5.5x5.5 m
 Depth : 3.00 m



Layer - I	SANDY silt		Z ₁
	N _{av} = 18		
	φ _{av} = 31 degree		
	c _{av} = 0 kPa		
	γ _{av} = 9 kN/m ³		
12.00 m			
Layer - II	silty SAND		Z ₂
	N _{av} = 39		
	φ _{av} = 33 degree		
	c _{av} = 0 kPa		
	γ _{av} = 9.5 kN/m ³		
14.00 m			
Layer - III	Silty SAND		Z ₃
	N _{av} = 60		
	φ _{av} = 34 degree		
	c _{av} = 0 kPa		
	γ _{av} = 10 kN/m ³		
30.00 m			

Safe Bearing Capacity from Shear Failure

Design φ= 31 degree

For Layer - I

As φ is 31° intermediate shear failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS= 2.5	w= 0.5		
N _c = 33.34	N _q = 21.38	N _γ = 27.54	General shear failure
N _c = 17.19	N _q = 8.1	N _γ = 7.59	Local shear failure
N _c = 19.88	N _q = 10.31	N _γ = 10.92	Intermediate condition
S _c = 1.300	S _q = 1.200	S _γ = 0.80	
dc= 1+0.2*(D/B)*tan(45+φ/2)= 1.19			
dq=dγ= 1+0.1*(D/B)*tan(45+φ/2)= 1.10			
ic=iq= (1-α/90) ² = 1.00	ig= (1-α/φ) ² = 1.00	α=0	
Q_{safe-I}= 326.8 kPa			

Design Bearing Capacity= **300 kPa**

Settlement for Layer - I

δ (mm) = m_v*H*Δp*μ_g*d_r*Rigidity Factor (0.8)

m_v= m²/kN μ_g= for clay

δ (mm)= [2.303*(H/C)*log₁₀((p_o+Δp)/p_o)]*d_r*Rigidity Factor (0.8)

C=1.5*(C_{kα}/p_o)= 155.2 C_{kα}/N= 250 kN/m² for sand

p_o= 43.5 p= 300.0 kPa 1st layer I = 0.42

Rigidity factor= 0.8 Depth Factor, d_r= 0.76

δ₁ (mm)= 47.96

Settlement for Layer-II

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$
 $m_v = 0.00013$
 $\mu_g = 0.7$

for clay

$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$
 $C = 1.5 \cdot (C_{kd}/p_o) =$

149.4

 $C_{kd}/N =$
300 KN/m²

IS:8009 (Part I)

2nd layer I = 0.152

 $p_o = 117.5$
 $p =$

300.0

kPa

Rigidity factor = 0.8

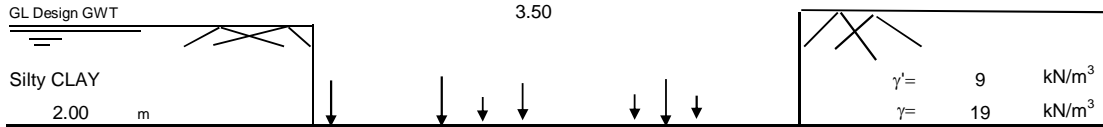
Depth Factor, $d_f =$

0.76

 $\delta_2 \text{ (mm)} = 2.67$
 $\delta_3 \text{ (mm)} = 2.19$
Total settlement = 50.63 mm
Allowable Bearing capacity for 25mm settlement =
161.3 KPa

Calculation for Bearing Capacity at Minor Bridge CH 27+820

Footing Size: 5.5x3.5 m
 Depth : 2.00 m



Layer	Soil Type	Thickness (m)	N_{av}	ϕ_{av} (degree)	c_{av} (kPa)	C_{design}	γ_{av} (kN/m ³)	Depth (m)
Layer - I	Silty CLAY	3.00	5	0	45	56.25	9	Z ₁
			Mv (Lab) =	Mv (Correl.) =	Design Mv =	Cdesign = (C1*d1 + C2*d2)/2		
			0.26 m2/MN	0.13 m2/MN	0.26 m2/MN			
Layer - II	Silty CLAY	6.00	12	0	60		9	Z ₂
			Mv (Lab) =	Mv (Correl.) =	Design Mv =			
			0.20 m2/MN	0.13 m2/MN	0.16 m2/MN			
Layer - III	Sandy SILT	9.00	13	31	0		9	Z ₃
			N_{av}	ϕ_{av} (degree)	c_{av} (kPa)		γ_{av} (kN/m ³)	
			13	31	0		9	

Safe Bearing Capacity from Shear Failure

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_{i,c} + (\gamma' D)(N_q) s_q d_{q,i_q} + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma,i_{\gamma}}) / FS$$

FS=	2.5	w=	0.5		
N_c =	5.14	N_q =	1	N_{γ} =	0.00
S_c =	1.127	S_q =	1.127	S_{γ} =	0.75
dc=	1+0.2*(D/B)*tan(45+φ/2)=		1.11		
dq=dγ=	1+0.1*(D/B)*tan(45+φ/2)		1.06		
ic=iq=	(1-α/90) ² =		1.00	ig=	(1-α/φ) ² =
					1.00
Q_{safe-I} =	145.2	kPa			α=0

Design Bearing Capacity= **145 kPa**

Settlement for Layer - I

δ (mm) = $m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor} (0.8)$ for clay

$m_v = 0.00018$ $\mu_g = 0.7$

δ (mm) = $[2.303 * (H/C) * \log_{10}((p_o + \Delta p) / p_o)] * d_r * \text{Rigidity Factor} (0.8)$ for sand

$C = 1.5 * (C_{kd} / p_o) = 0.0$ $C_{kd} / N =$ $1st\ layer\ I = 1$

$p_o = 6.5$ $p = 145.0$ kPa

Rigidity factor = 0.8 Depth Factor, $d_r = 0.86$

δ_1 (mm) = 12.57

Settlement for Layer-II

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v = 0.00016$ m²/kN $\mu_g = 0.7$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) = 0.0$ $C_{kd}/N =$ KN/m² 2nd layer I = 0.6
 $p_o = 40.5$ $p = 145.0$ kPa
 Rigidity factor = 0.8 Depth Factor, $d_f = 0.86$

δ_2 (mm) = 20.11

Settlement for Layer-III

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$ for clay

$m_v =$ m²/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) = 72.2$ $C_{kd}/N =$ 250 KN/m² 3RD layer I = 0.22
 $p_o = 67.5$ $p = 145.0$ kPa
 Rigidity factor = 0.8 Depth Factor, $d_f = 0.86$

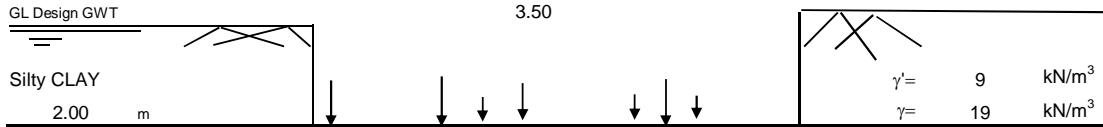
δ_3 (mm) = 11.06

Total settlement = 43.74 mm

Allowable Bearing capacity for 25mm settlement = 83.0 KPa

Calculation for Bearing Capacity at Minor Bridge CH 28+660

Footing Size: 5.5x3.5 m
 Depth : 2.00 m



Layer	Soil Type	Thickness (m)	N_{av}	ϕ_{av} (degree)	c_{av} (kPa)	γ_{av} (kN/m ³)	Depth Marker
Layer - I	Silty CLAY	3.00	7	0	42	9	Z ₁
			M_v (Lab) = 0.18 m ² /MN				
			M_v (Correl.) = 0.19 m ² /MN				
			Design M_v = 0.185 m ² /MN				
Layer - II	Sandy SILT	6.00	14	31	0	9.5	Z ₂
Layer - III	silty SAND	9.00	25	33	0	10	Z ₃

Safe Bearing Capacity from Shear Failure

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma \cdot D)(N_q) s_q d_q i_q + 0.5B\gamma N_s s_\gamma d_\gamma i_\gamma) / FS$$

FS=	2.5	w=	0.5			
N_c =	5.14	N_q =	1	N_γ =	0.00	Local shear failure
S_c =	1.127	S_q =	1.127	S_γ =	0.75	
dc=	1+0.2*(D/B)*tan(45+φ/2)=		1.11			
dq=dγ=	1+0.1*(D/B)*tan(45+φ/2)		1.06			
ic=iq=	(1-α/90) ² = 1.00		ig= (1-α/φ) ² = 1.00		α=0	
Q_{safe-I}=	108.4		kPa			

Design Bearing Capacity= **108 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$m_v = 0.000185 \text{ m}^2/\text{kN}$ $\mu_g = 0.7$ for clay }
 $\delta (\text{mm}) = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$
 $C = 1.5 \cdot (C_{kd}/p_o) = 323.1$ $C_{kd}/N = 200 \text{ KN/m}^2$ for sand }
 $p_o = 6.5$ $p = 108.0$ kPa Depth Factor, $d_r = 0.86$ 1st layer I = 1
δ₁ (mm) = 9.62

Settlement for Layer-II

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$

$m_v =$		$\mu_g = 0.7$	for clay
	m2/kN		

$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$

$C = 1.5 \cdot (C_{kd}/p_o) =$	101.8	$C_{kd}/N =$	for sand	IS:8009 (Part I)
$p_o =$	41.25	$p =$	200 KN/m ²	2nd layer I = 0.592
Rigidity factor =	0.8	Depth Factor, $d_f =$	0.86	

$\delta_2 \text{ (mm)} = 18.98$

Settlement for Layer-III

$$\delta \text{ (mm)} = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$

$m_v =$		$\mu_g =$	for clay
	m2/kN		}

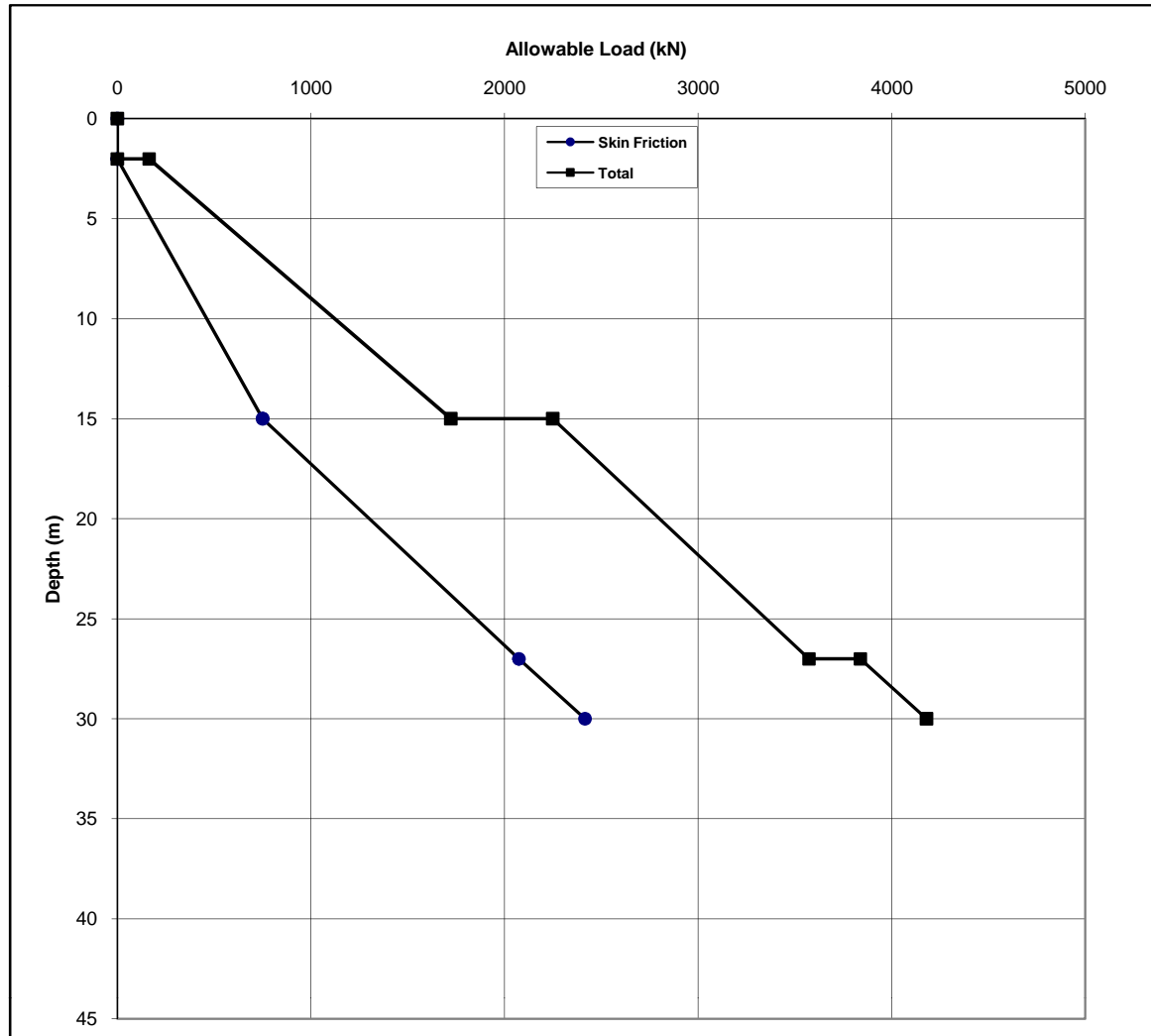
$$\delta \text{ (mm)} = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$$

$C = 1.5 \cdot (C_{kd}/p_o) =$	106.4	$C_{kd}/N =$	for sand	IS:8009 (Part I)
$p_o =$	70.5	$p =$	200 KN/m ²	3RD layer I = 0.22
Rigidity factor =	0.8	Depth Factor, $d_f =$	0.86	

$\delta_3 \text{ (mm)} = 5.64$

Total settlement = 34.23 mm

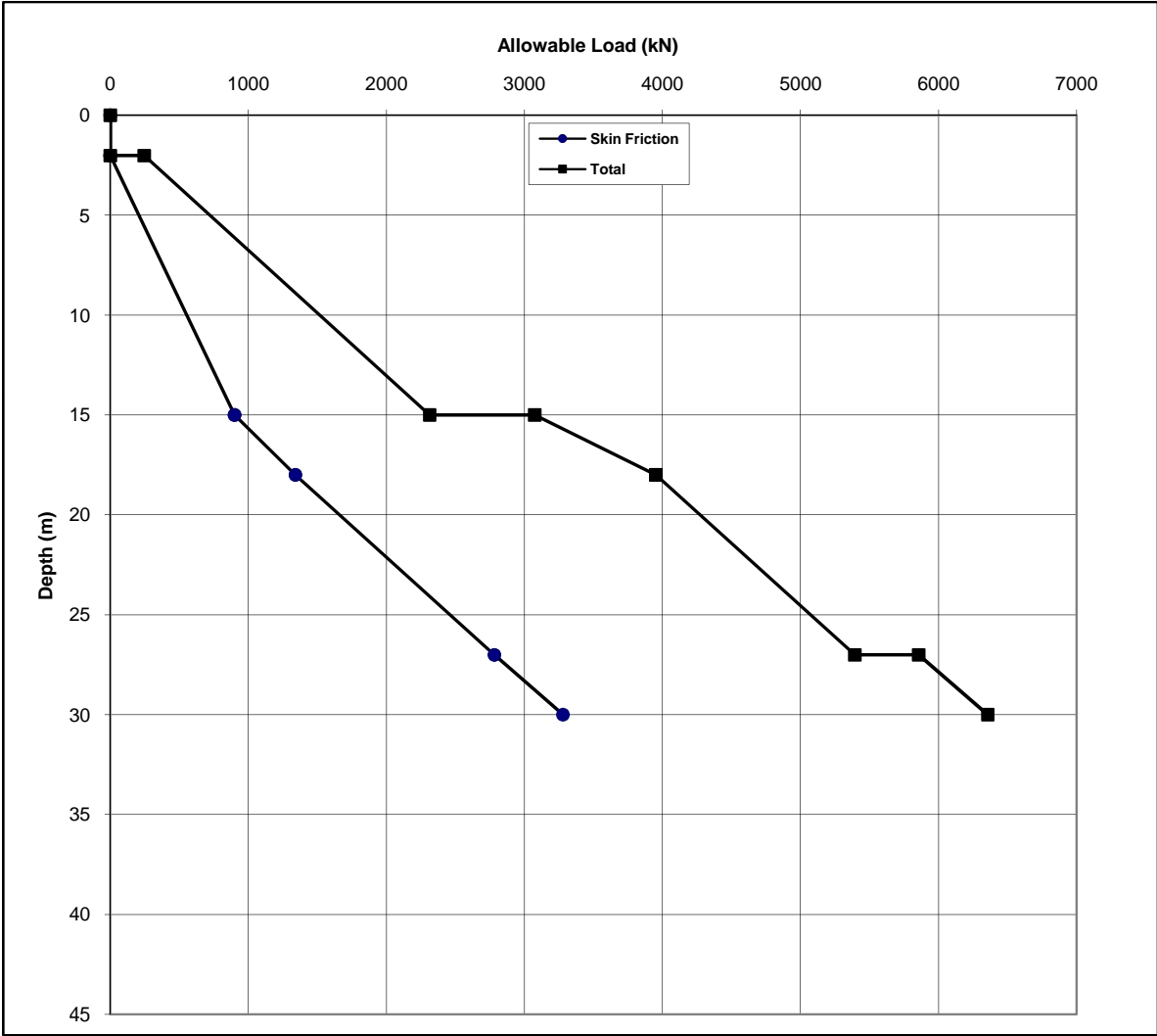
Allowable Bearing capacity for 25mm settlement = 79.2 KPa



Allowable Pile Capacity at Major Bridge Ch. 28+880

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 28+880

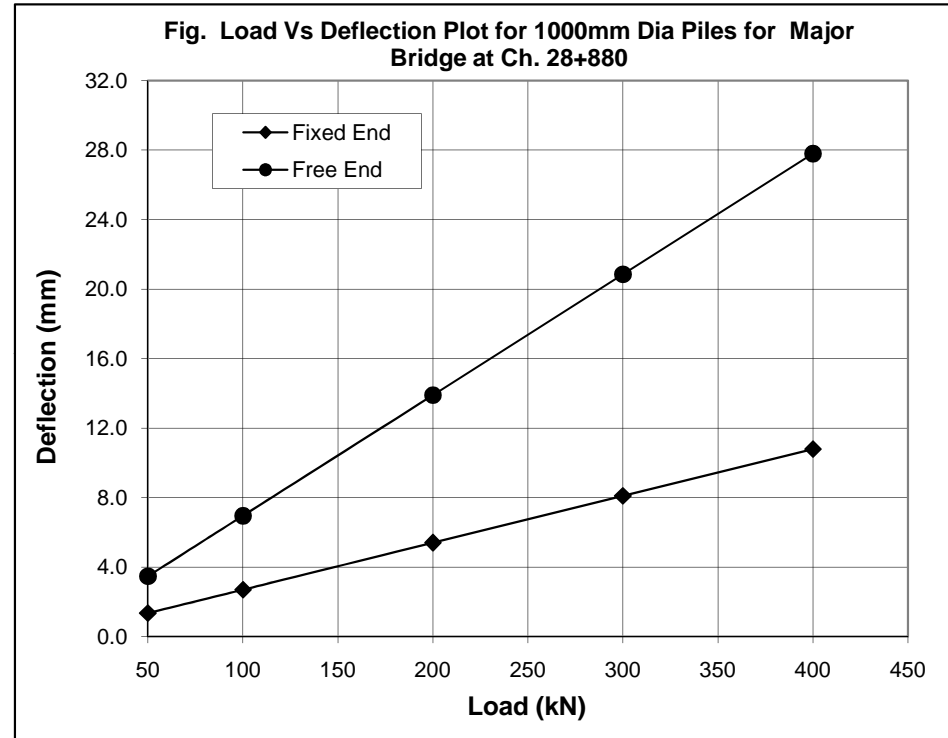
Pile Type=	Bored	Factor of Safety	
Pile Dia (mm)=	1200	End Bearing =	2.5
		Skin Friction =	2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 28+880 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.35	3.47
100	2.70	6.95
200	5.39	13.90
300	8.09	20.85
400	10.79	27.80



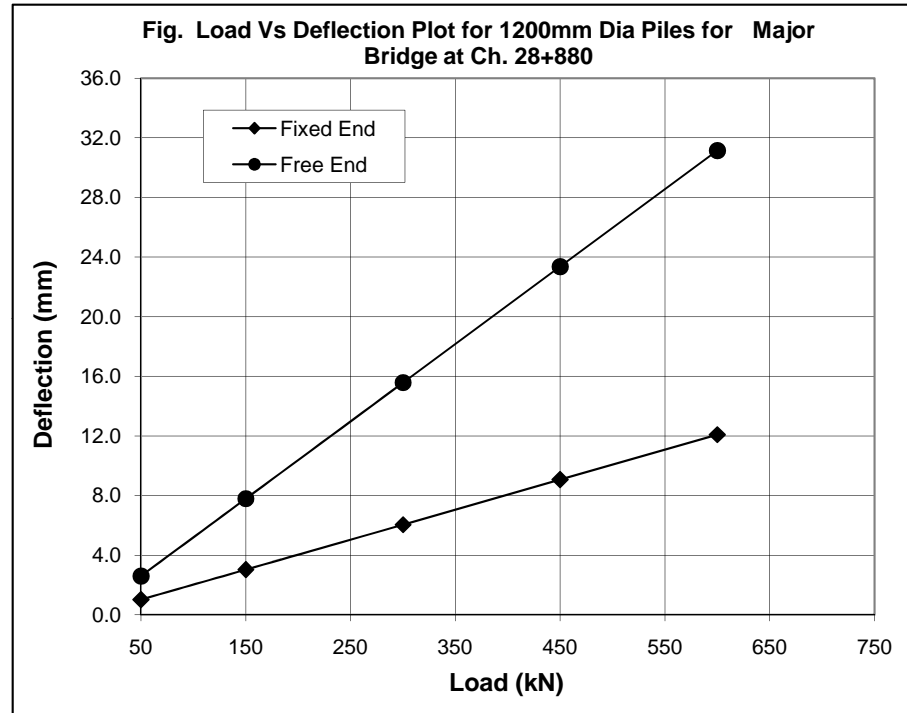
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 370kN (for fixed head condition)
 = 140 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 28+880 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 η_b = 0.280 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²

T= (EI/ηh)^{0.2}
 396.63
 L_f/T = 2.2 Fixed
 L_f (Fixed)= 872.58 cm
 L_f/T = 1.9 Free
 L_f (Free)= 753.59 cm
 L_1 = 0 cm
 d = $\frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $\frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.01	2.60
150	3.02	7.79
300	6.04	15.57
450	9.07	23.36
600	12.09	31.14

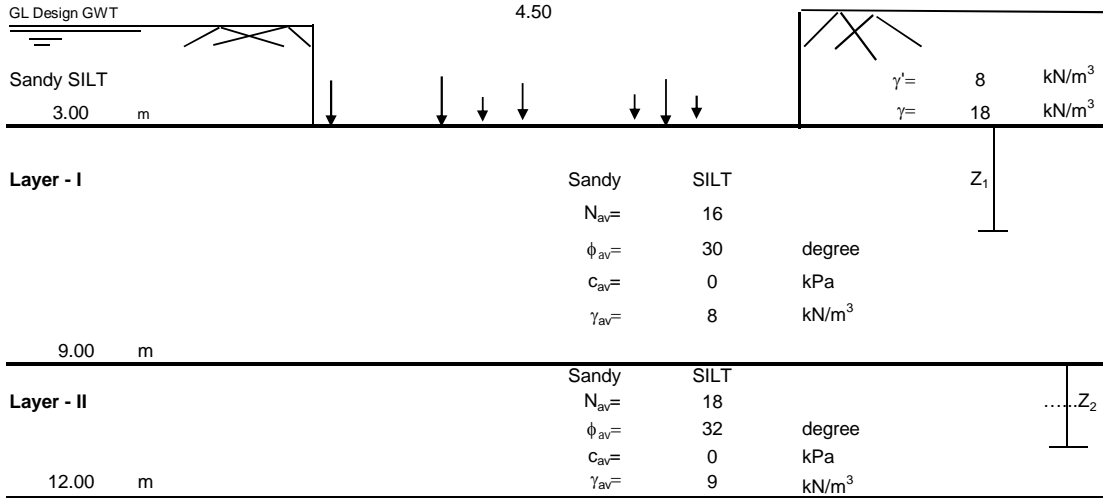


Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 600kN (for fixed head condition)
 = 220kN (for free head condition)

Calculation for Bearing Capacity at Minor Bridge CH 30+780

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_s s_\gamma d_\gamma i_\gamma) / FS$$

FS =	2.5	w =	0.5		
N_c =	16	N_q =	7.25	N_γ =	7.59
S_c =	1.164	S_q =	1.164	S_γ =	0.67
d_c =	$1 + 0.2 * (D/B) * \tan(45 + \phi/2) =$		1.23		
$d_q = d_\gamma =$	$1 + 0.1 * (D/B) * \tan(45 + \phi/2)$		1.12		
$i_c = i_q =$	$(1 - \alpha/90)^2 =$	1.00	$i_\gamma = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$		195.8		kPa	

Design Bearing Capacity = **195 kPa**

Settlement for Layer - I

δ (mm) = $m_v * H * \Delta p * \mu_g * d_r * \text{Rigidity Factor} (0.8)$

$m_v =$ m²/kN $\mu_g =$ for clay

δ (mm) = $[2.303 * (H/C) * \log_{10}((p_o + \Delta p)/p_o)] * d_r * \text{Rigidity Factor} (0.8)$

$C = 1.5 * (C_{k0}/p_o) =$ 222.2 $C_{k0}/N =$ 250 KN/m² for sand

$p_o =$ 27 $p =$ 195.0 kPa 1st layer I = 0.6

Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.84

δ_1 (mm) = 30.37

Settlement for Layer-II

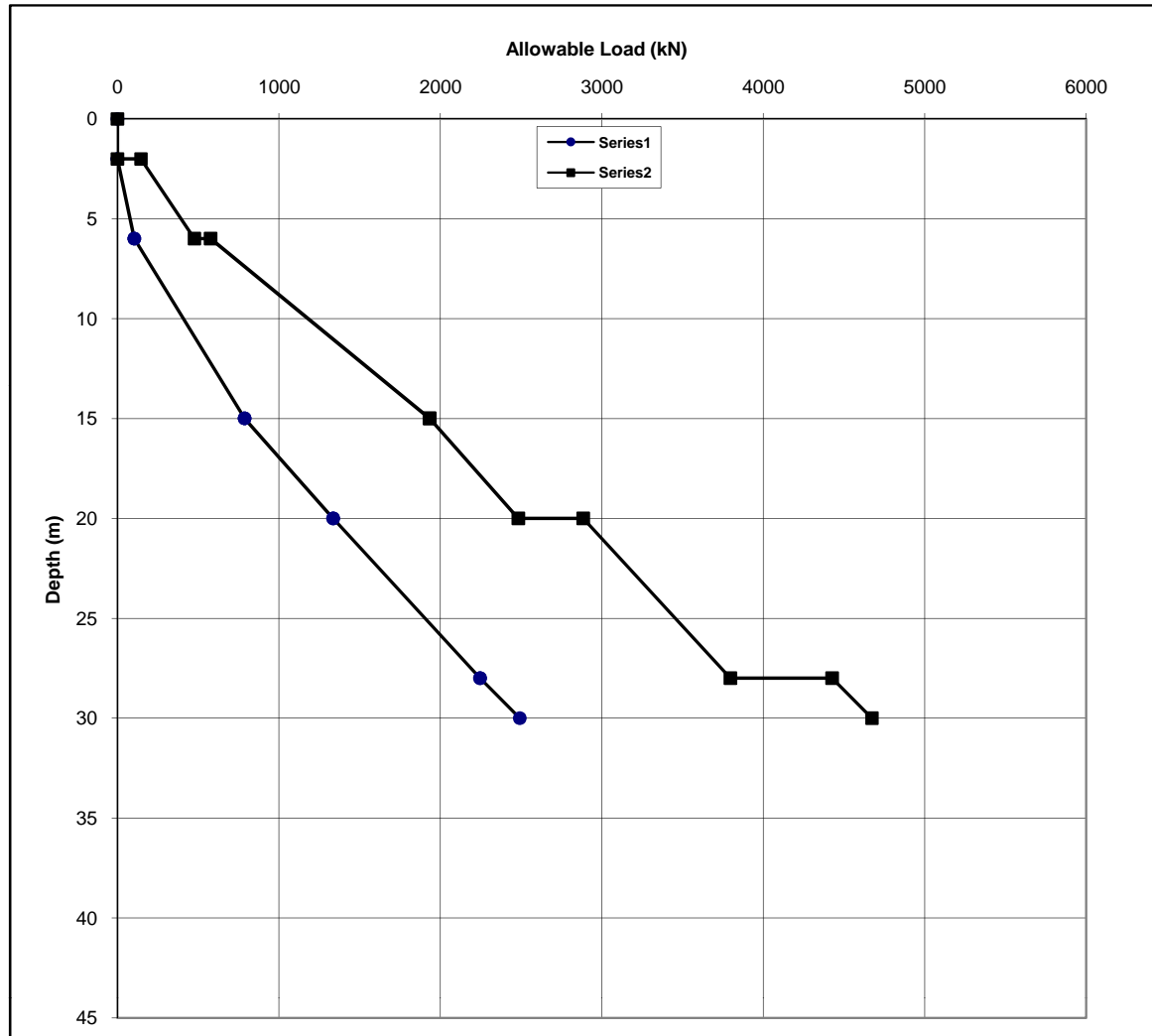
δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ $\mu_g =$ m^2/kN

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) = 78.9$ $C_{kd}/N = 250 \text{ KN/m}^2$ 2nd layer I = 0.2
 $p_o = 85.5$ $p = 195.0$ kPa
 Rigidity factor = 0.8 Depth Factor, $d_r = 0.84$

δ_2 (mm) = 9.60

Total settlement = 39.97 mm

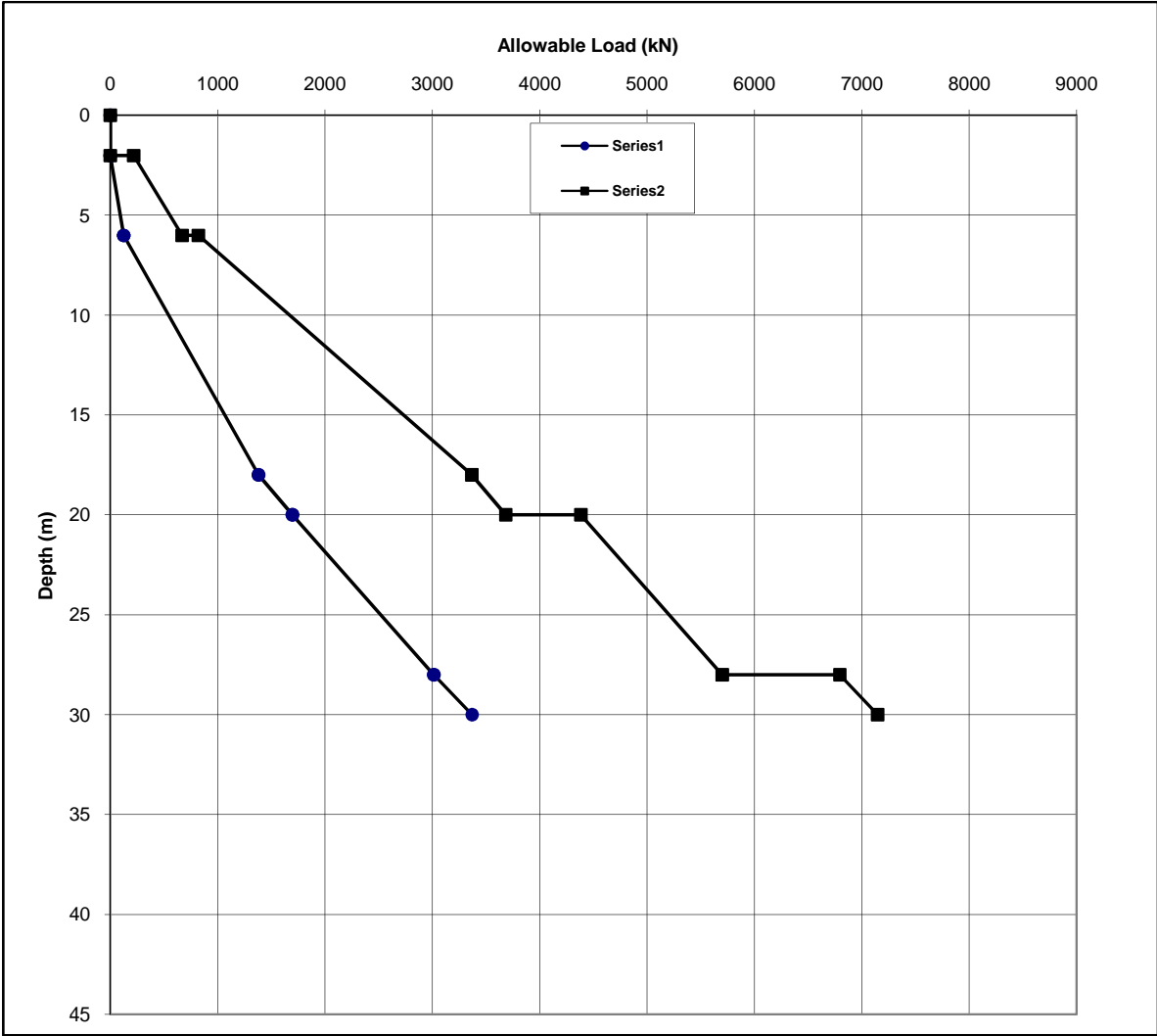
Allowable Bearing capacity for 25mm settlement = 122.5 KPa



Allowable Pile Capacity at Major Bridge Ch. 33+050

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 33+050

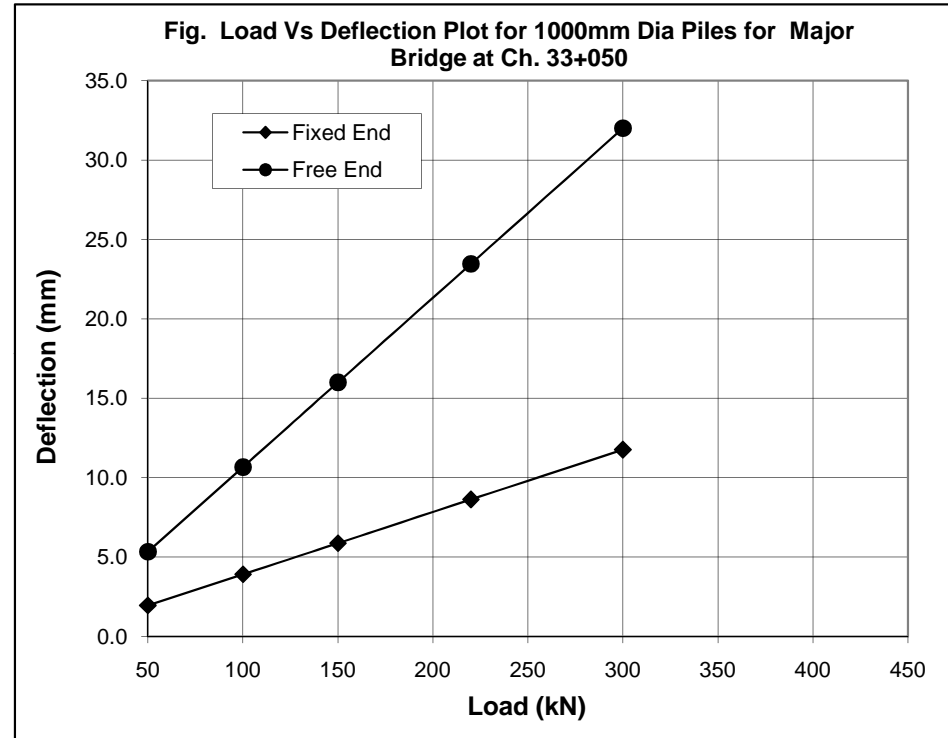
Pile Type=	Bored	Factor of Safety	
Pile Dia (mm)=	1200	End Bearing =	2.5
		Skin Friction =	2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 33+050 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 100 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.96	5.33
100	3.92	10.67
150	5.88	16.00
220	8.62	23.47
300	11.75	32.00



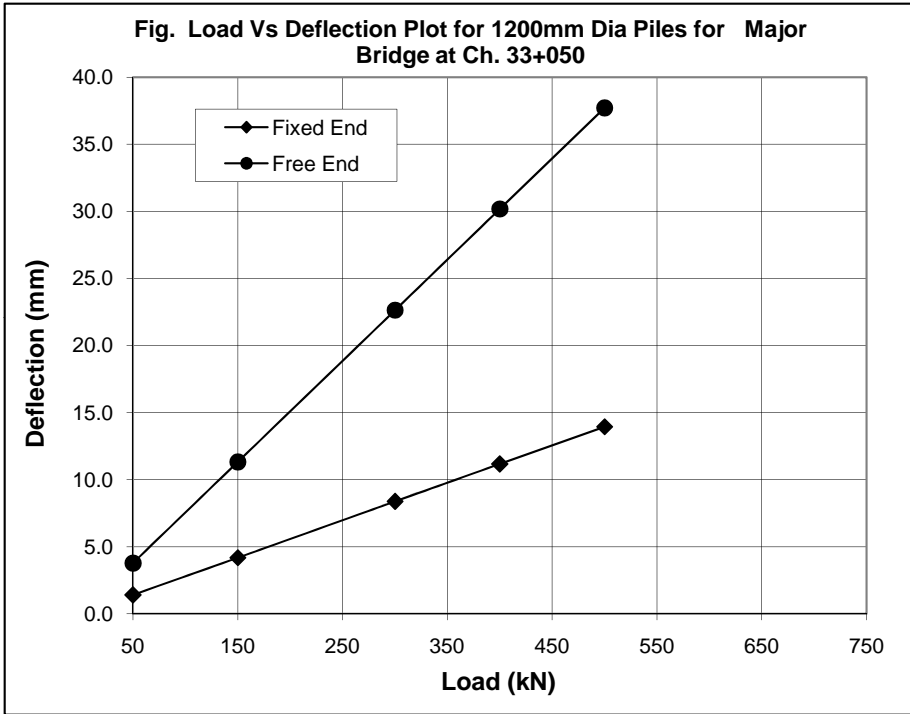
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 250kN (for fixed head condition)
 = 90 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 33+050 (IS: 2911 - PART-1/SEC-2-2010)

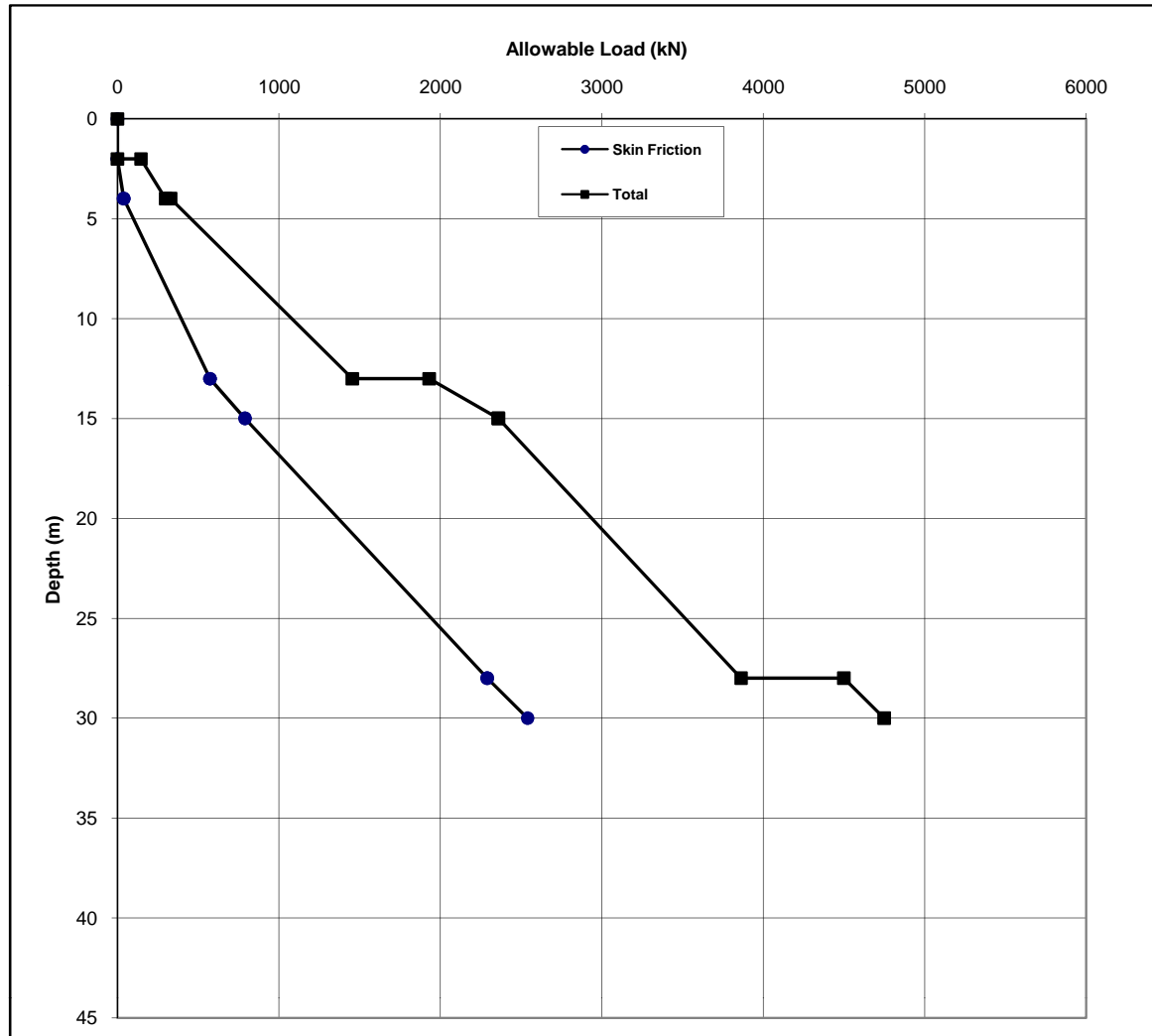
D= 120 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 396.63$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 872.58$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 753.59$ cm
 $L_1 = 100$ cm
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $d = Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.39	3.77
150	4.18	11.32
300	8.37	22.63
400	11.16	30.17
500	13.95	37.72



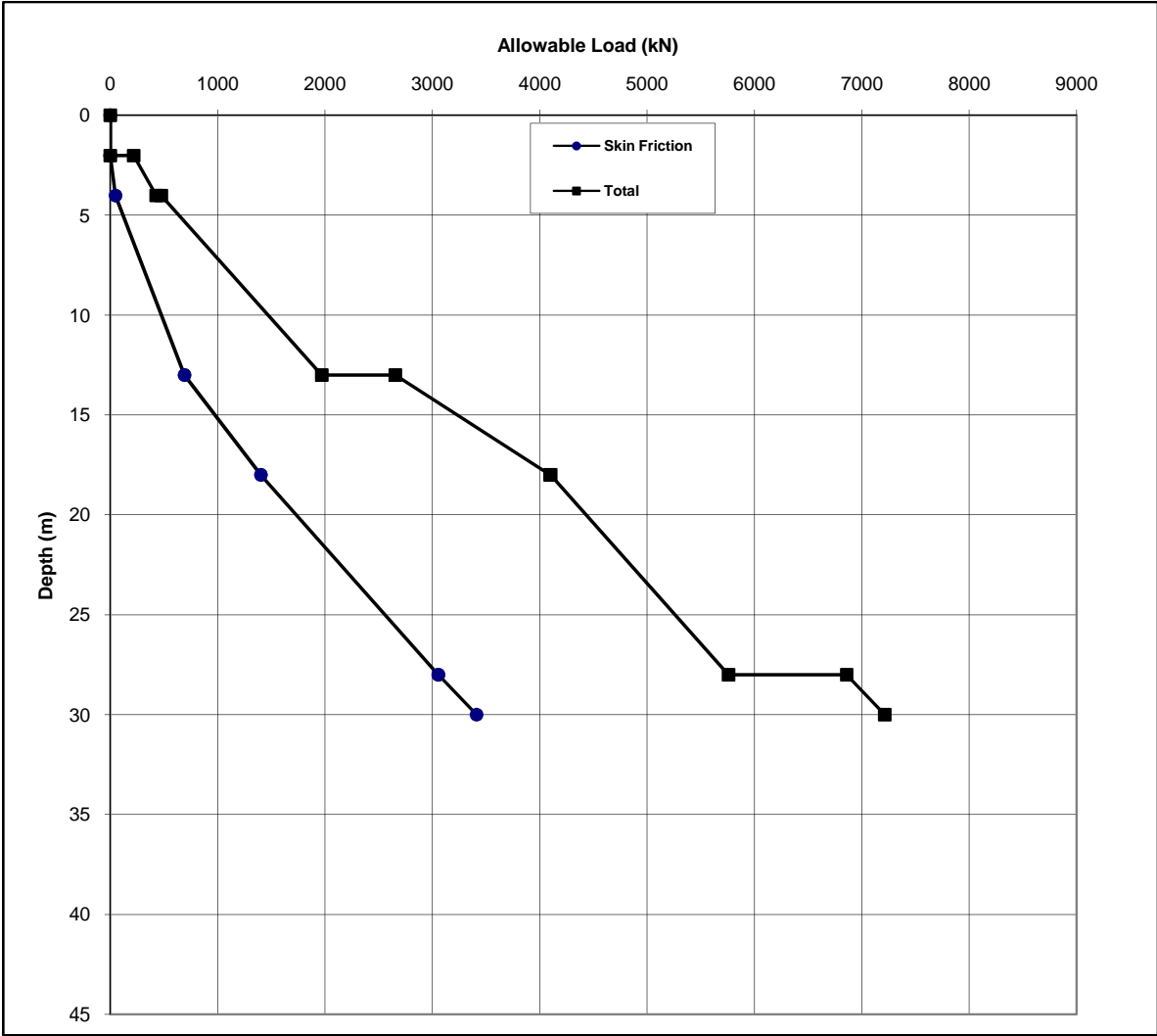
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 420kN (for fixed head condition)
 = 160kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 34+986

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 34+986

Pile Type= Bored
 Pile Dia (mm)= 1200

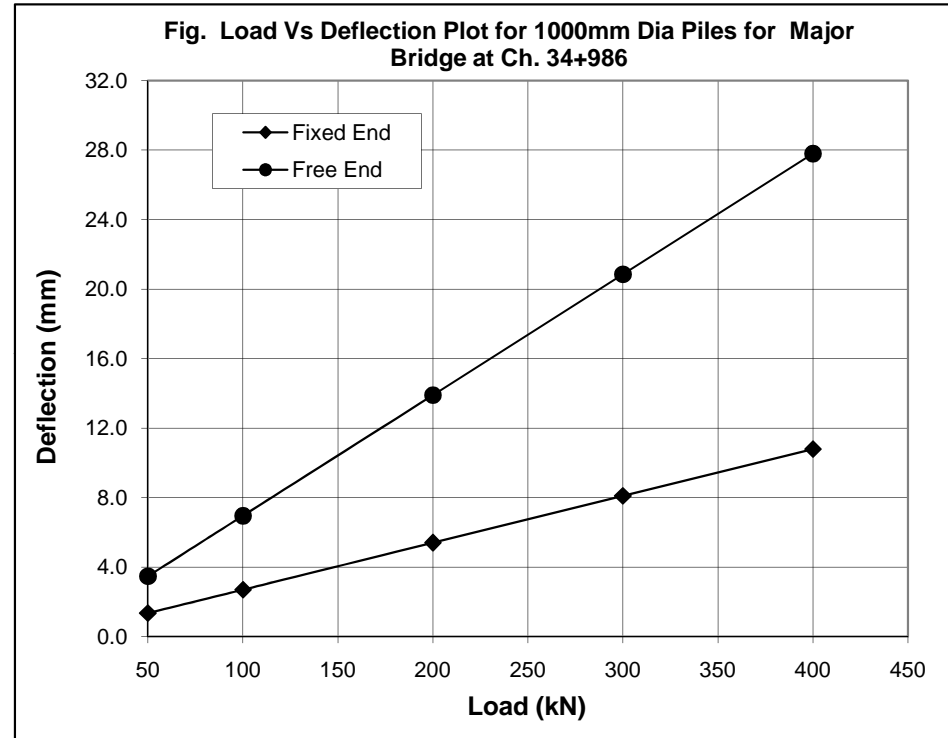
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 34+986 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.35	3.47
100	2.70	6.95
200	5.39	13.90
300	8.09	20.85
400	10.79	27.80



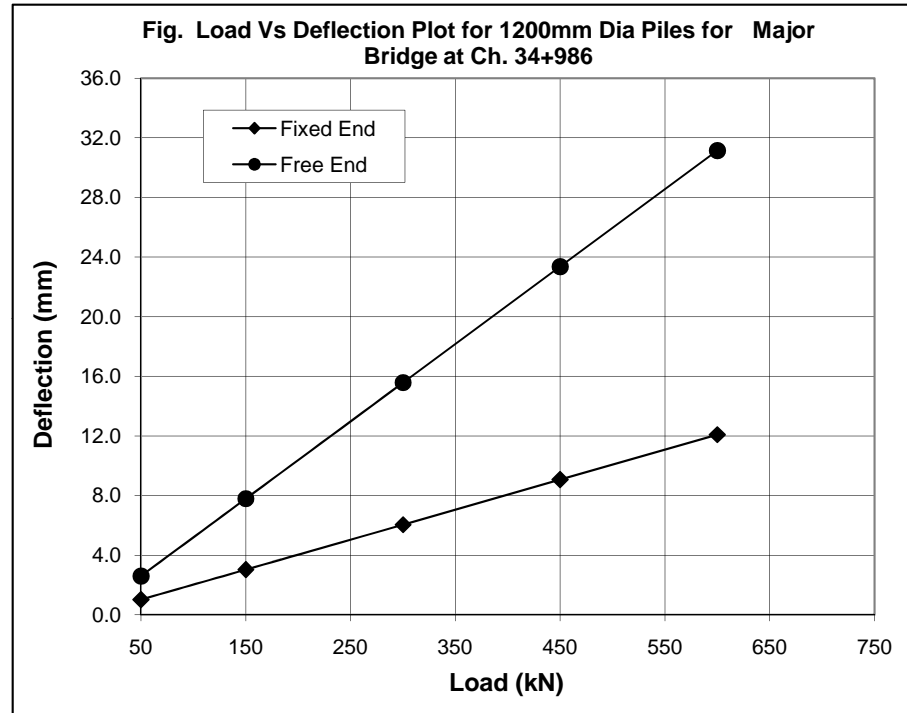
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 370kN (for fixed head condition)
 = 140 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 34+986 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 396.63$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 872.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 753.59 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

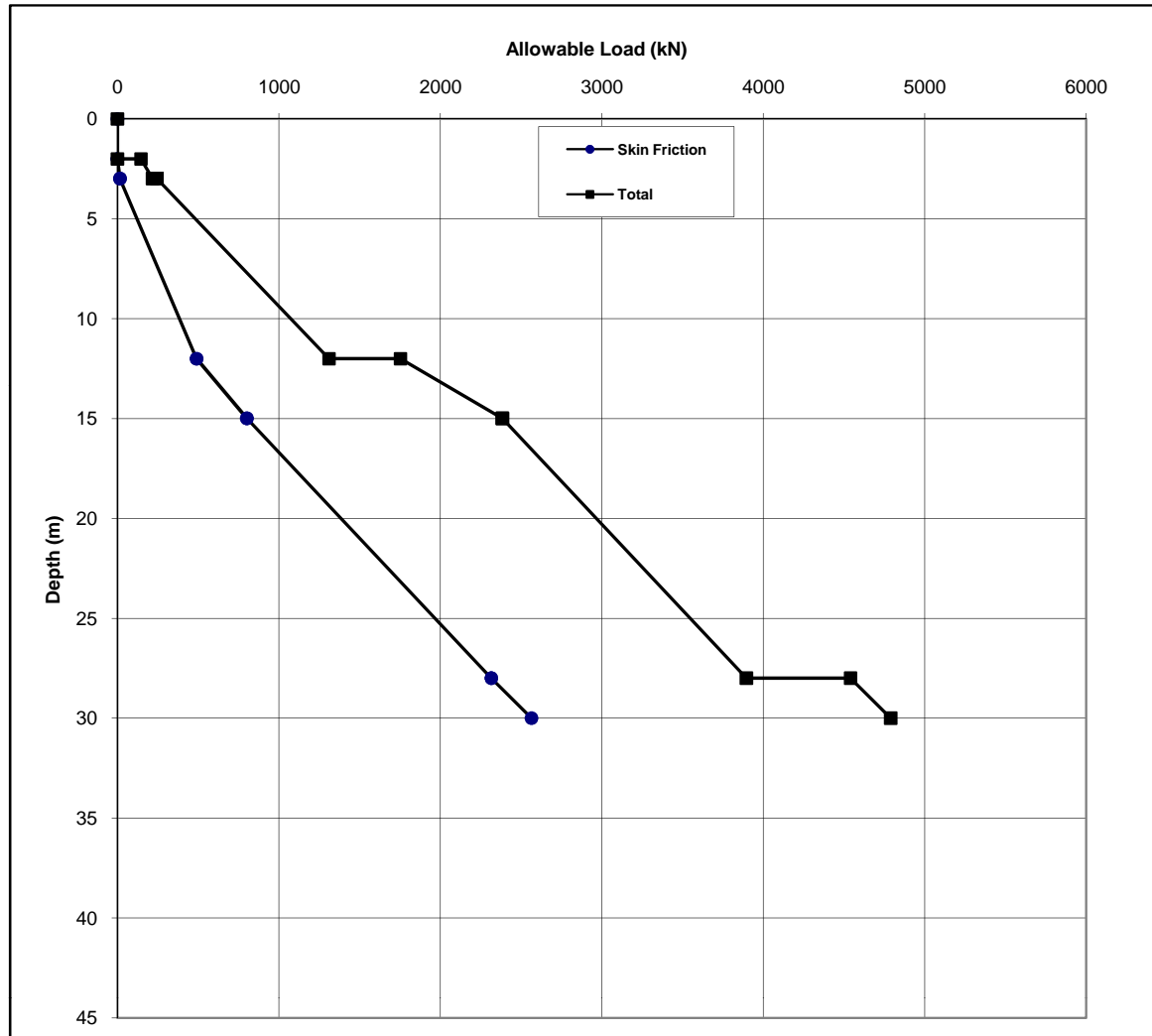
Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.01	2.60
150	3.02	7.79
300	6.04	15.57
450	9.07	23.36
600	12.09	31.14



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 600kN (for fixed head condition)

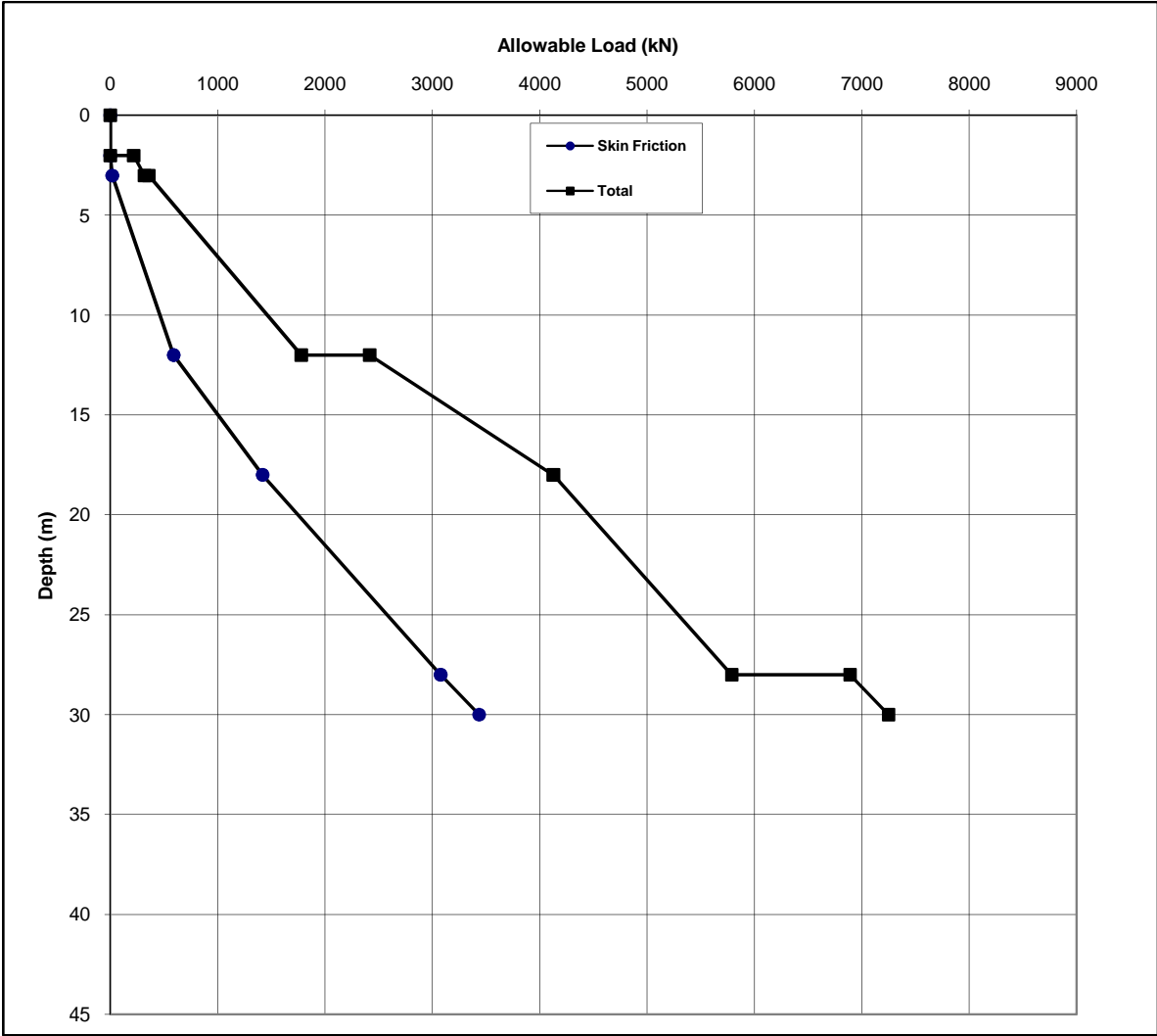
= 220kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 35+549

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 35+549

Pile Type= Bored
 Pile Dia (mm)= 1200

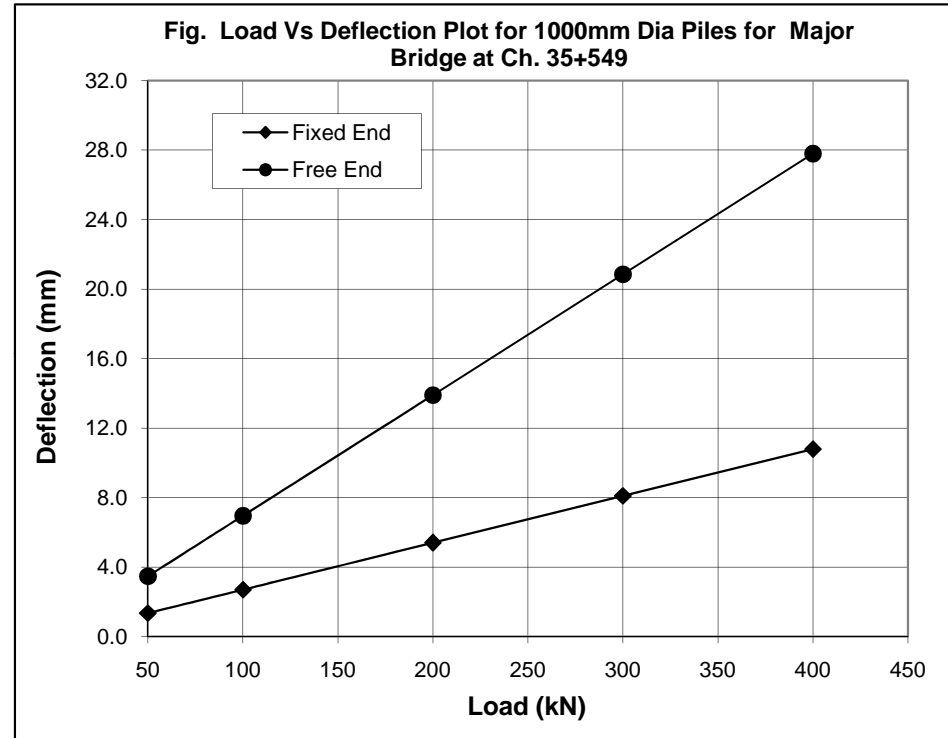
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 35+549 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $d = Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.35	3.47
100	2.70	6.95
200	5.39	13.90
300	8.09	20.85
400	10.79	27.80



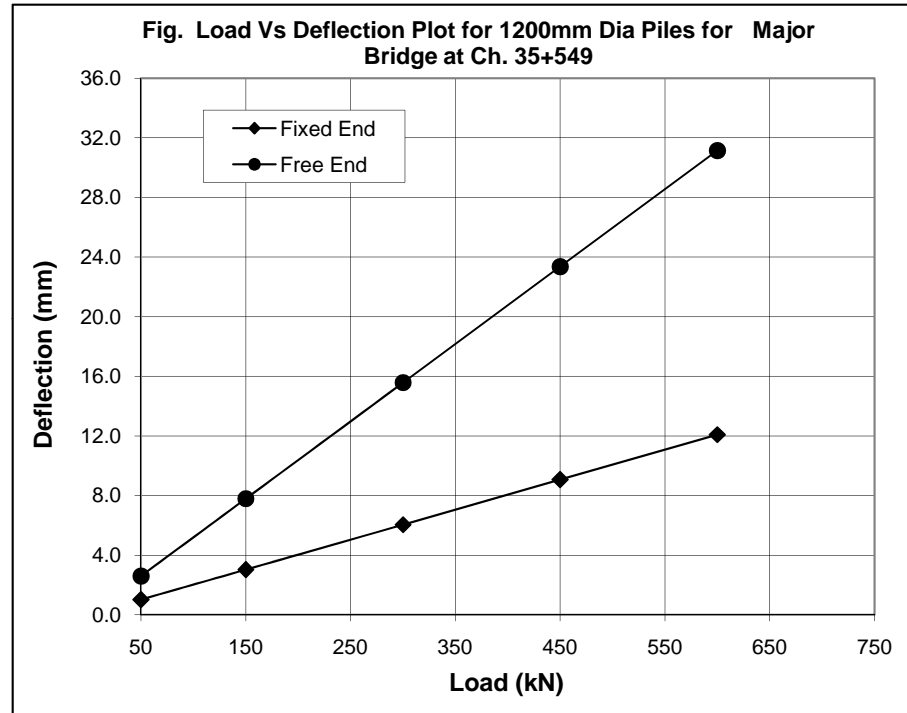
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 370kN (for fixed head condition)
 = 140 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 35+549 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 396.63$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 872.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 753.59 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.01	2.60
150	3.02	7.79
300	6.04	15.57
450	9.07	23.36
600	12.09	31.14

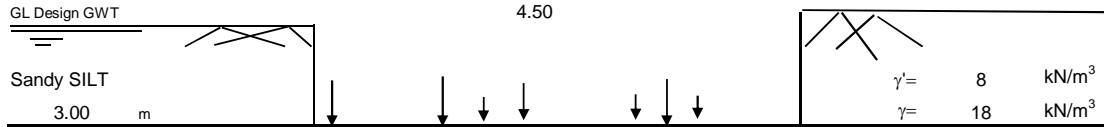


Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 600kN (for fixed head condition)
 = 220kN (for free head condition)

Calculation for Bearing Capacity at Minor Bridge CH 37+360

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Layer	Thickness (m)	Soil Type	N_{av}	ϕ_{av} (degree)	c_{av} (kPa)	γ_{av} (kN/m ³)
Layer - I	5.00	Silty SAND	18	31	0	8
Layer - II	12.00	Silty SAND	21	33	0	9

Safe Bearing Capacity from Shear Failure

Design ϕ = 31 degree

For Layer - I

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma \cdot D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	w =	0.5	$N_{\gamma} =$	27.54	General shear failure
$N_c =$	33.34	$N_q =$	21.38	$N_{\gamma} =$	7.59	Local shear failure
$N_c =$	17.19	$N_q =$	8.1	$N_{\gamma} =$	10.92	Intermediate condition
$N_c =$	19.88	$N_q =$	10.31	$S_{\gamma} =$	0.67	
$S_c =$	1.164	$S_q =$	1.164			
dc =	1 + 0.2 * (D/B) * tan(45 + ϕ /2) =		1.24			
dq = d γ =	1 + 0.1 * (D/B) * tan(45 + ϕ /2) =		1.12			
ic = iq =	(1 - α /90) ² =		1.00	$i_{\gamma} = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$	291.3	kPa				

Design Bearing Capacity = **290.0 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor} (0.8)$$

$m_v =$ m²/kN $\mu_g =$ for clay }
 $\delta (\text{mm}) = [2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor} (0.8)$

$C = 1.5 \cdot (C_{kd}/p_o) =$ 613.6 $C_{kd}/N =$ 250 KN/m² } for sand }
 $p_o =$ 11 $p =$ 290.0 kPa } 1st layer I = 0.952
 Rigidity factor = 0.8 Depth Factor, $d_f =$ 0.84

$\delta_1 (\text{mm}) = 7.14$

Settlement for Layer-II

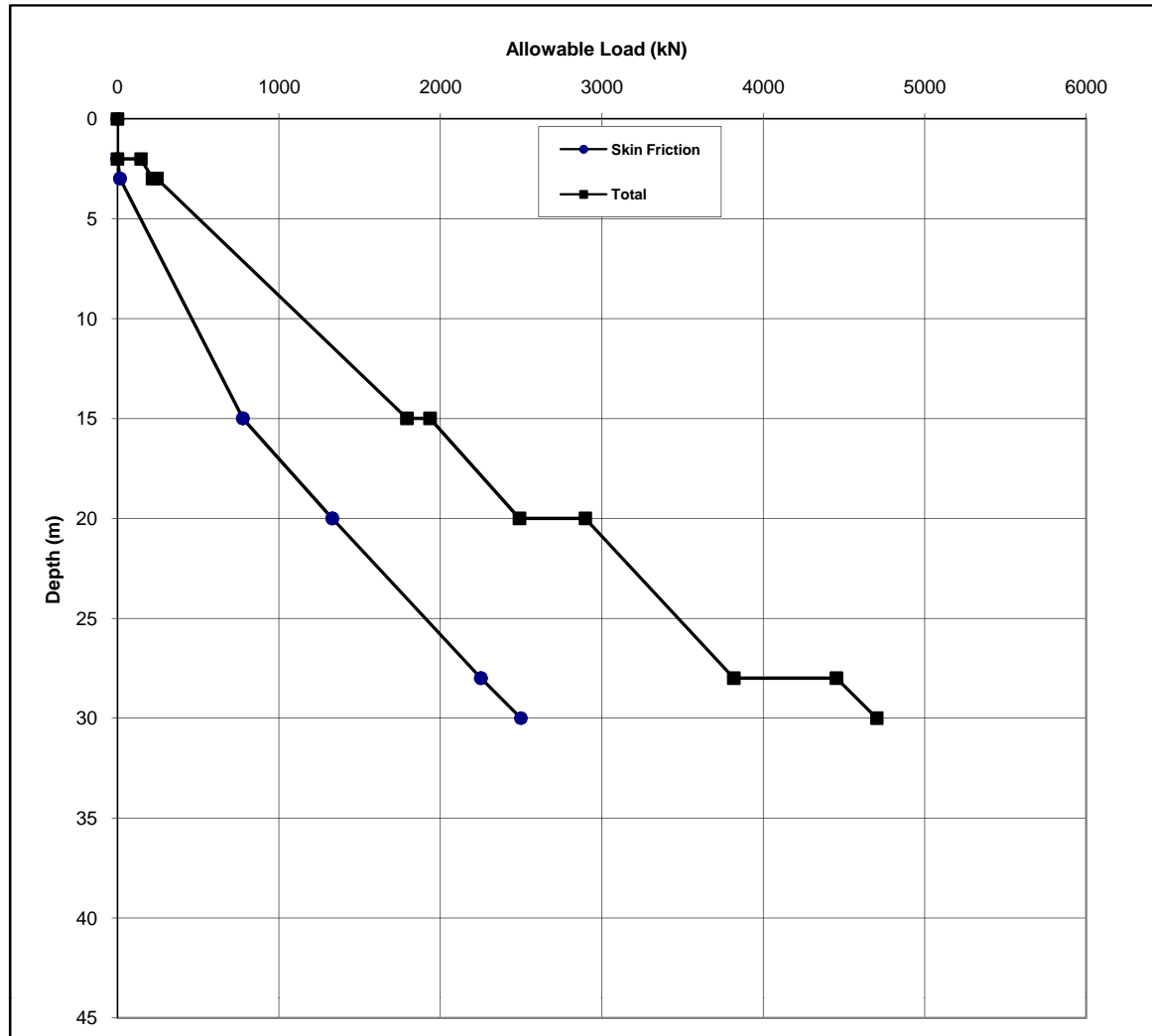
δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for clay
 $m_v =$ m^2/kN $\mu_g =$

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$ for sand IS:8009 (Part I)
 $C = 1.5 \cdot (C_{kd}/p_o) =$ 110.1 $C_{kd}/N =$ 250 kN/m^2 2nd layer I = 0.28
 $p_o =$ 71.5 $p =$ 290.0 kPa
 Rigidity factor = 0.8 Depth Factor, $d_r =$ 0.84

δ_2 (mm) = 32.41

Total settlement = 39.55 mm

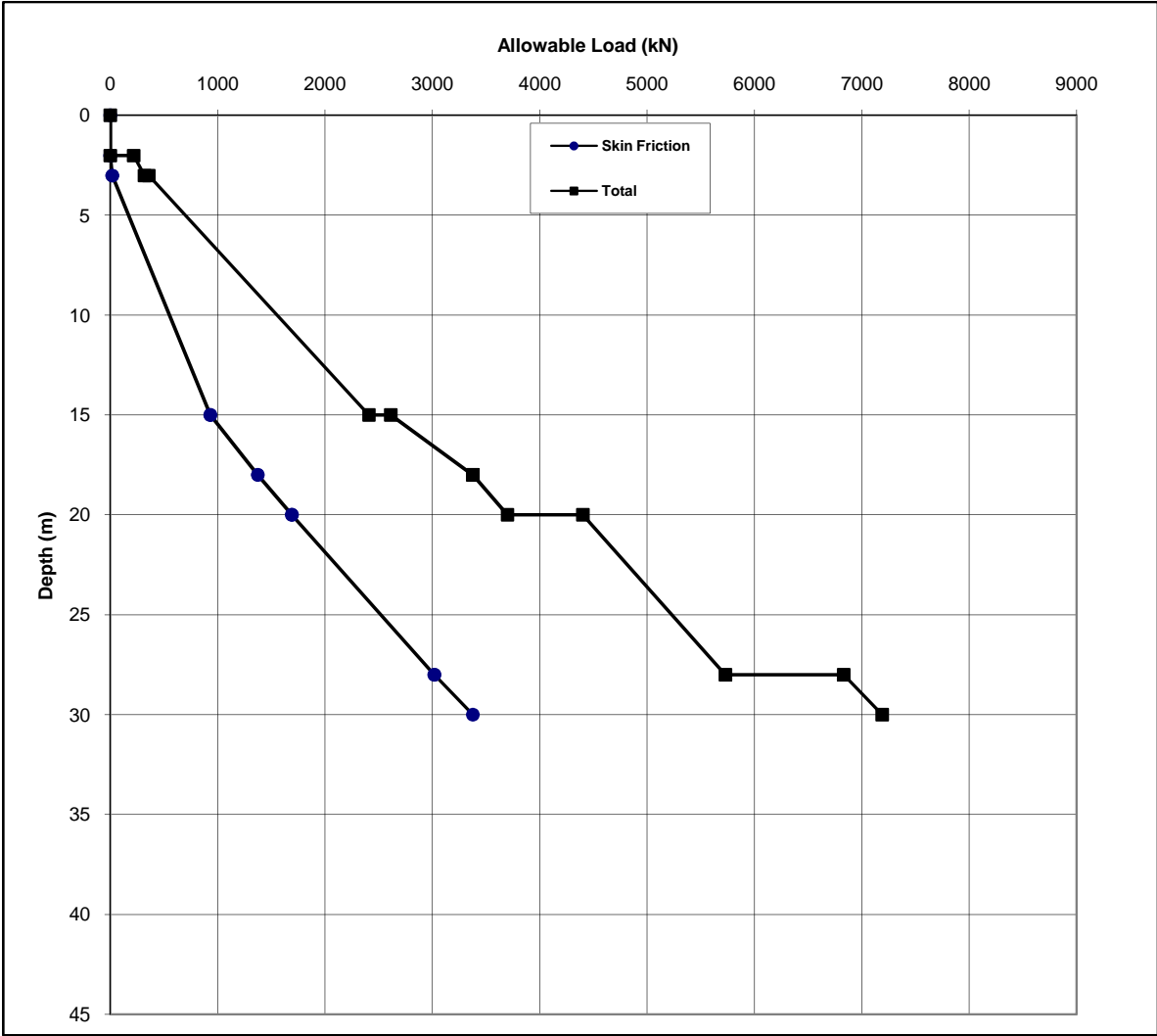
Allowable Bearing capacity for 25mm settlement = 184.1 KPa



Allowable Pile Capacity at Major Bridge Ch. 38+580

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 38+580

Pile Type= Bored
 Pile Dia (mm)= 1200

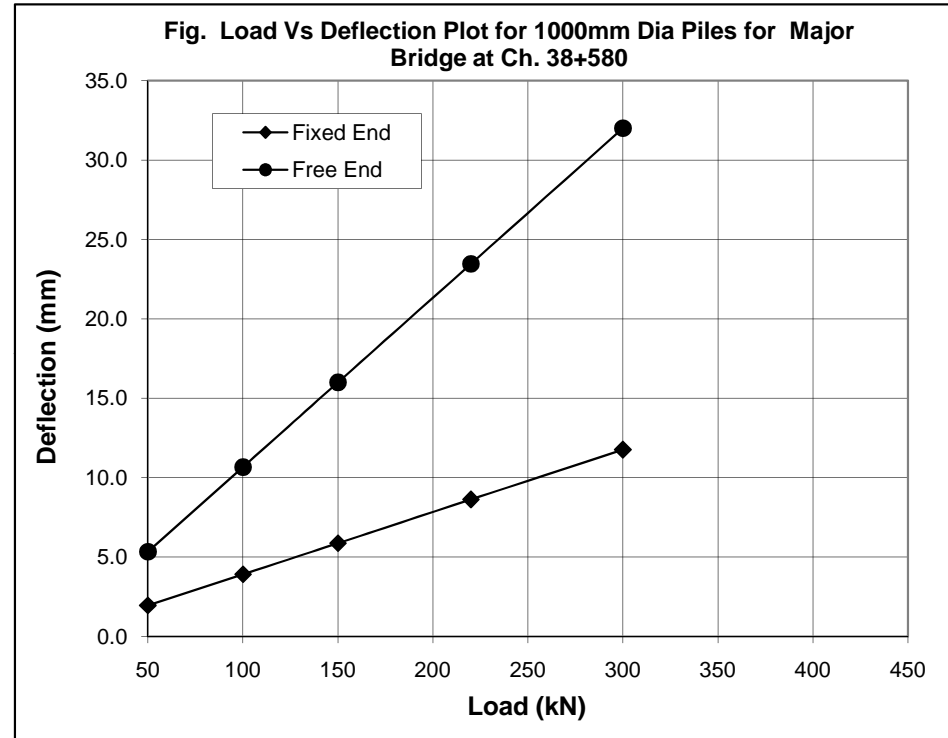
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 38+580 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 100 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.96	5.33
100	3.92	10.67
150	5.88	16.00
220	8.62	23.47
300	11.75	32.00



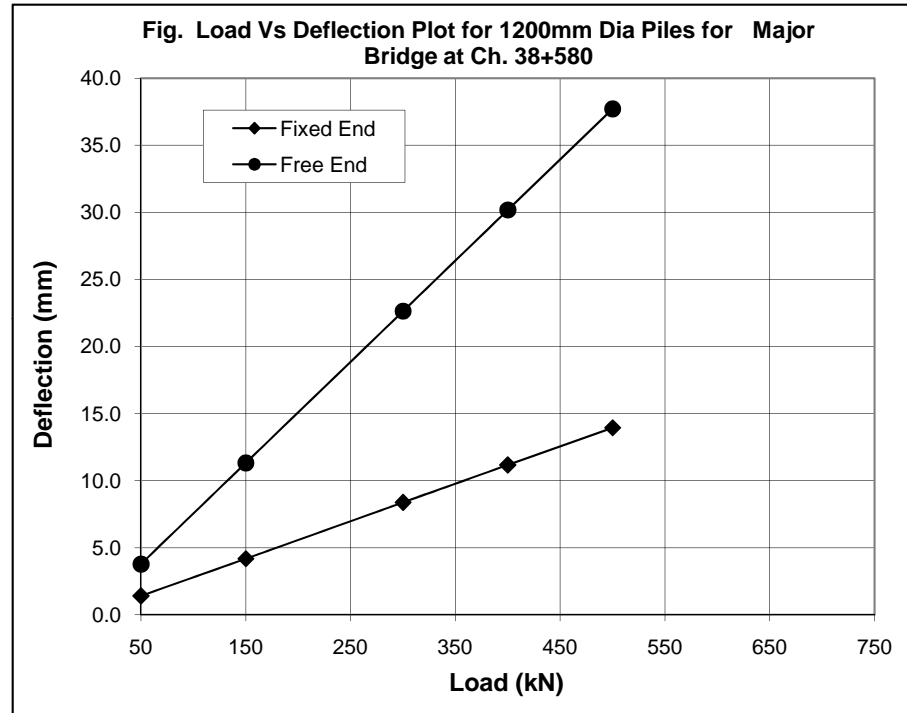
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 250kN (for fixed head condition)
 = 90 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 38+580 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 396.63$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 872.58$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 753.59$ cm
 $L_1 = 100$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

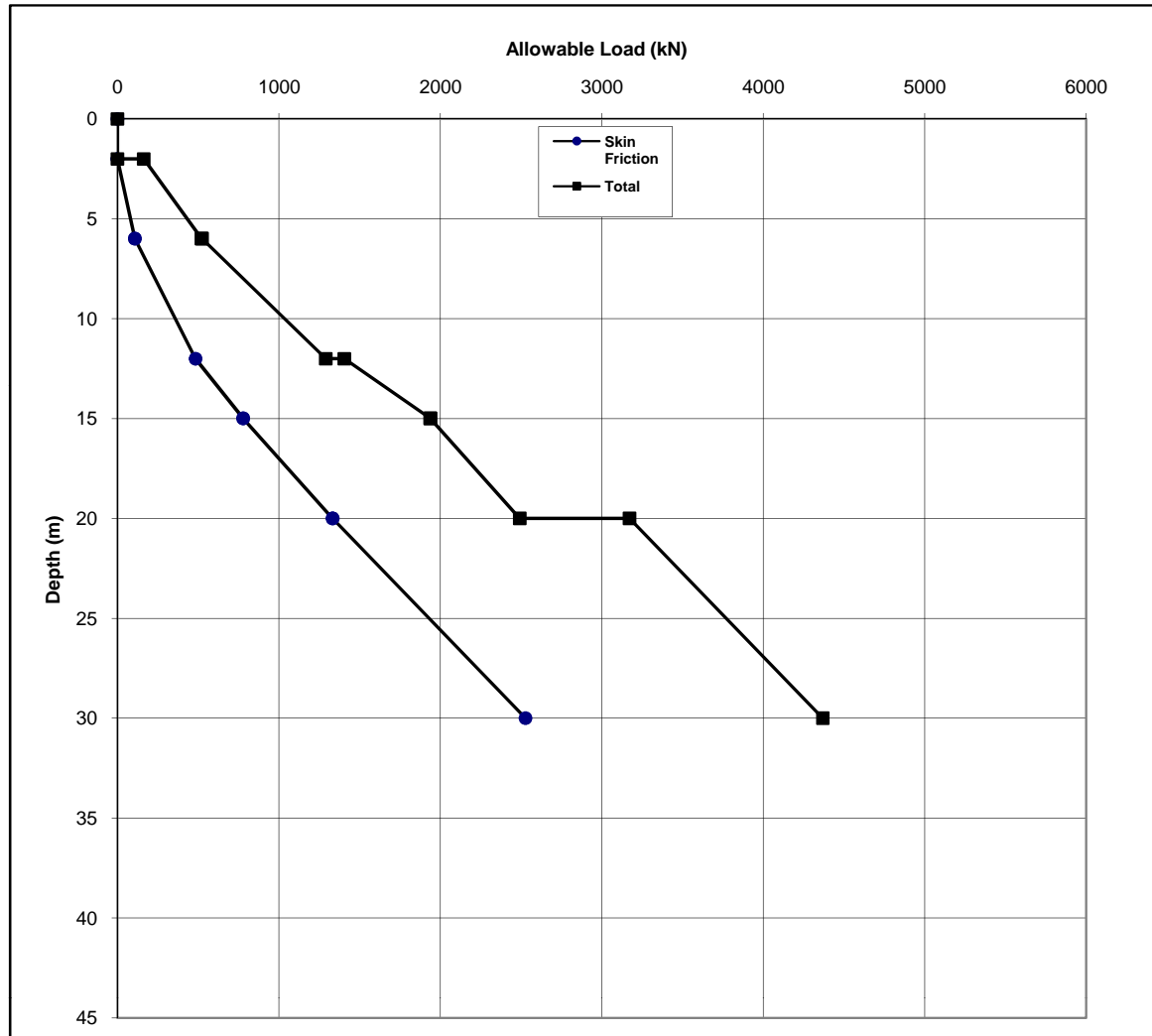
Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.39	3.77
150	4.18	11.32
300	8.37	22.63
400	11.16	30.17
500	13.95	37.72



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 420kN (for fixed head condition)

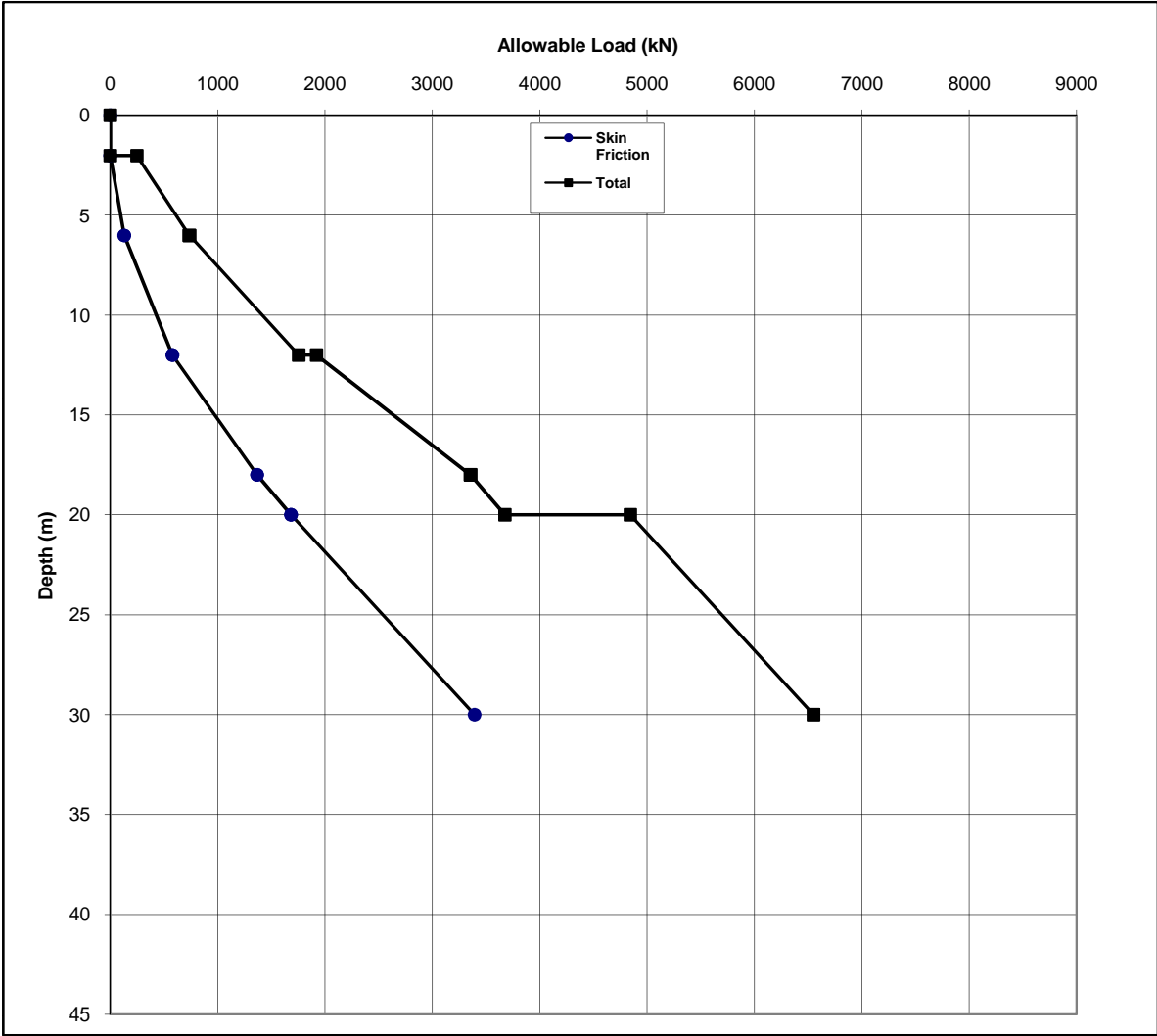
= 160kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 39+120

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 39+120

Pile Type= Bored
 Pile Dia (mm)= 1200

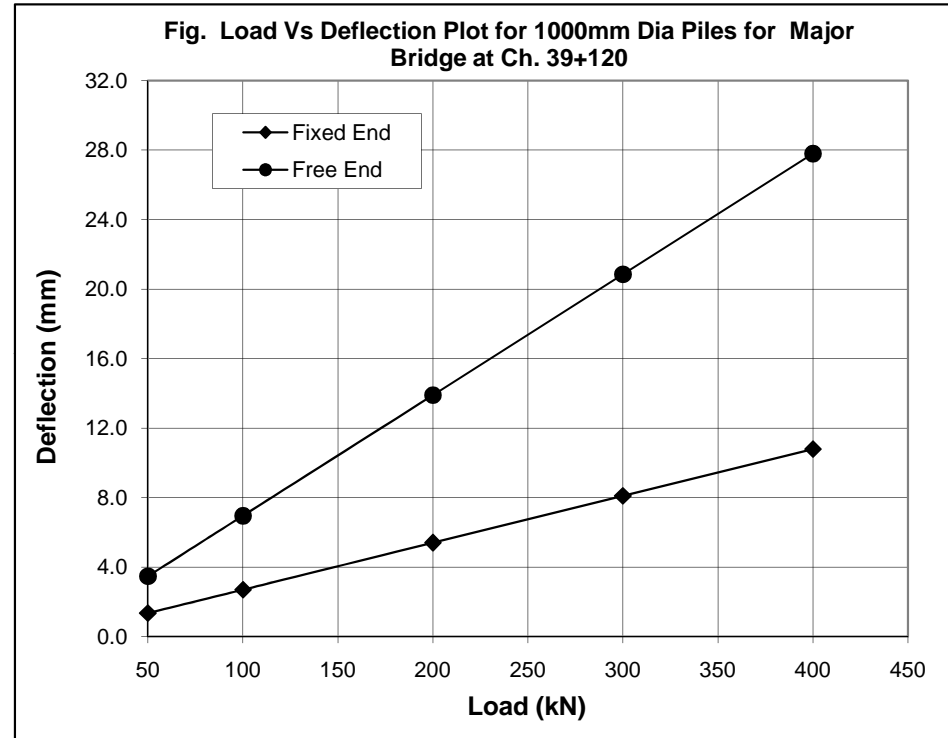
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 39+120 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 342.80$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 754.15 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 651.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = Q(L_1+L_f)^3/12EI$ Fixed
 $d = Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.35	3.47
100	2.70	6.95
200	5.39	13.90
300	8.09	20.85
400	10.79	27.80



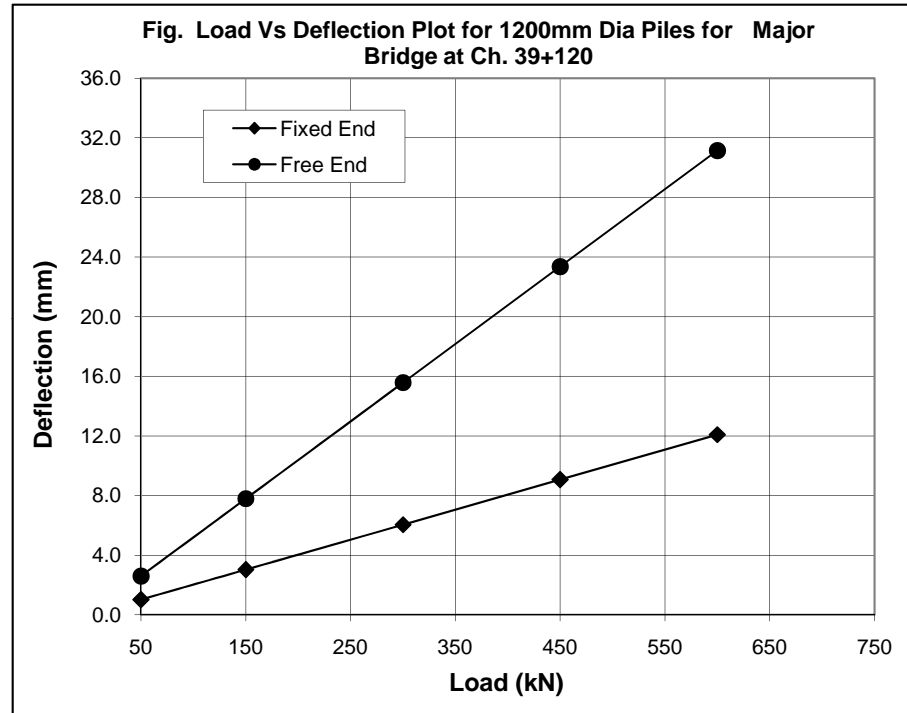
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 370kN (for fixed head condition)
 = 140 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 39+120 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 $\eta_b = 0.280 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 396.63$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 872.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 753.59 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

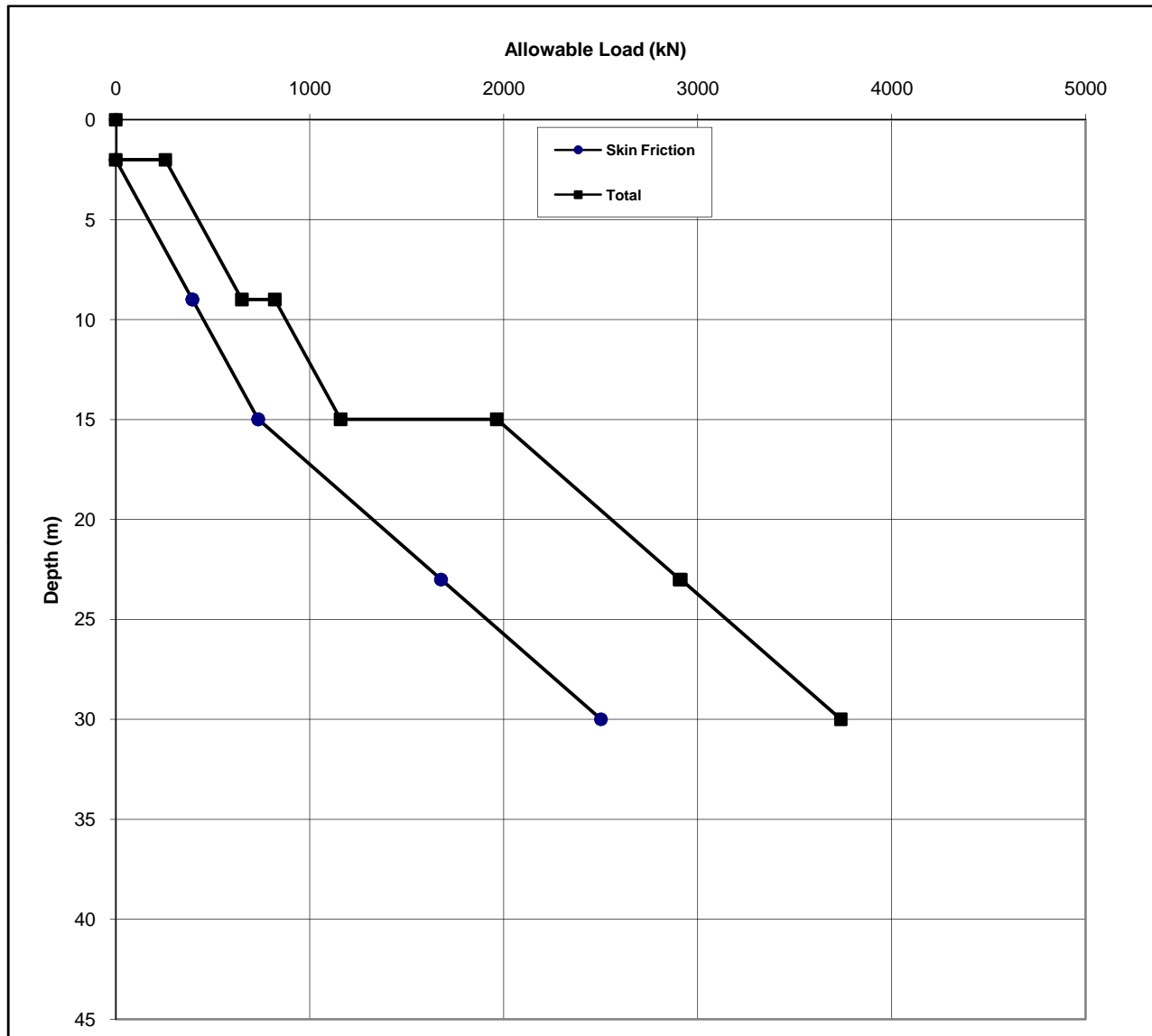
Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.01	2.60
150	3.02	7.79
300	6.04	15.57
450	9.07	23.36
600	12.09	31.14



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 600kN (for fixed head condition)

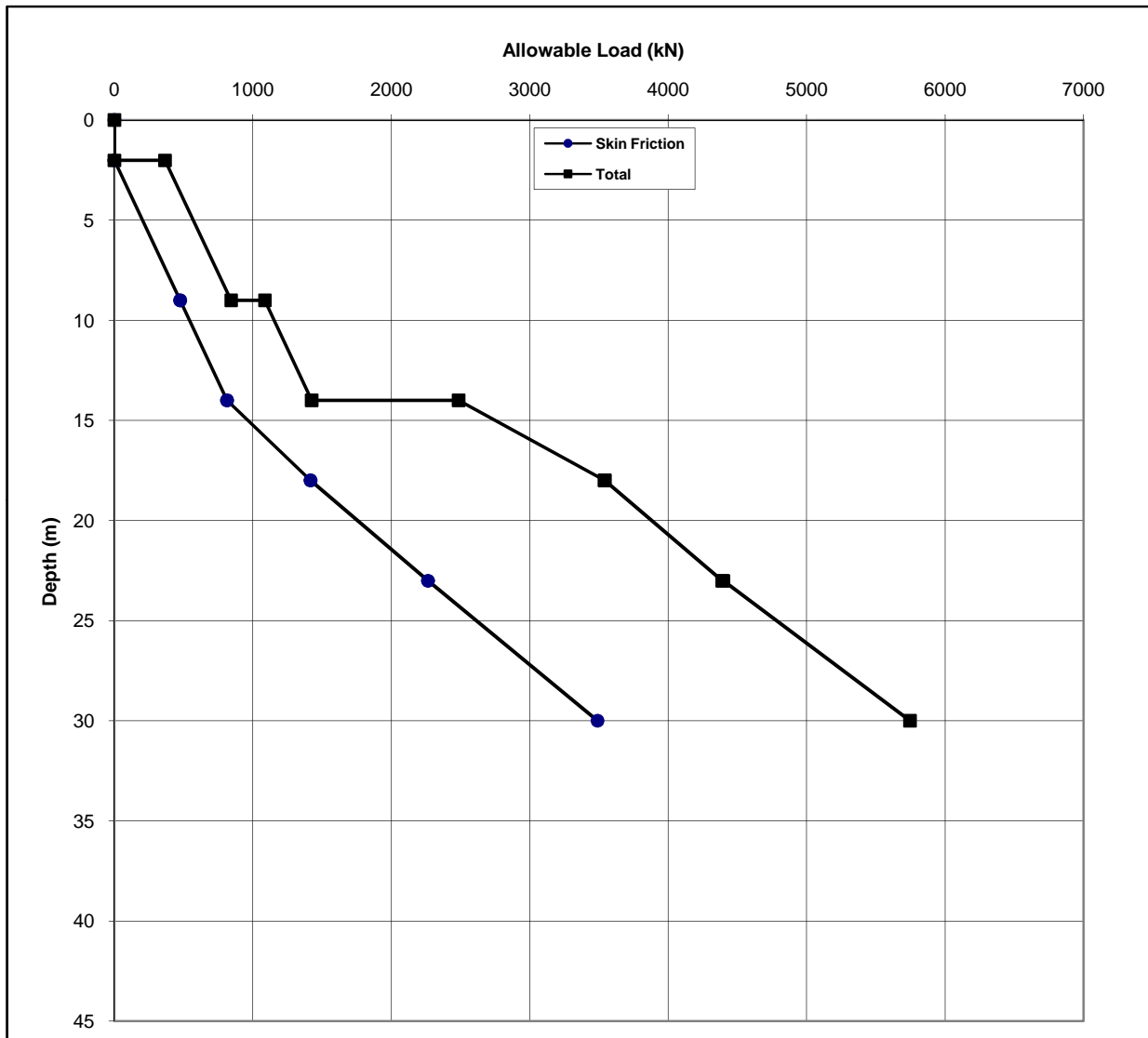
= 220kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 41+916

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 41+916

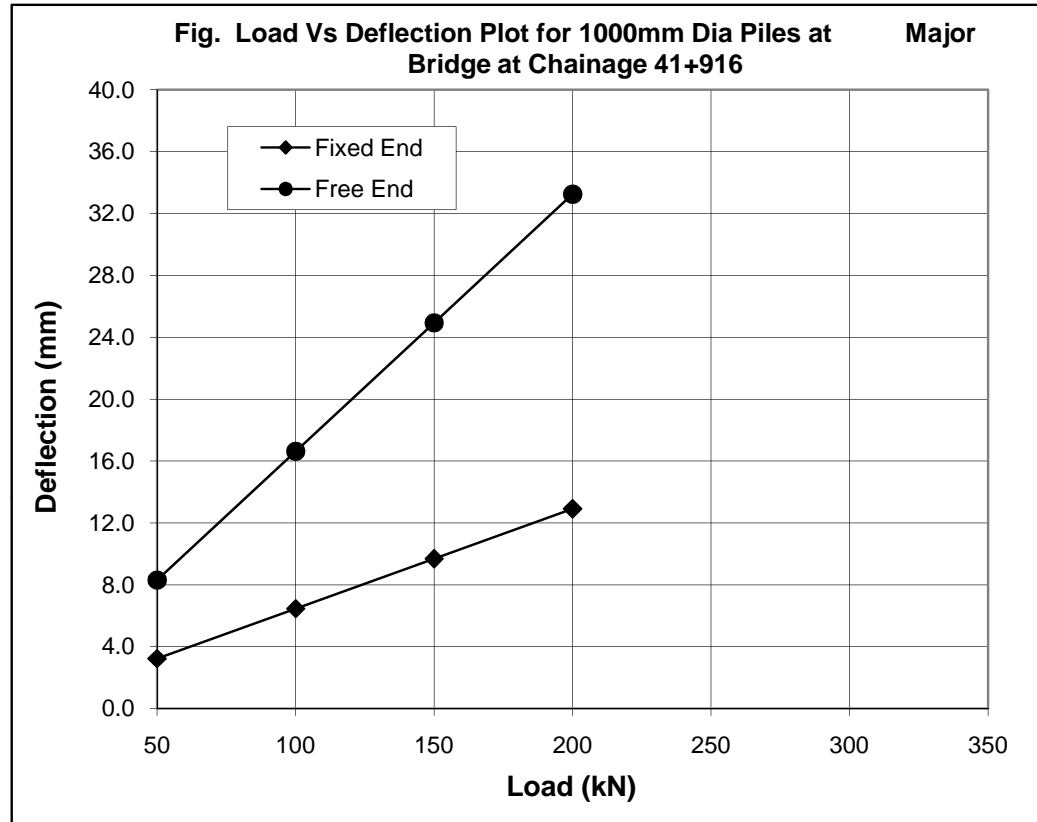
Pile Type= Bored
Pile Dia (mm)= 1200

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5

D= 100 cm
 K₁ 1.500 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²
 K=(K₁*0.3)/(1.5B) 0.3 kg/cm³

T= (EI/KB)^{0.25}
 458.46
 L_f/T= 2.2 Fixed
 L_f (Fixed)= 1008.62 cm
 L_f/T= 1.9 Free
 L_f (Free)= 871.08 cm
 L₁= 0 cm
 d= $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.23	8.31
100	6.45	16.62
150	9.68	24.93
200	12.90	33.25

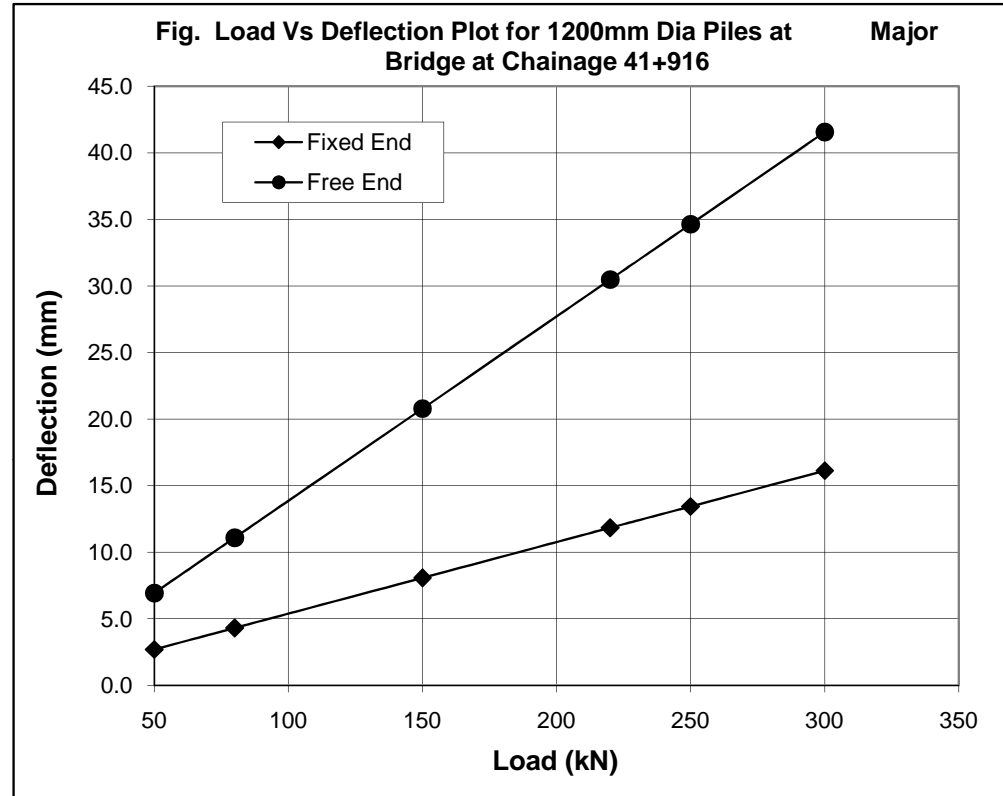


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 150kN (for fixed head condition)
 = 60 kN (for free head condition)

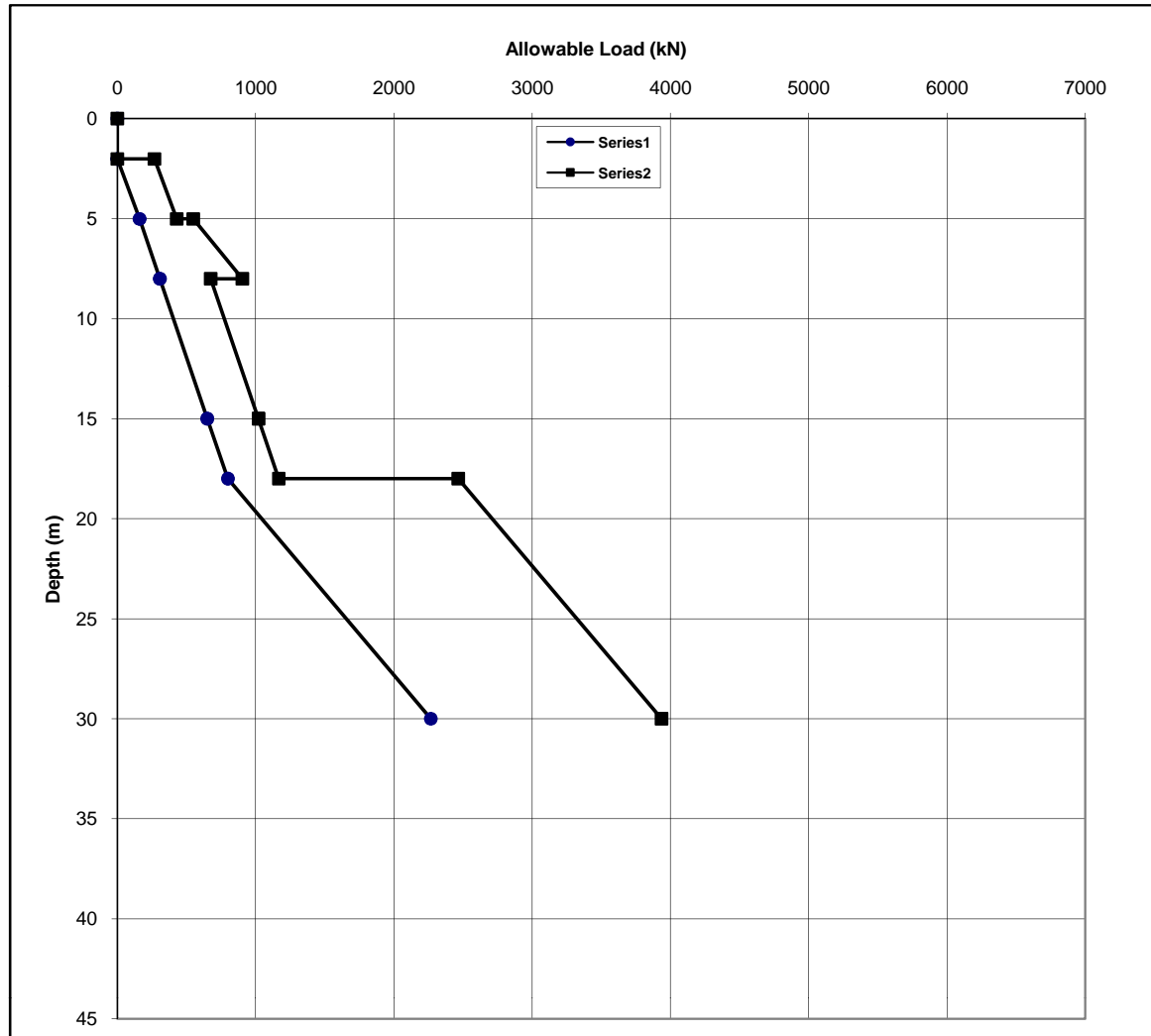
D= 120 cm
 $K_1 = 1.500 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.25 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 550.15$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 1210.34 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 1045.29 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	2.69	6.93
80	4.30	11.08
150	8.06	20.78
220	11.83	30.48
250	13.44	34.63
300	16.13	41.56



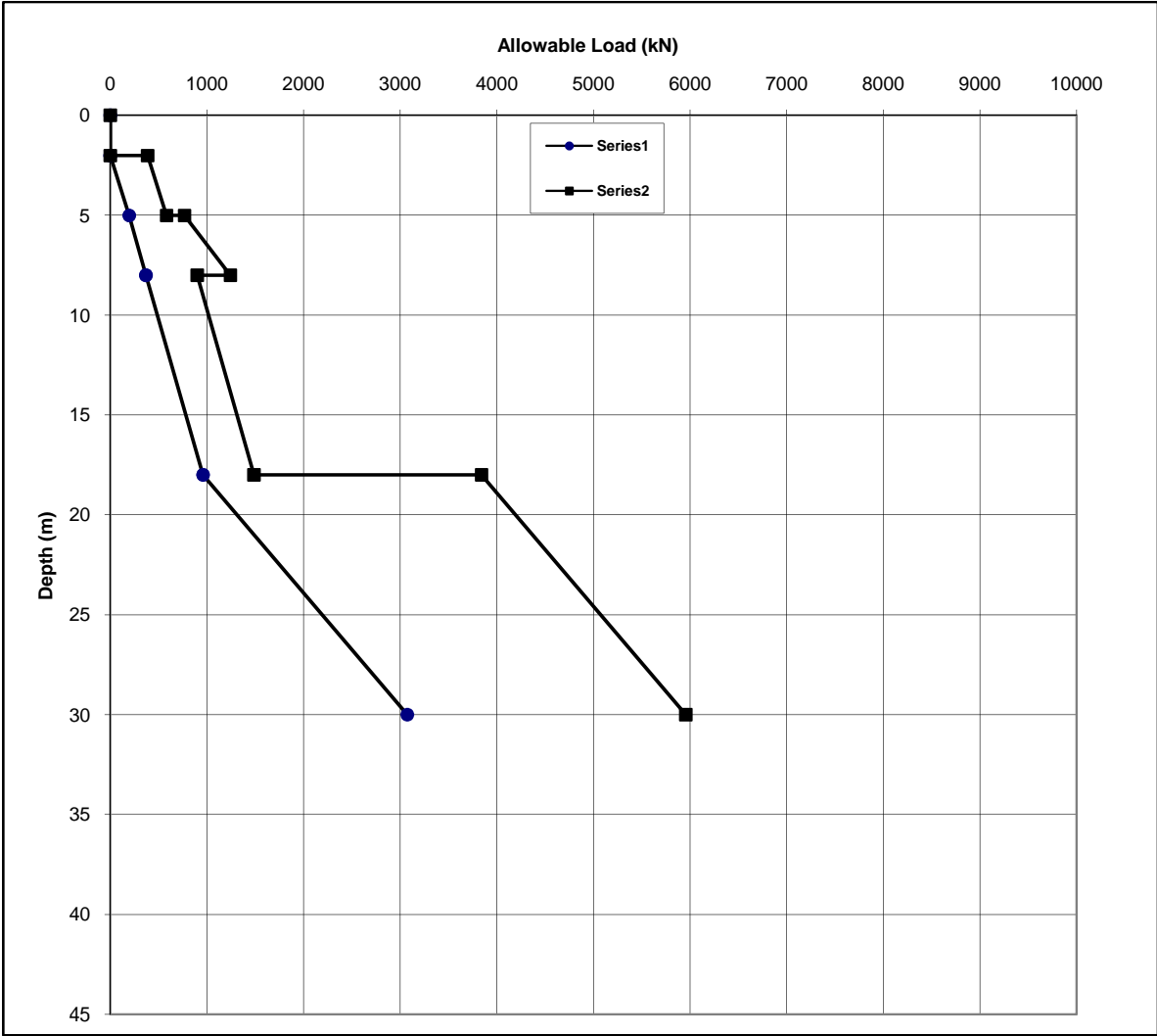
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 220kN (for fixed head condition)
 = 90kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 43+900

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 43+900

Pile Type= Bored
 Pile Dia (mm)= 1200

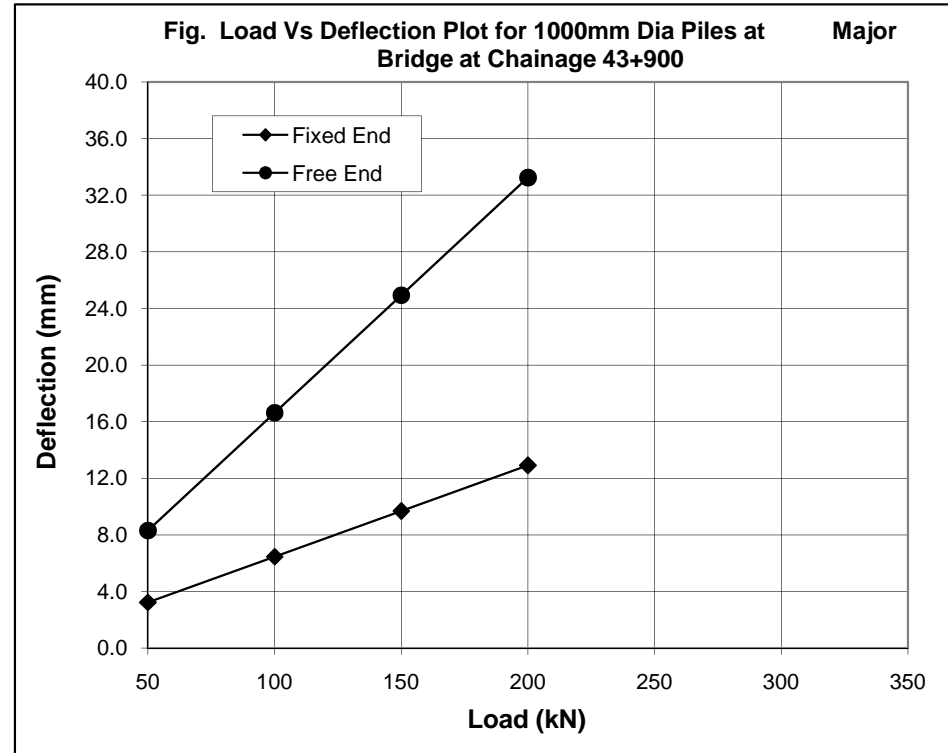
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 43+900 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 K_1 1.500 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²
 $K=(K_1*0.3)/(1.5B)$ 0.3 kg/cm³

T= (EI/KB)^{0.25}
 458.46
 $L_f/T=$ 2.2 Fixed
 L_f (Fixed)= 1008.62 cm
 $L_f/T=$ 1.9 Free
 L_f (Free)= 871.08 cm
 $L_1=$ 0 cm
 $d=$ $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.23	8.31
100	6.45	16.62
150	9.68	24.93
200	12.90	33.25



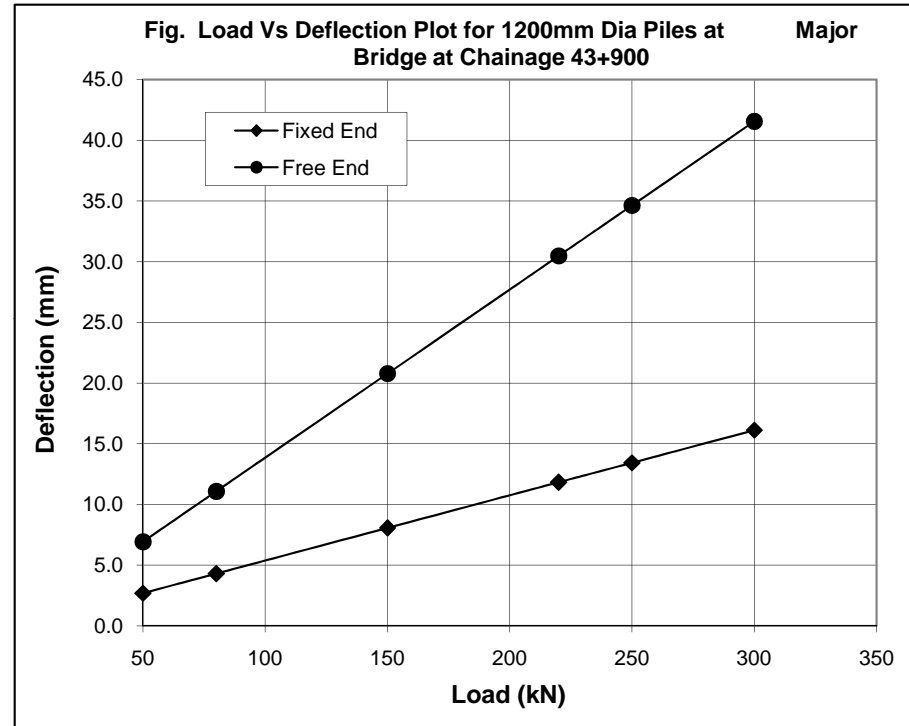
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 150kN (for fixed head condition)
 = 60 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 43+900 (IS: 2911 - PART-1/SEC-2-2010)

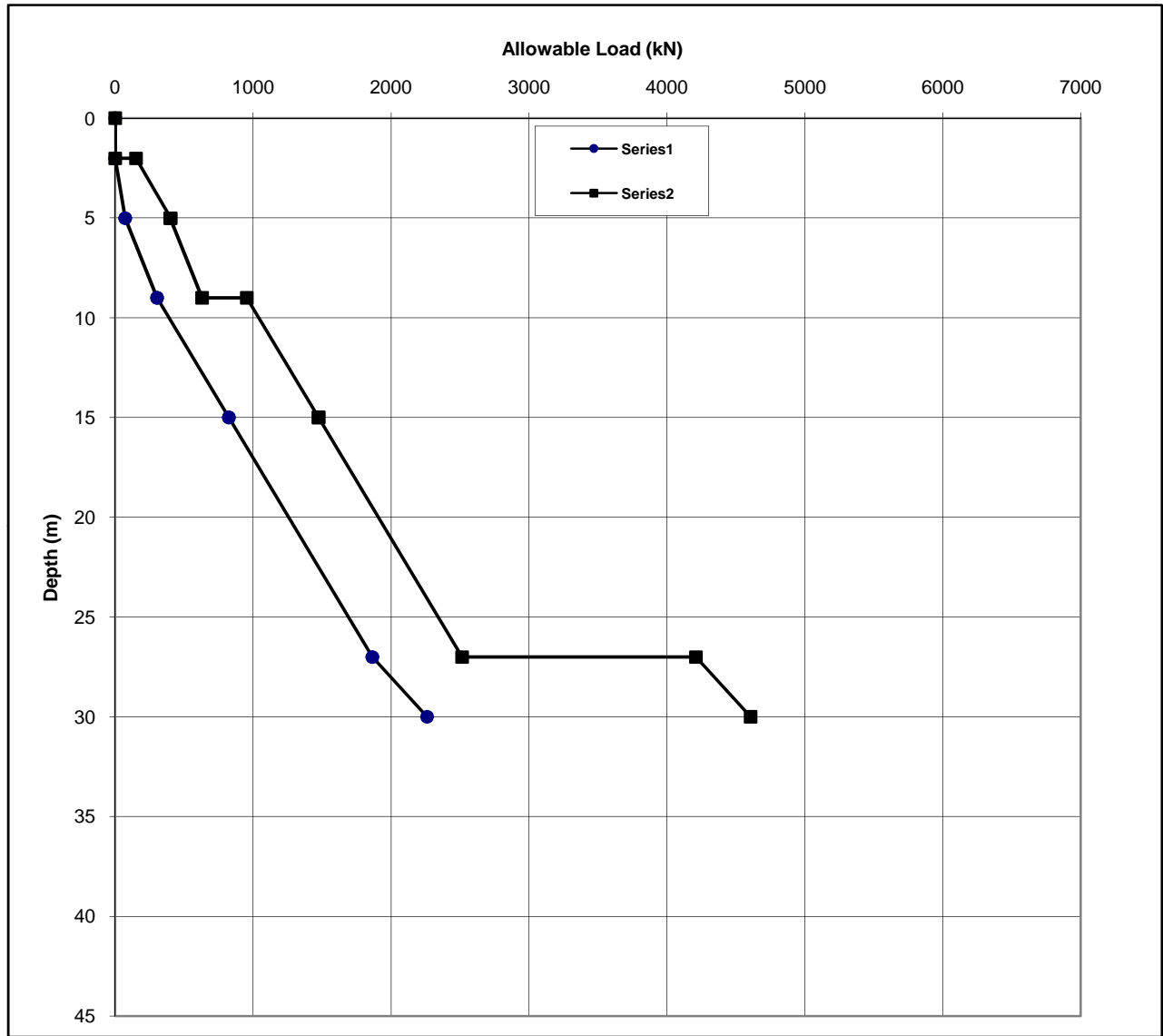
D= 120 cm
 $K_1 = 1.500 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.25 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 550.15$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 1210.34$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 1045.29$ cm
 $L_1 = 0$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	2.69	6.93
80	4.30	11.08
150	8.06	20.78
220	11.83	30.48
250	13.44	34.63
300	16.13	41.56



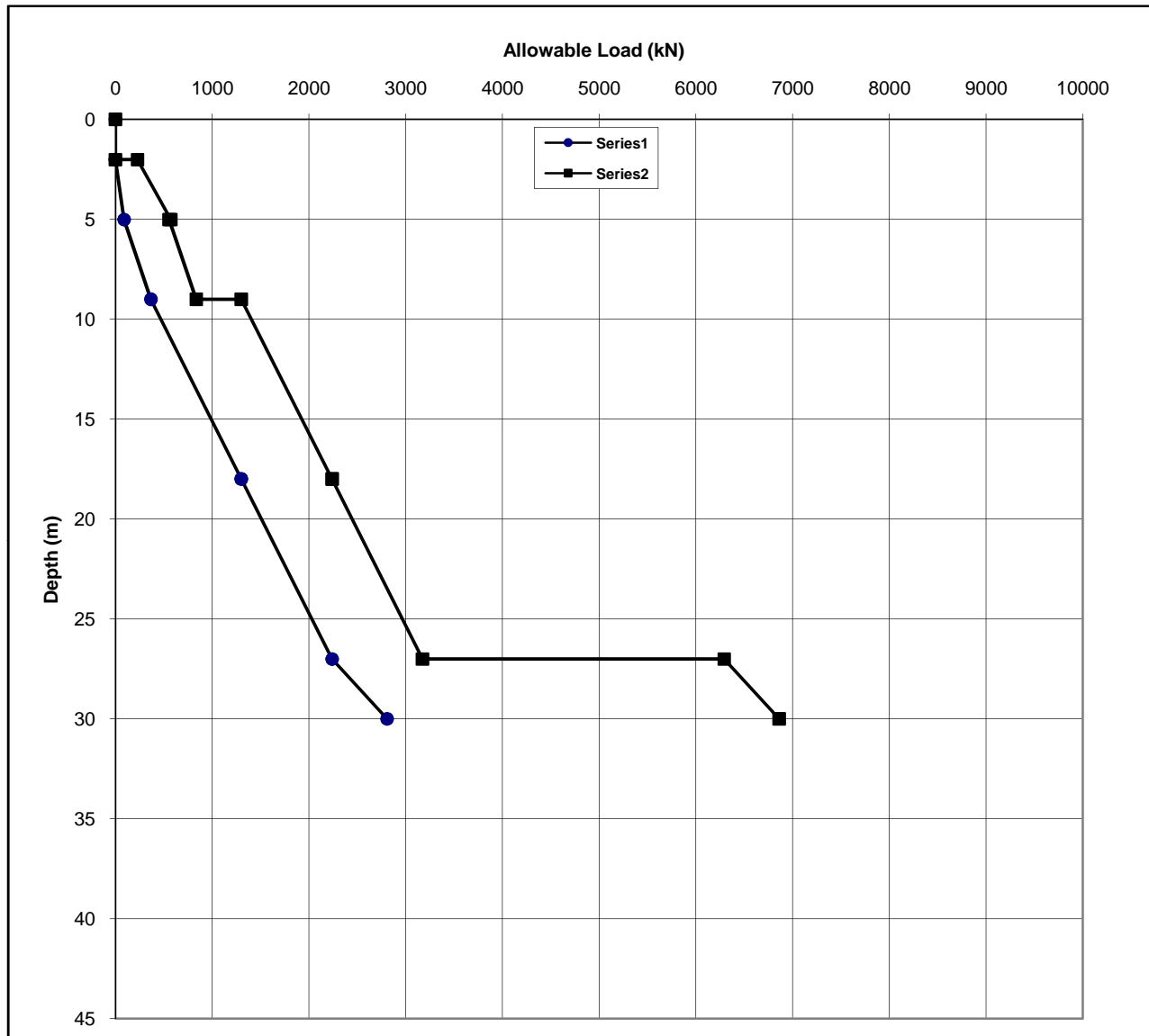
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 220kN (for fixed head condition)
 = 90kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 46+362

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 46+362

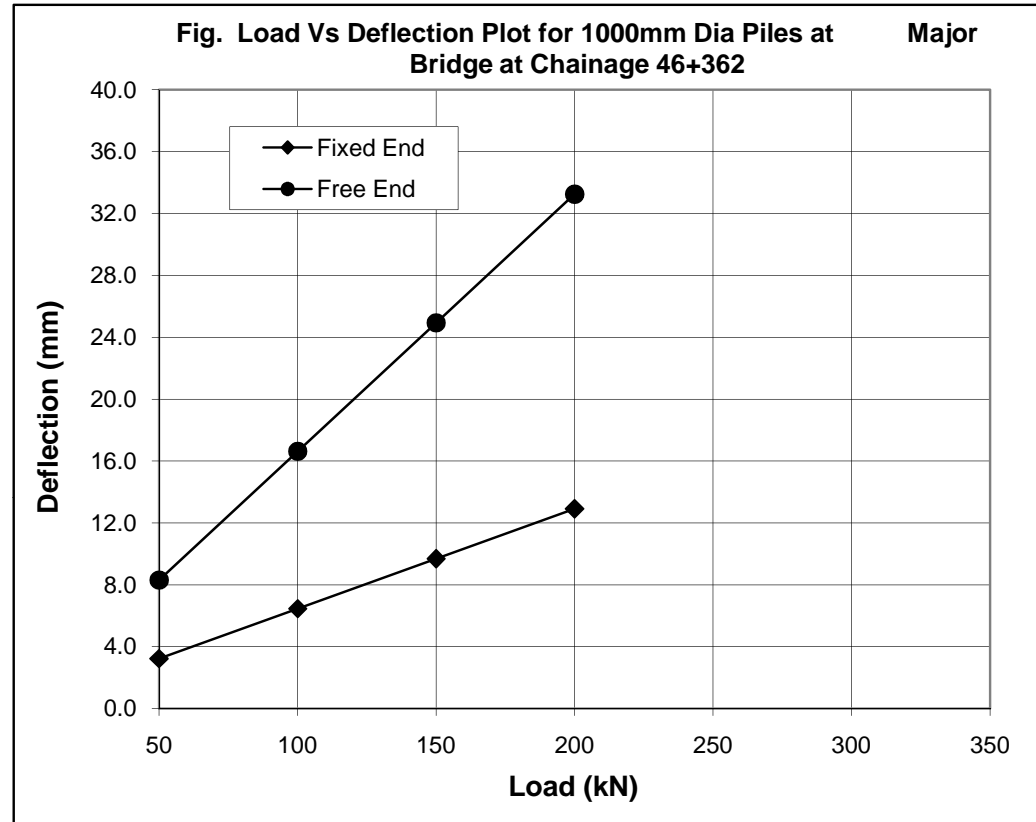
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 K₁ 1.500 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²
 K=(K₁*0.3)/(1.5B) 0.3 kg/cm³

T= (EI/KB)^{0.25}
 458.46
 L_f/T= 2.2 Fixed
 L_f (Fixed)= 1008.62 cm
 L_f/T= 1.9 Free
 L_f (Free)= 871.08 cm
 L₁= 0 cm
 d= Q(L₁+L_f)³/12EI Fixed
 Q(L₁+L_f)³/3EI Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.23	8.31
100	6.45	16.62
150	9.68	24.93
200	12.90	33.25

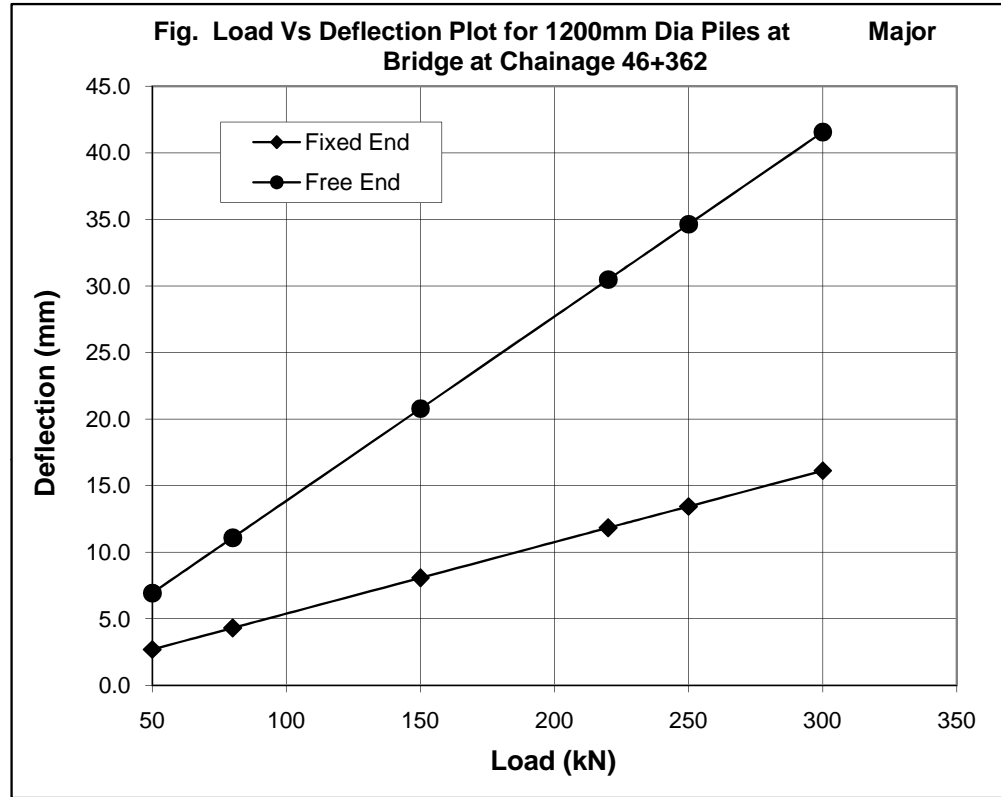


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 150kN (for fixed head condition)
 = 60 kN (for free head condition)

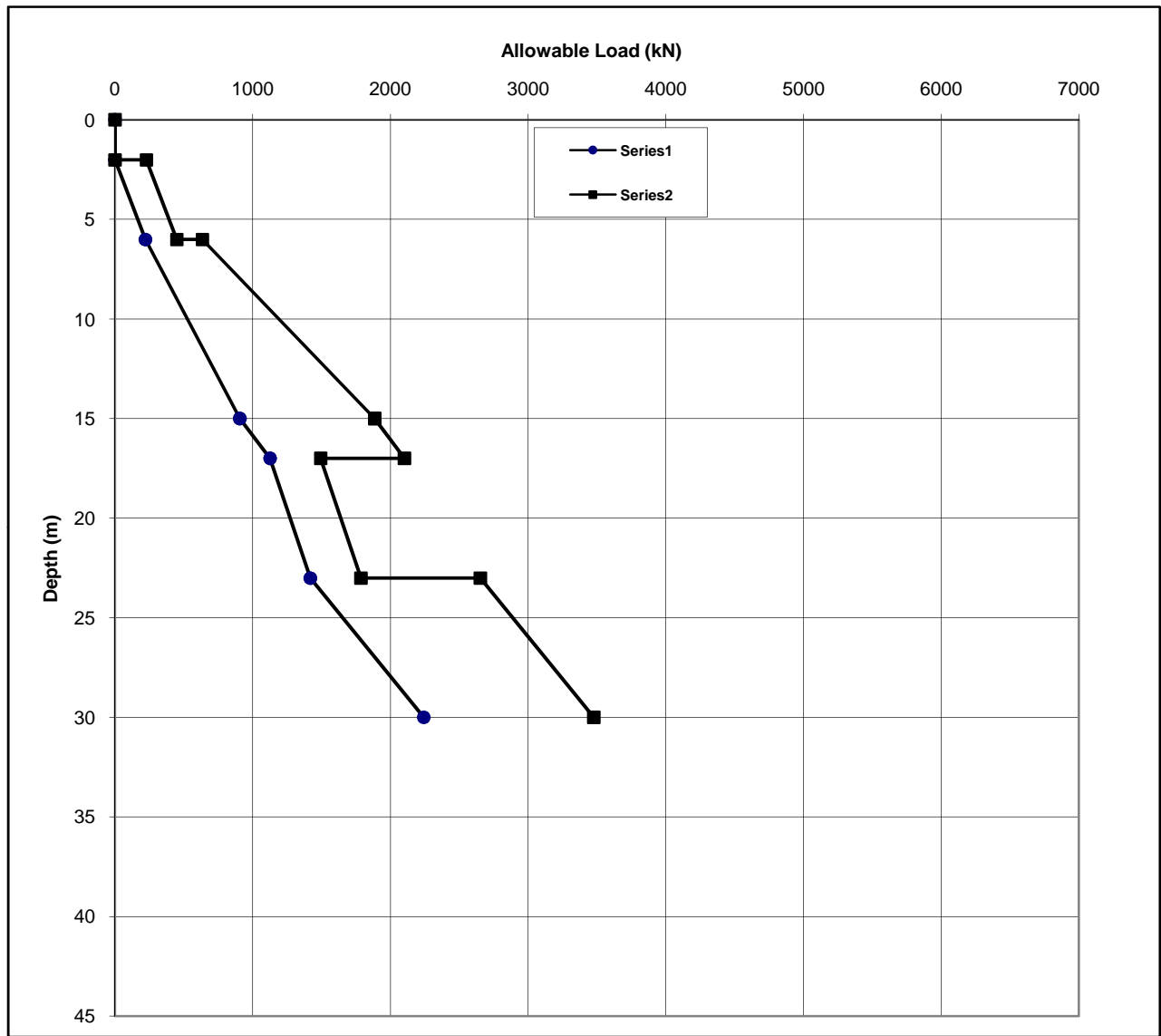
D= 120 cm
 $K_1 = 1.500 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.25 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 550.15$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 1210.34 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 1045.29 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	2.69	6.93
80	4.30	11.08
150	8.06	20.78
220	11.83	30.48
250	13.44	34.63
300	16.13	41.56



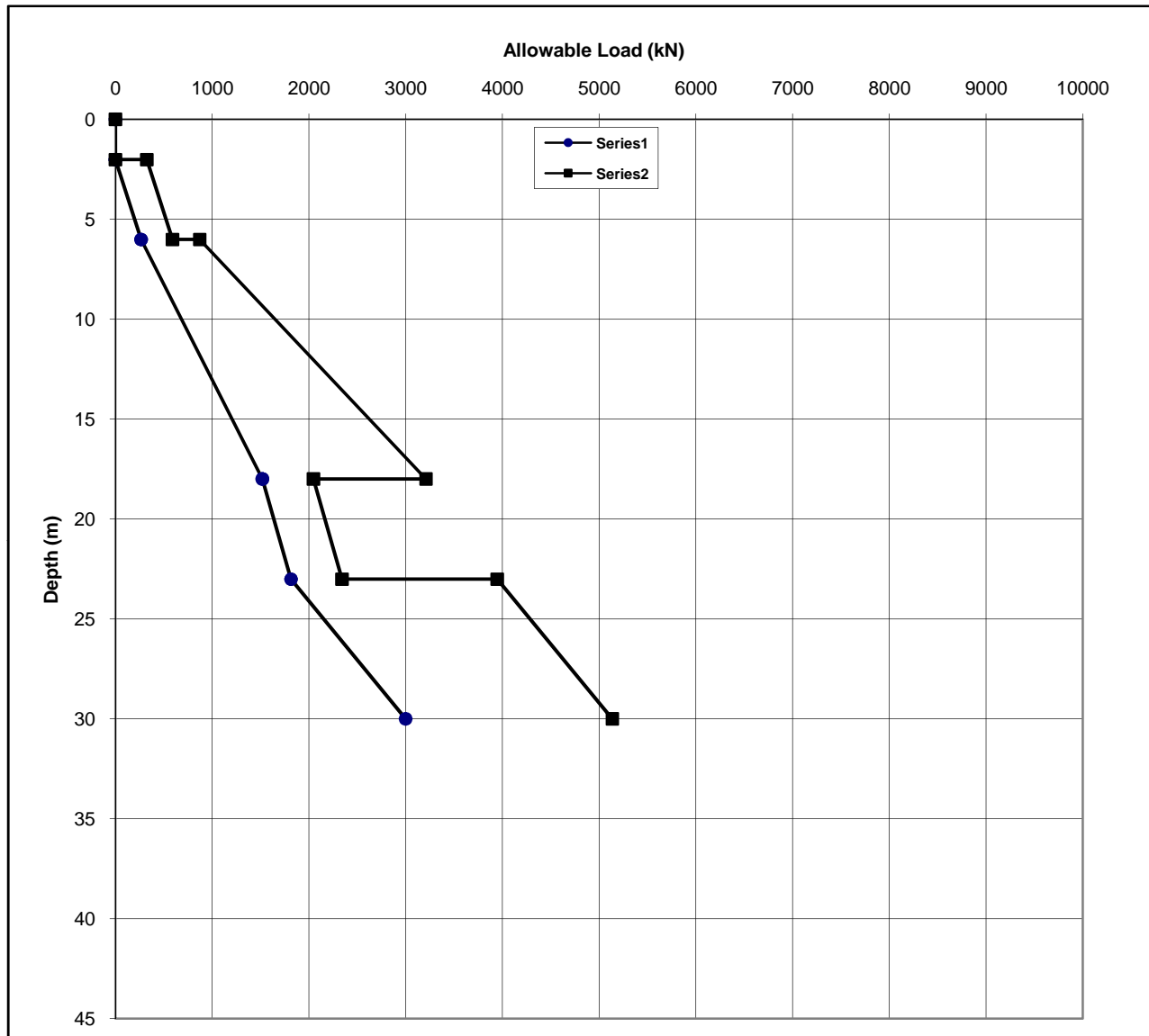
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 220kN (for fixed head condition)
 = 90kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 48+122

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 48+122

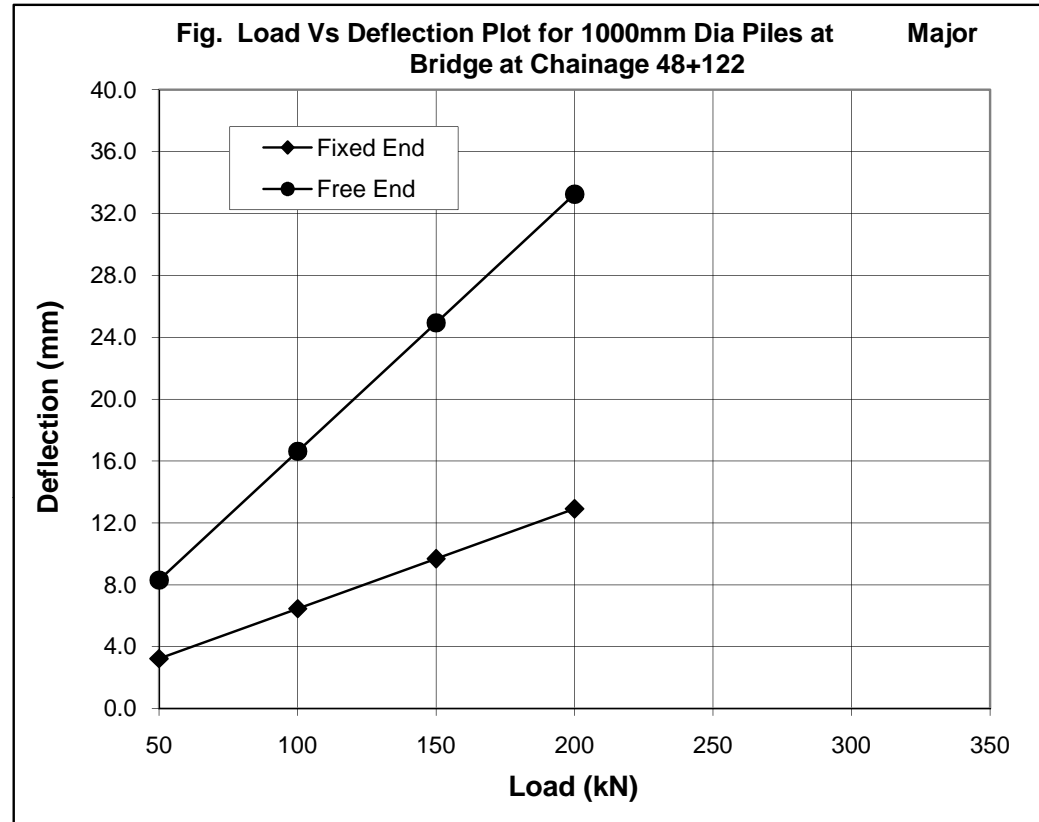
Pile Type= Bored
Pile Dia (mm)= 1200

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5

D= 100 cm
 K₁ 1.500 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²
 K=(K₁*0.3)/(1.5B) 0.3 kg/cm³

T= (EI/KB)^{0.25}
 458.46
 L_f/T= 2.2 Fixed
 L_f (Fixed)= 1008.62 cm
 L_f/T= 1.9 Free
 L_f (Free)= 871.08 cm
 L₁= 0 cm
 d= $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.23	8.31
100	6.45	16.62
150	9.68	24.93
200	12.90	33.25

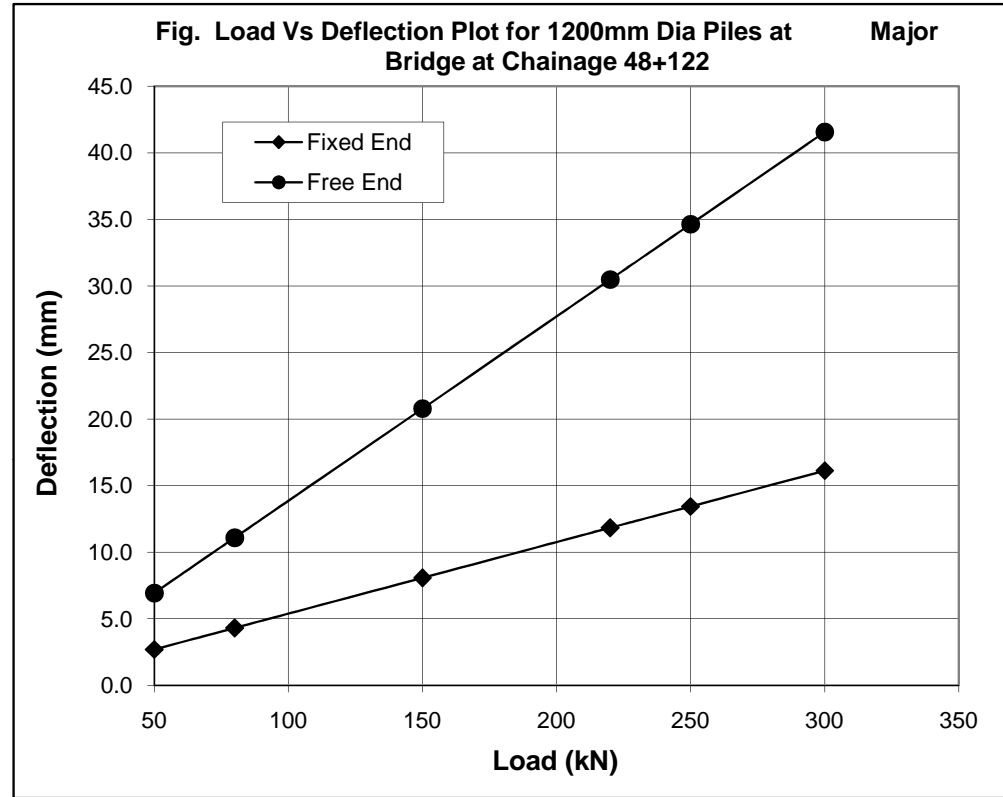


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 150kN (for fixed head condition)
 = 60 kN (for free head condition)

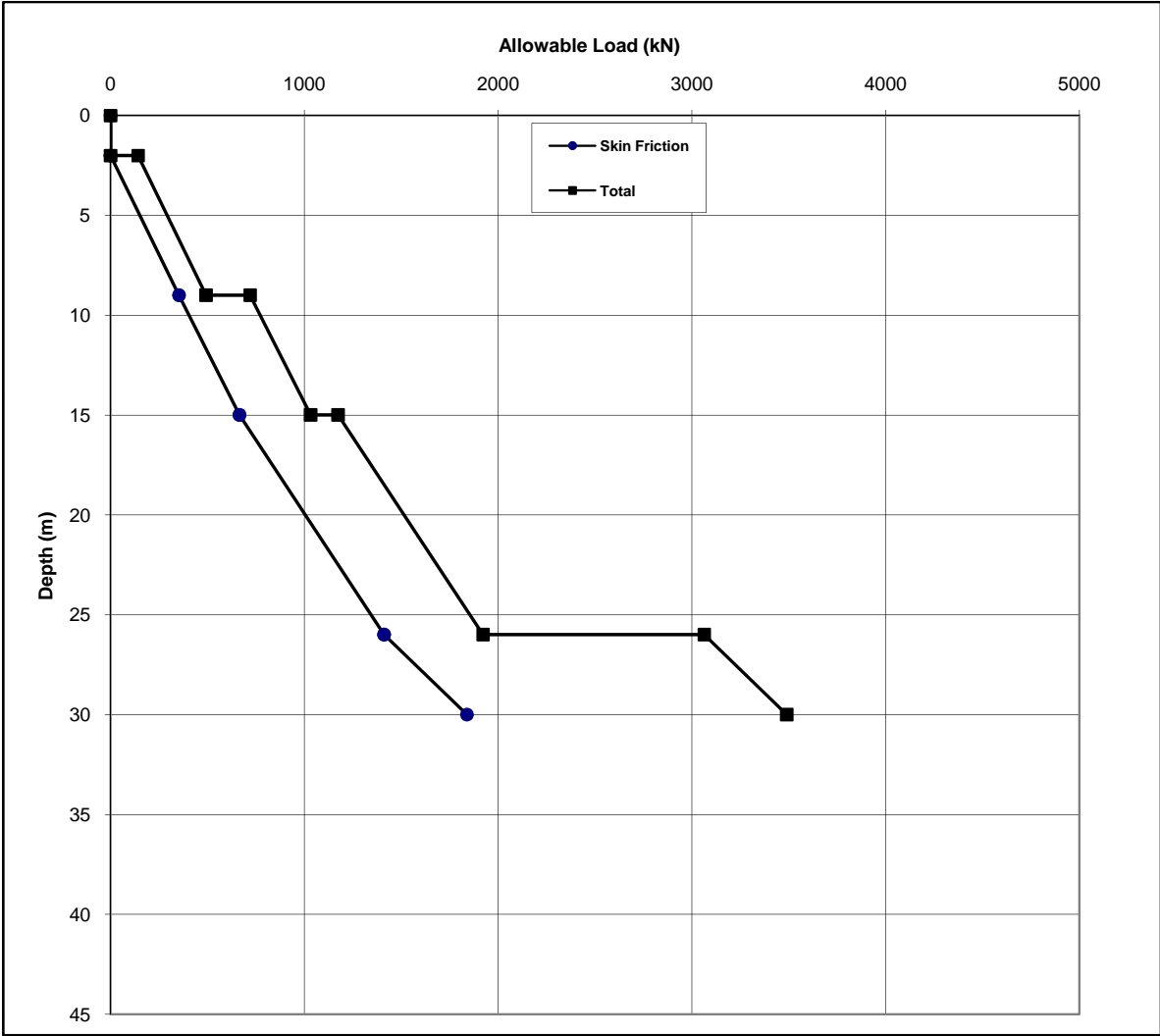
D= 120 cm
 K_1 1.500 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²
 $K=(K_1*0.3)/(1.5B)$ 0.25 kg/cm³

T= (EI/KB)^{0.25}
 550.15
 $L_f/T=$ 2.2 Fixed
 L_f (Fixed)= 1210.34 cm
 $L_f/T=$ 1.9 Free
 L_f (Free)= 1045.29 cm
 $L_1=$ 0 cm
 $d=$ $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	2.69	6.93
80	4.30	11.08
150	8.06	20.78
220	11.83	30.48
250	13.44	34.63
300	16.13	41.56



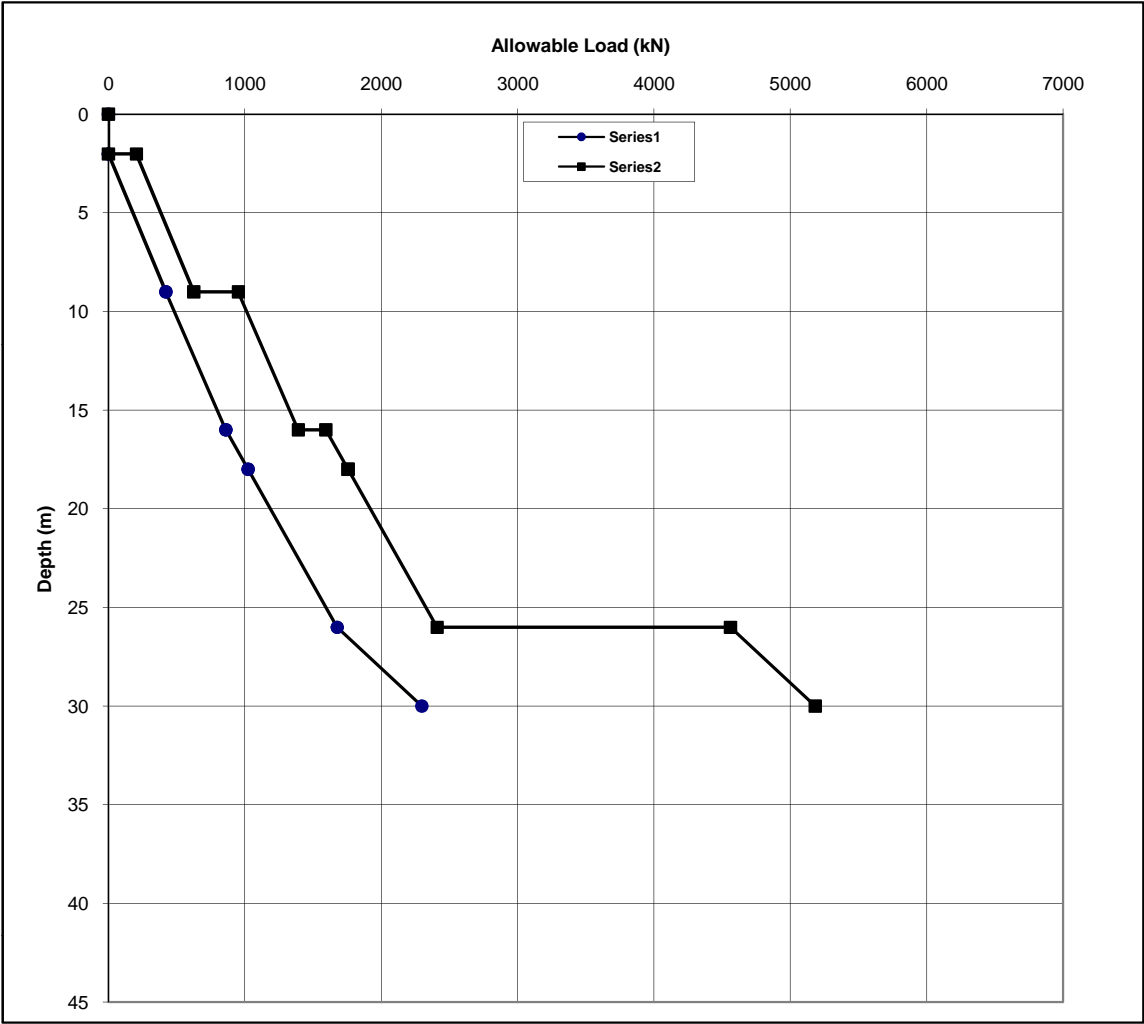
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 220kN (for fixed head condition)
 = 90kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 48+400

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 48+400

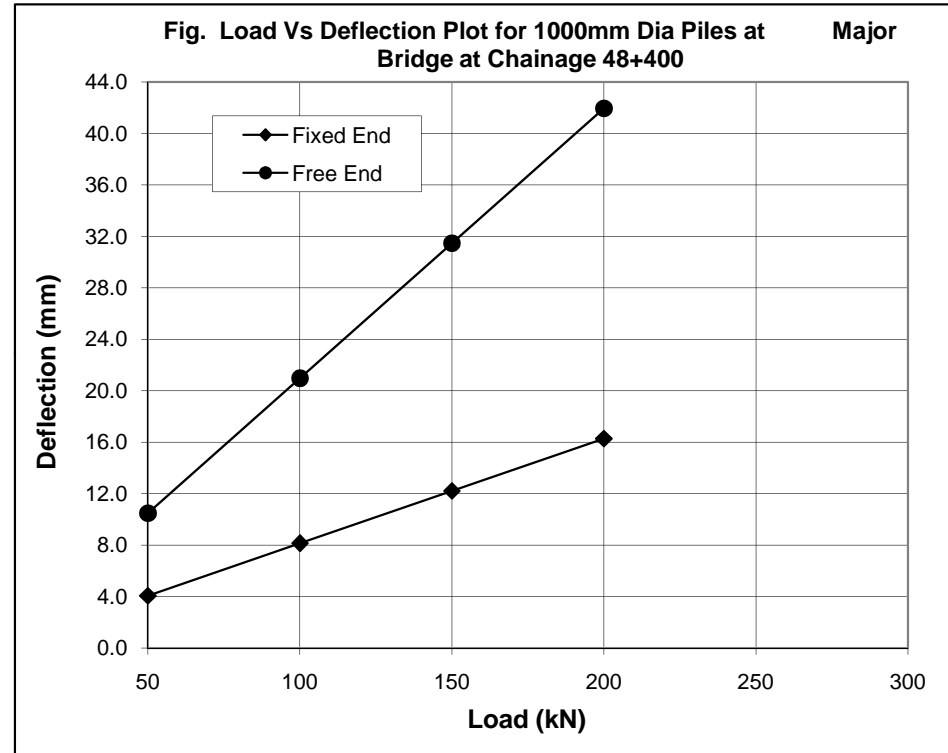
Pile Type=	Bored	Factor of Safety	
Pile Dia (mm)=	1200	End Bearing =	2.5
		Skin Friction =	2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 48+400 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $K_1 = 1.100 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.22 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 495.42$
 $L_f/T = 2.2$ Fixed
 $L_f \text{ (Fixed)} = 1089.93 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f \text{ (Free)} = 941.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = Q(L_1 + L_f)^3 / 12EI$ Fixed
 $d = Q(L_1 + L_f)^3 / 3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	4.07	10.49
100	8.14	20.98
150	12.21	31.47
200	16.28	41.95



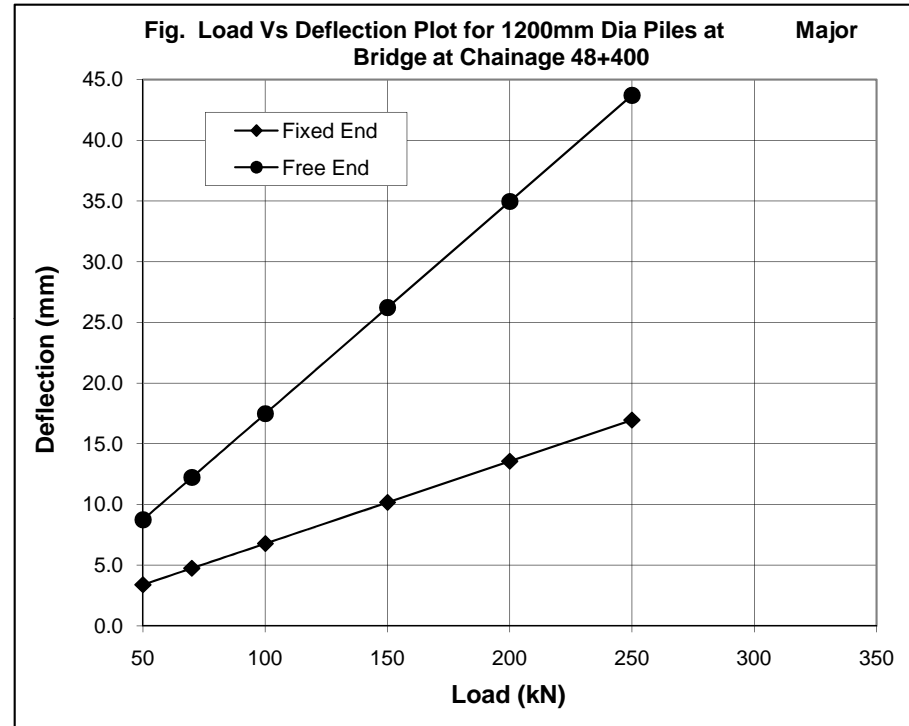
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 120kN (for fixed head condition)
 = 50 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 48+400 (IS: 2911 - PART-1/SEC-2-2010)

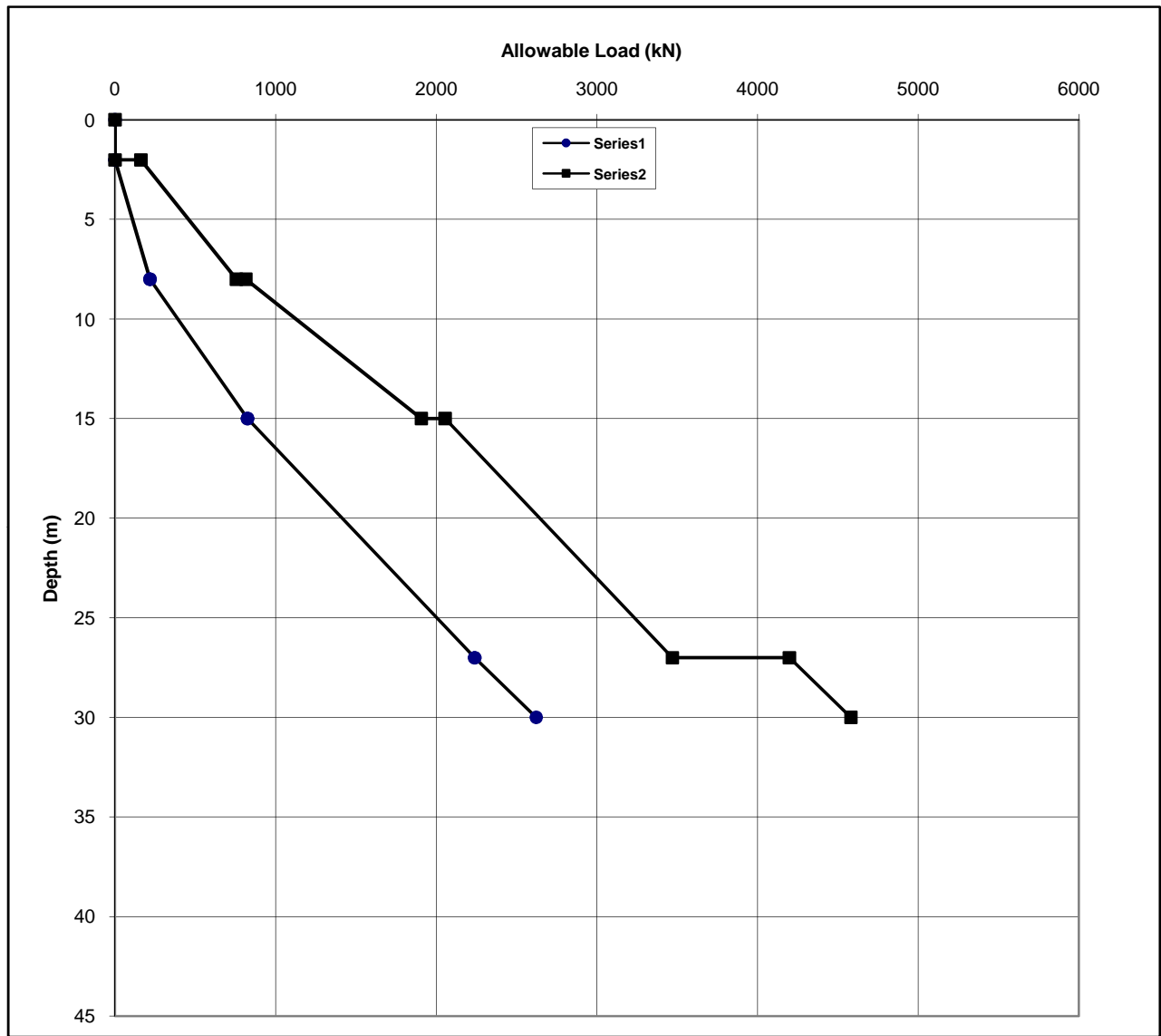
D= 120 cm
 K₁ 1.100 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²
 K=(K₁*0.3)/(1.5B) 0.183333333 kg/cm³

T= (EI/KB)^{0.25}
 594.51
 L_i/T= 2.2 Fixed
 L_i (Fixed)= 1307.92 cm
 L_i/T= 1.9 Free
 L_i (Free)= 1129.57 cm
 L₁= 0 cm
 d= $\frac{Q(L_1+L_i)^3}{12EI}$ Fixed
 $\frac{Q(L_1+L_i)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.39	8.74
70	4.75	12.24
100	6.78	17.48
150	10.18	26.22
200	13.57	34.96
250	16.96	43.70



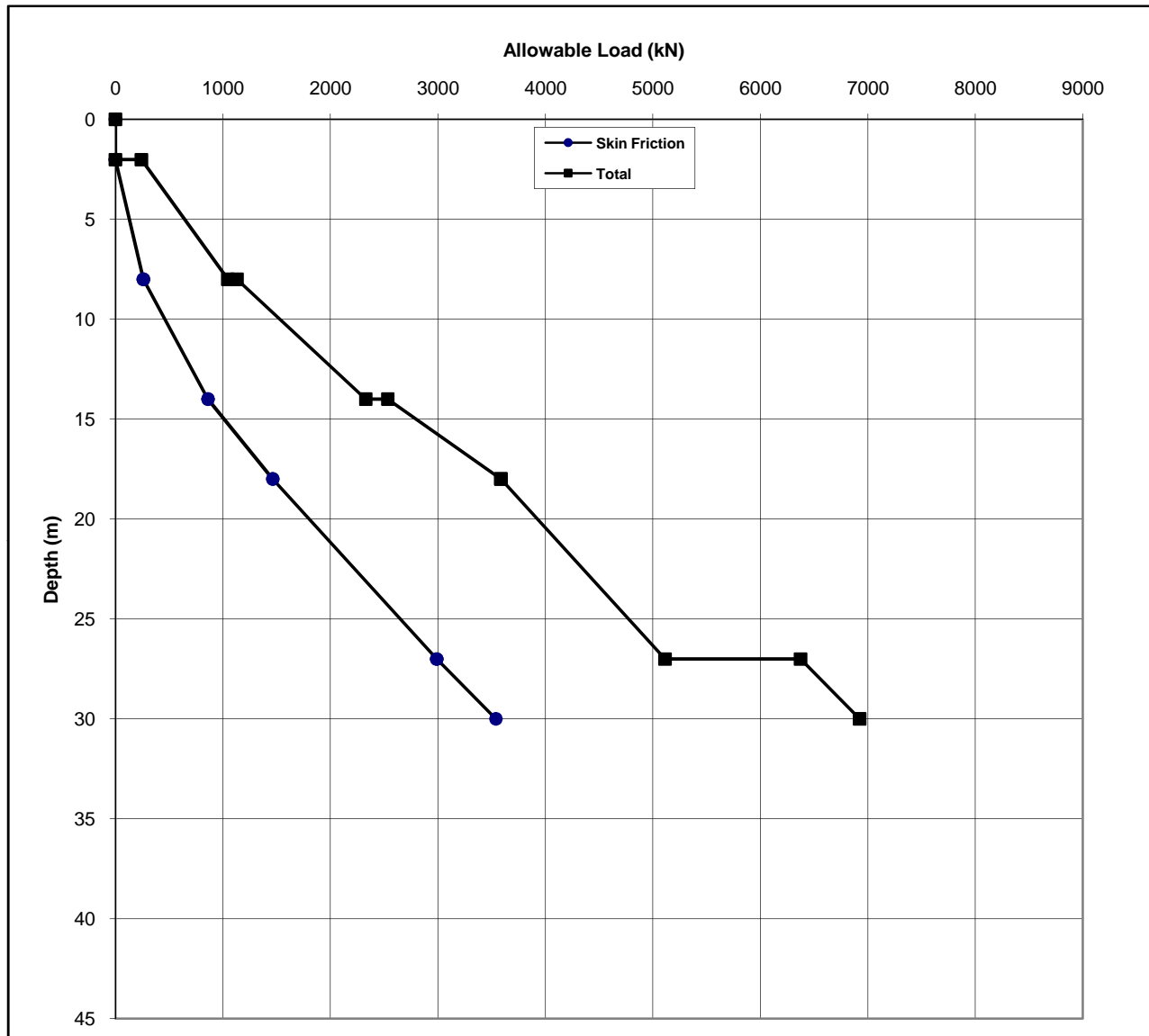
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 170kN (for fixed head condition)
 = 70kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 48+510

Pile Type= Bored
Pile Dia (mm)= 1000

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 48+510

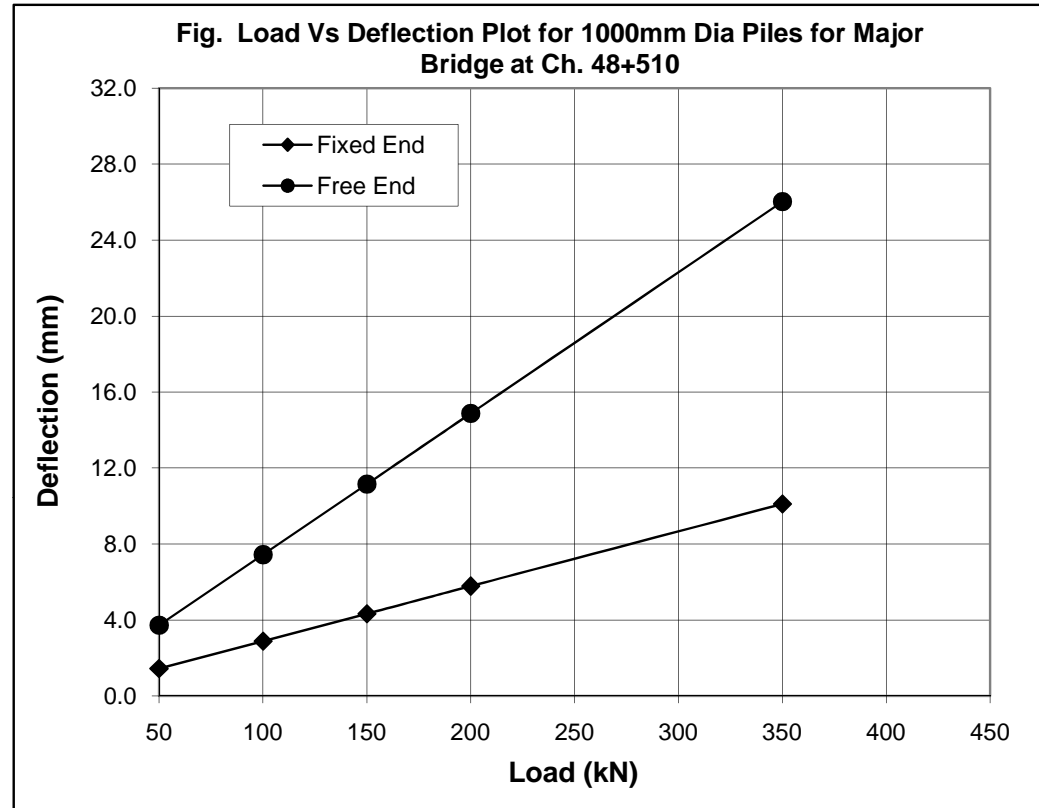
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 350.65$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 771.44 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 666.24 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03

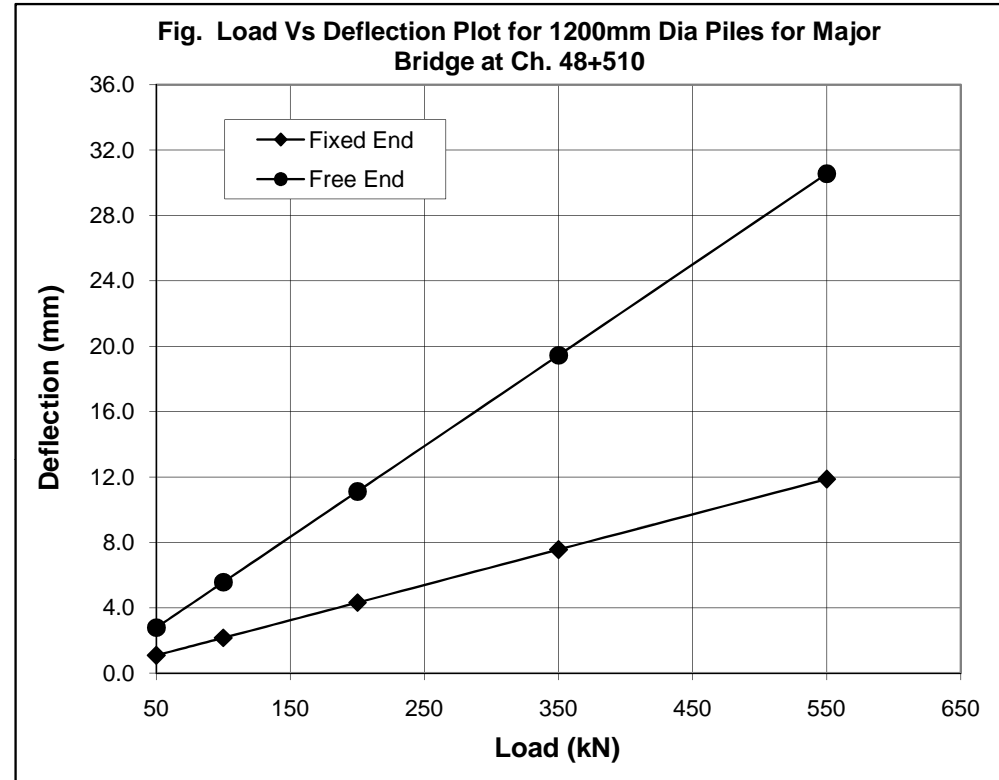


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827\text{E}+12 \text{ kg-cm}^2$

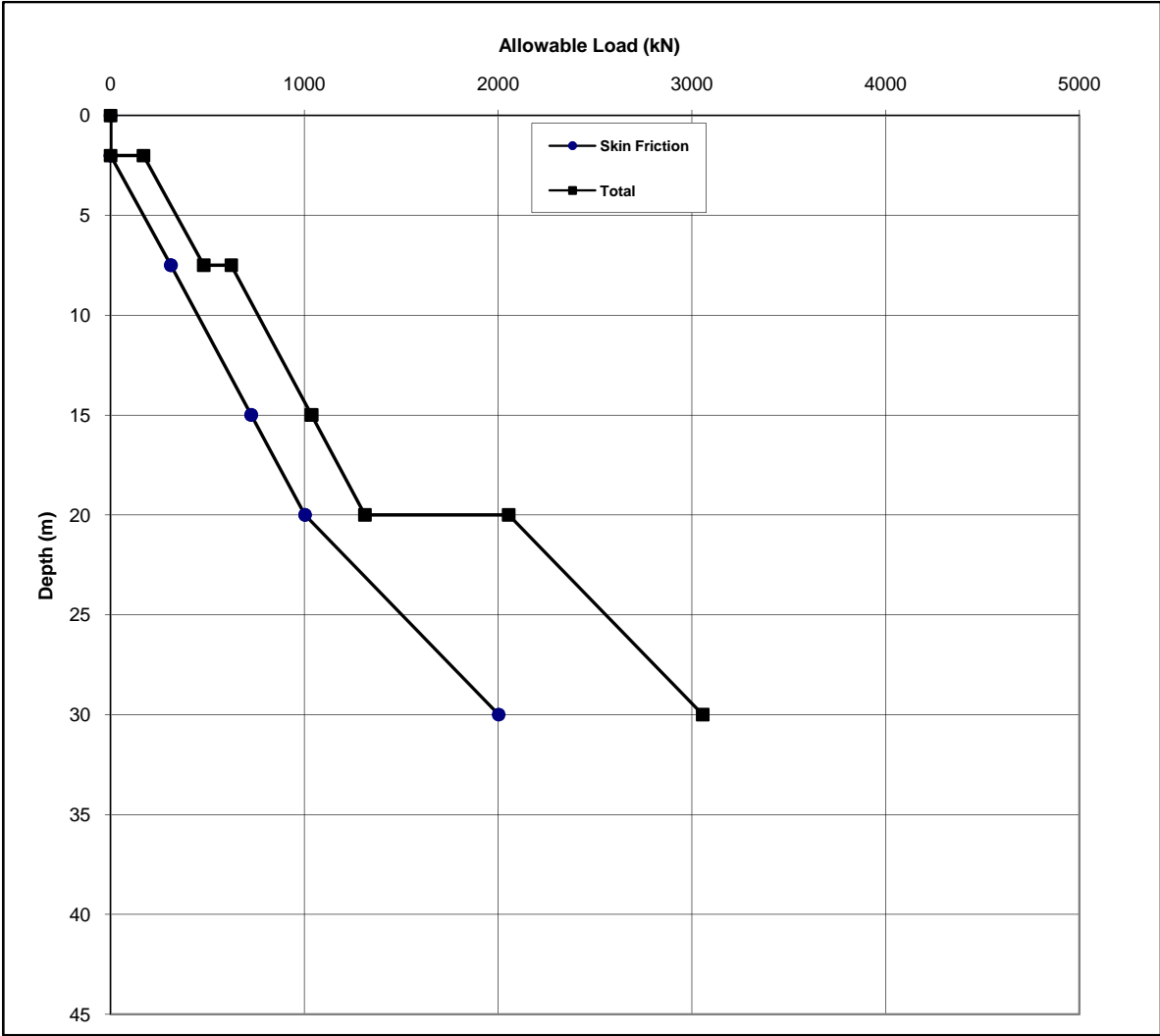
$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

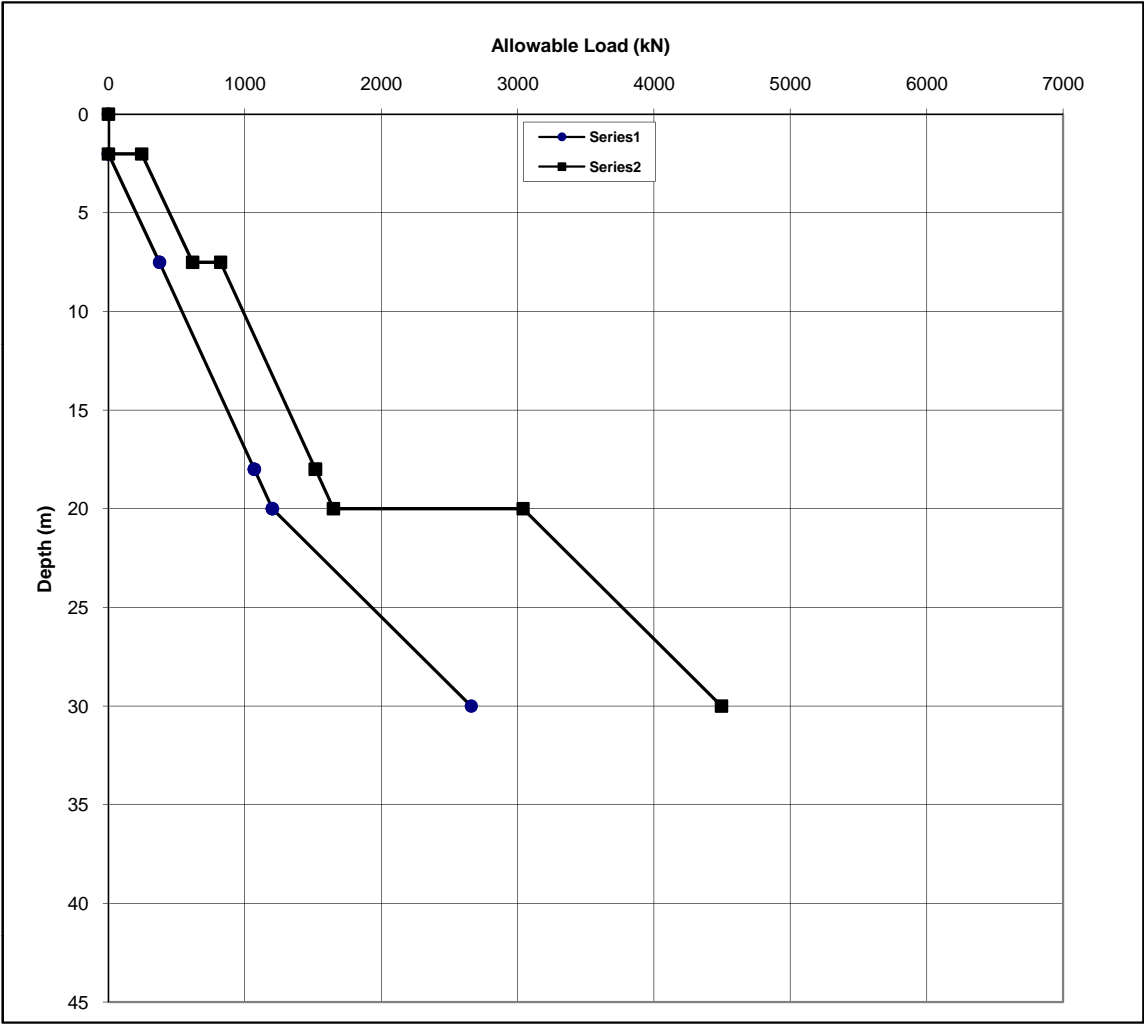
= 550kN (for fixed head condition)
 = 210kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 49+250

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 49+250

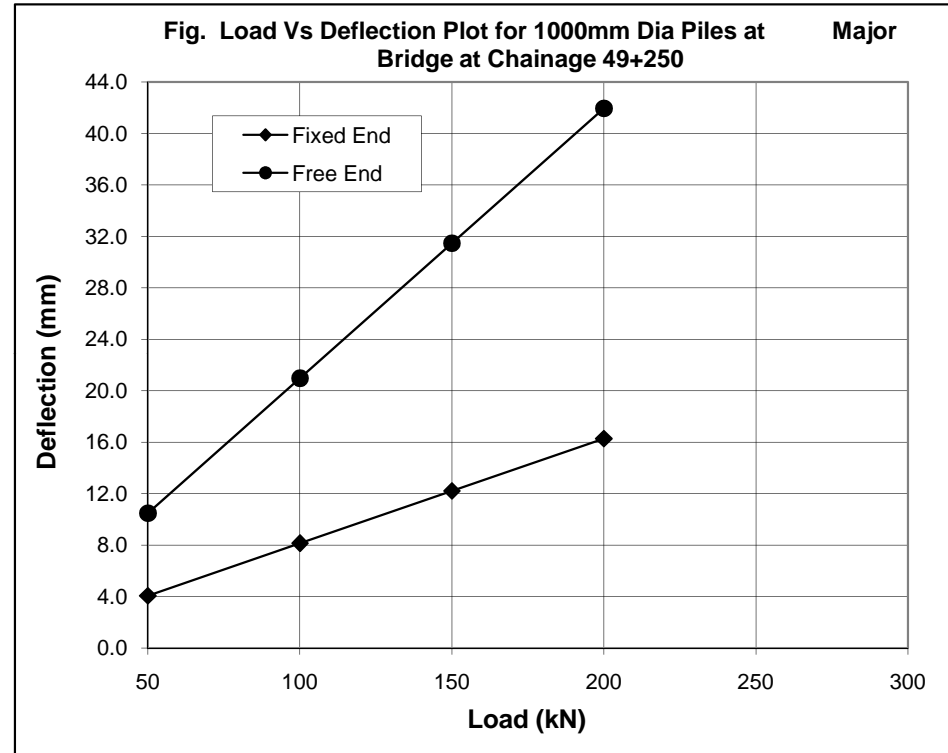
Pile Type=	Bored	Factor of Safety	
Pile Dia (mm)=	1200	End Bearing =	2.5
		Skin Friction =	2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 49+250 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $K_1 = 1.100 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.22 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 495.42$
 $L_f/T = 2.2$ Fixed
 $L_f \text{ (Fixed)} = 1089.93 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f \text{ (Free)} = 941.31 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1 + L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1 + L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	4.07	10.49
100	8.14	20.98
150	12.21	31.47
200	16.28	41.95



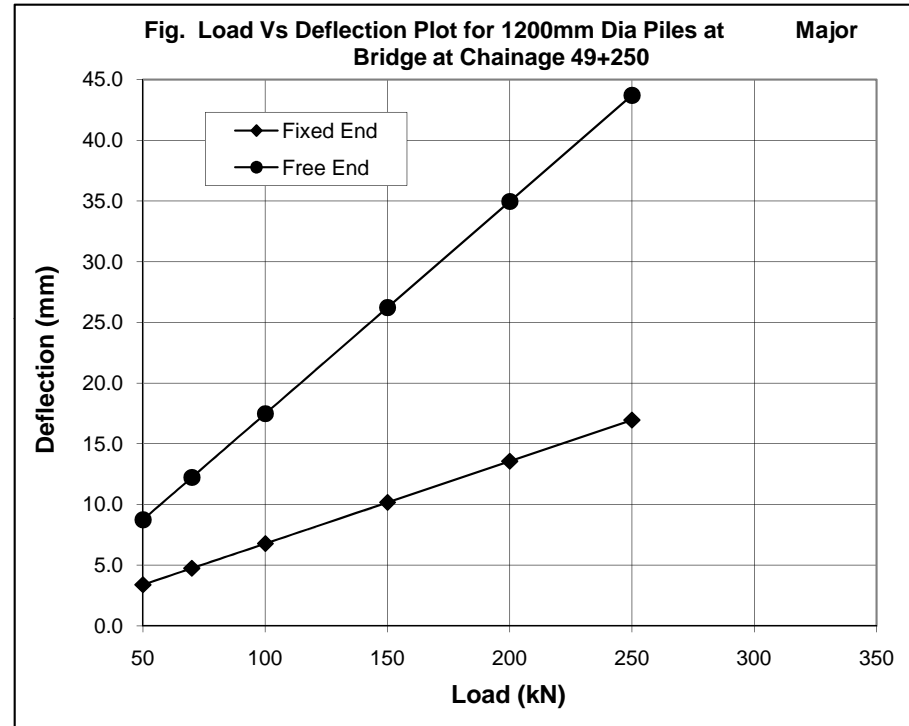
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 120kN (for fixed head condition)
 = 50 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE AT MAJOR BRIDGE CHAINAGE 49+250 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 K₁ 1.100 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²
 K=(K₁*0.3)/(1.5B) 0.183333333 kg/cm³

T= (EI/KB)^{0.25}
 594.51
 L_i/T= 2.2 Fixed
 L_i (Fixed)= 1307.92 cm
 L_i/T= 1.9 Free
 L_i (Free)= 1129.57 cm
 L₁= 0 cm
 d= $\frac{Q(L_1+L_i)^3}{12EI}$ Fixed
 $\frac{Q(L_1+L_i)^3}{3EI}$ Free

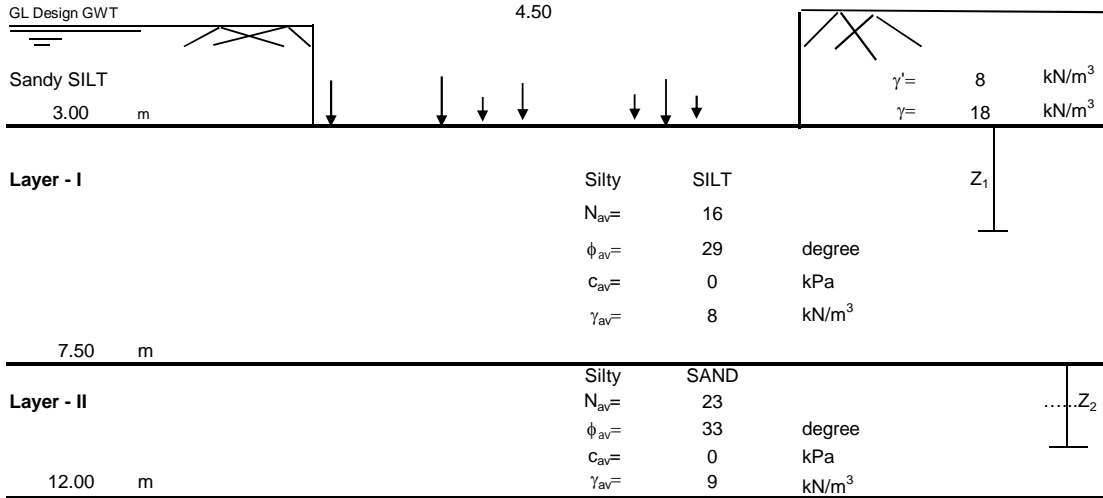
Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.39	8.74
70	4.75	12.24
100	6.78	17.48
150	10.18	26.22
200	13.57	34.96
250	16.96	43.70



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 170kN (for fixed head condition)
 = 70kN (for free head condition)

Calculation for Bearing Capacity at Minor Bridge CH 50+100 km

Footing Size: 5.5x4.5 m
 Depth : 3.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 29 degree

For Layer - I

As ϕ is 29° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma^* D)(N_q \cdot 1) s_q d_q i_q + 0.5 B \gamma N_s s_\gamma d_\gamma i_\gamma) / FS$$

FS=	2.5	w=	0.5		
N_c =	15.5	N_q =	6.88	N_γ =	6.65
S_c =	1.164	S_q =	1.164	S_γ =	0.67
dc=	1+0.2*(D/B)*tan(45+ ϕ /2)=		1.23		
dq=d γ =	1+0.1*(D/B)*tan(45+ ϕ /2)		1.11		
ic=iq=	(1- α /90) ² =		1.00	α =0	
Q_{safe-I} =	113.5	kPa			

Design Bearing Capacity= **110 kPa**

Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

m_v = m²/kN μ_g = for clay

δ (mm) = $[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$

$C = 1.5 \cdot (C_{k0}/p_o) = 285.7$ $C_{k0}/N = 250$ KN/m² for sand } 1st layer I = 0.74

$p_o = 21$ $p = 110.0$ kPa

Rigidity factor = 0.8 Depth Factor, $d_f = 0.82$

δ_1 (mm) = 16.37

Settlement for Layer-II

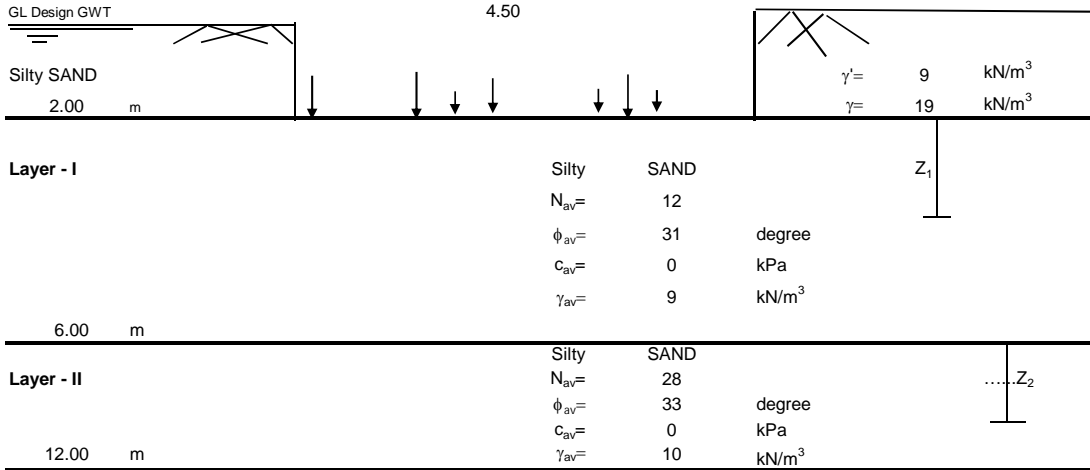
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$					for clay
	$m_v =$	m^2/kN		$\mu_g =$		
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$					for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	129.0		$C_{kd}/N =$	300 kN/m ²	IS:8009 (Part I)
	$p_o =$	80.25		$p =$	110.0	2nd layer I = 0.2
	Rigidity factor =	0.8		Depth Factor, $d_r =$	0.82	
	δ_2 (mm) =	5.55				

Total settlement = 21.91 mm

Allowable Bearing capacity for 25mm settlement = 110.0 KPa

Calculation for Bearing Capacity at Minor Bridge CH 51+100

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 31 degree

For Layer - I

As ϕ is 31° Intermediate shear failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma D)(N_q - 1) s_q d_q i_q + 0.5 B \gamma N_s \gamma_v d_v i_{vv}) / FS$$

FS =	2.5			w =	0.5		
$N_c =$	33.34	$N_q =$	21.38	$N_y =$	27.54	General shear failure	
$N_c =$	17.19	$N_q =$	8.1	$N_y =$	7.59	Local shear failure	
$N_c =$	19.88	$N_q =$	10.31	$N_y =$	10.92	Intermediate condition	
$S_c =$	1.164	$S_q =$	1.164	$S_y =$	0.67		
dc =	$1 + 0.2(D/B) \tan(45 + \phi/2) =$			1.16			
dq = d _γ =	$1 + 0.1(D/B) \tan(45 + \phi/2) =$			1.08			
ic = iq =	$(1 - \alpha/90)^2 =$			1.00	$\alpha = 0$		
Q_{safe-I} =	151.9			kPa			

Design Bearing Capacity = **150 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = \frac{m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor} (0.8)}{m_v \cdot \text{m}^2/\text{kN}} \quad \mu_g = \text{for clay}$$

$$\delta (\text{mm}) = \frac{[2.303(H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor} (0.8)}{C = 1.5 \cdot (C_{kd}/p_o) = 225.0 \quad C_{kd}/N = 250 \text{ KN/m}^2 \quad \text{for sand}} \quad \text{1st layer I} = 0.78$$

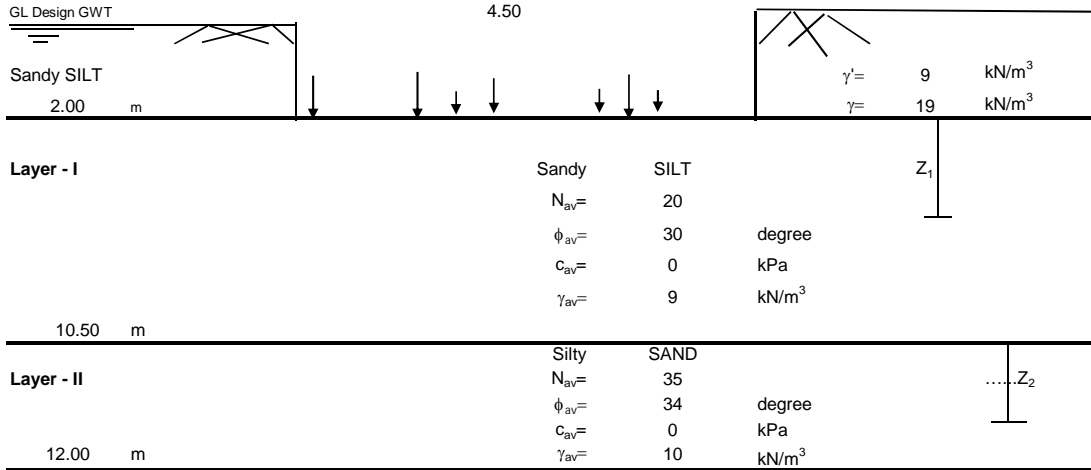
Rigidity factor = 0.8 $p = 150.0$ kPa Depth Factor, $d_f = 0.88$

δ_1 (mm) = 24.08

Settlement for Layer-II			
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$		for clay
	$m_v =$	m^2/kN	$\mu_g =$
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$		for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	150.0	$C_{kd}/N =$ 300 kN/m^2
	$p_o =$ 84	$p =$ 150.0	kPa
	Rigidity factor = 0.8		Depth Factor, $d_f =$ 0.88
	δ_2 (mm) =	8.60	
Total settlement	=	32.68 mm	
Allowable Bearing capacity for 25mm settlement =			116.2 KPa

Calculation for Bearing Capacity at Minor Bridge CH 52+640

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 30 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_{c1} + (\gamma D)(N_q - 1) s_q d_{q1} + 0.5 B \gamma N_s s_\gamma d_{\gamma 1}) / FS$$

FS=	2.5			w=	0.5		
N_c =	16	N_q =	7.25	N_γ =	7.59	Local shear failure	
S_c =	1.164	S_q =	1.164	S_γ =	0.67		
dc=	$1 + 0.2(D/B) \tan(45 + \phi/2) =$		1.15				
dq=d γ =	$1 + 0.1(D/B) \tan(45 + \phi/2) =$		1.08				
ic=iq=	$(1 - \alpha/90)^2 =$	1.00	ig=	$(1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$	
Q_{safe-I}=	103.4	kPa					

Design Bearing Capacity= **100 kPa**

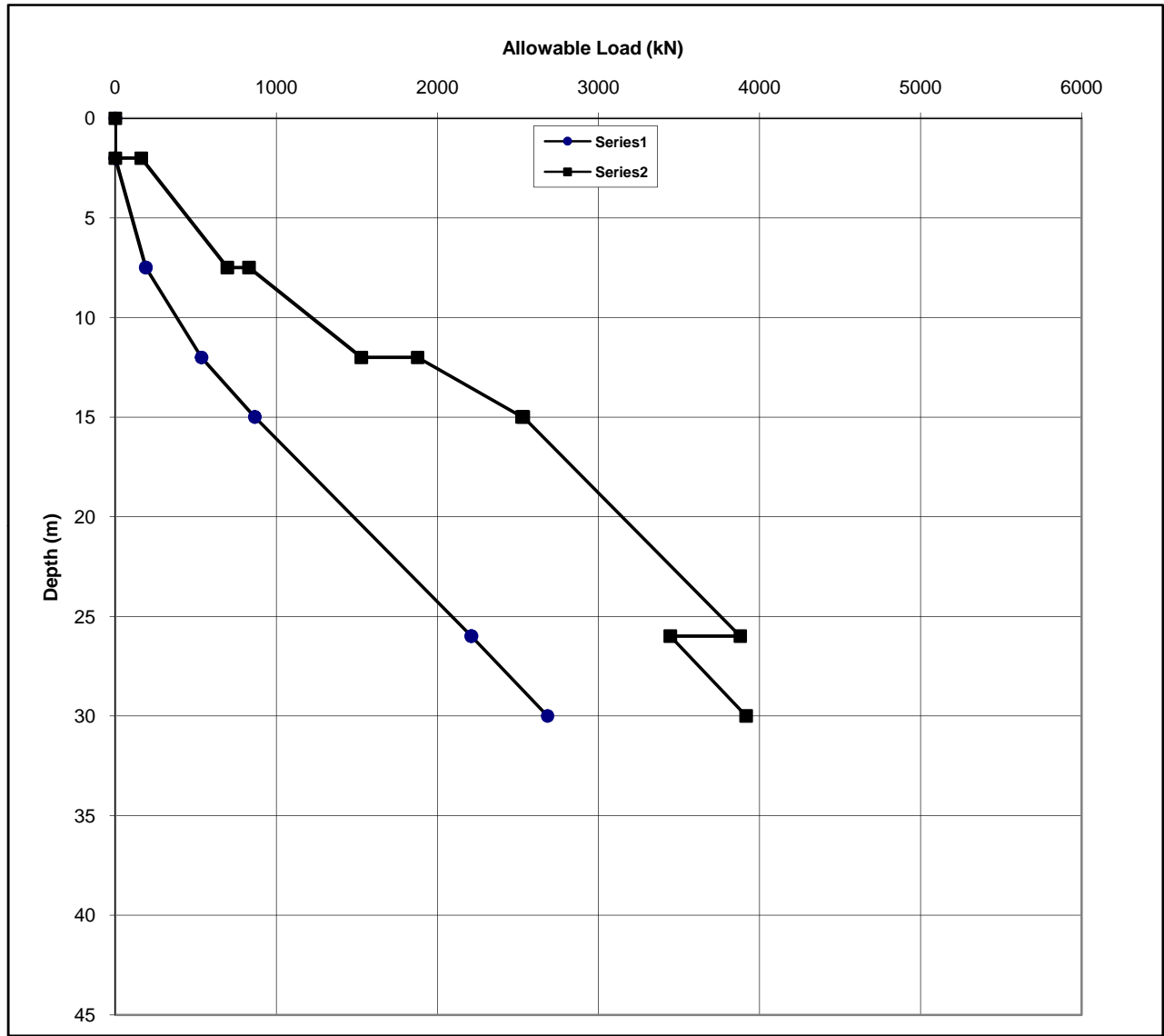
Settlement for Layer - I

δ (mm) = $m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$

m_v =		μ_g =		for clay	
	m ² /kN			}	
δ (mm)=	$[2.303(H/C) \log_{10}((p_o + \Delta p)/p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$			for sand	
C=	$1.5(C_{kd}/\rho_o) =$	186.3	$C_{kd}/N =$	250 kN/m ²	1st layer I = 0.36
ρ_o =	40.25	$\rho =$	100.0	kPa	
Rigidity factor=	0.8	Depth Factor, d_r =		0.88	

δ_1 (mm)= 20.52

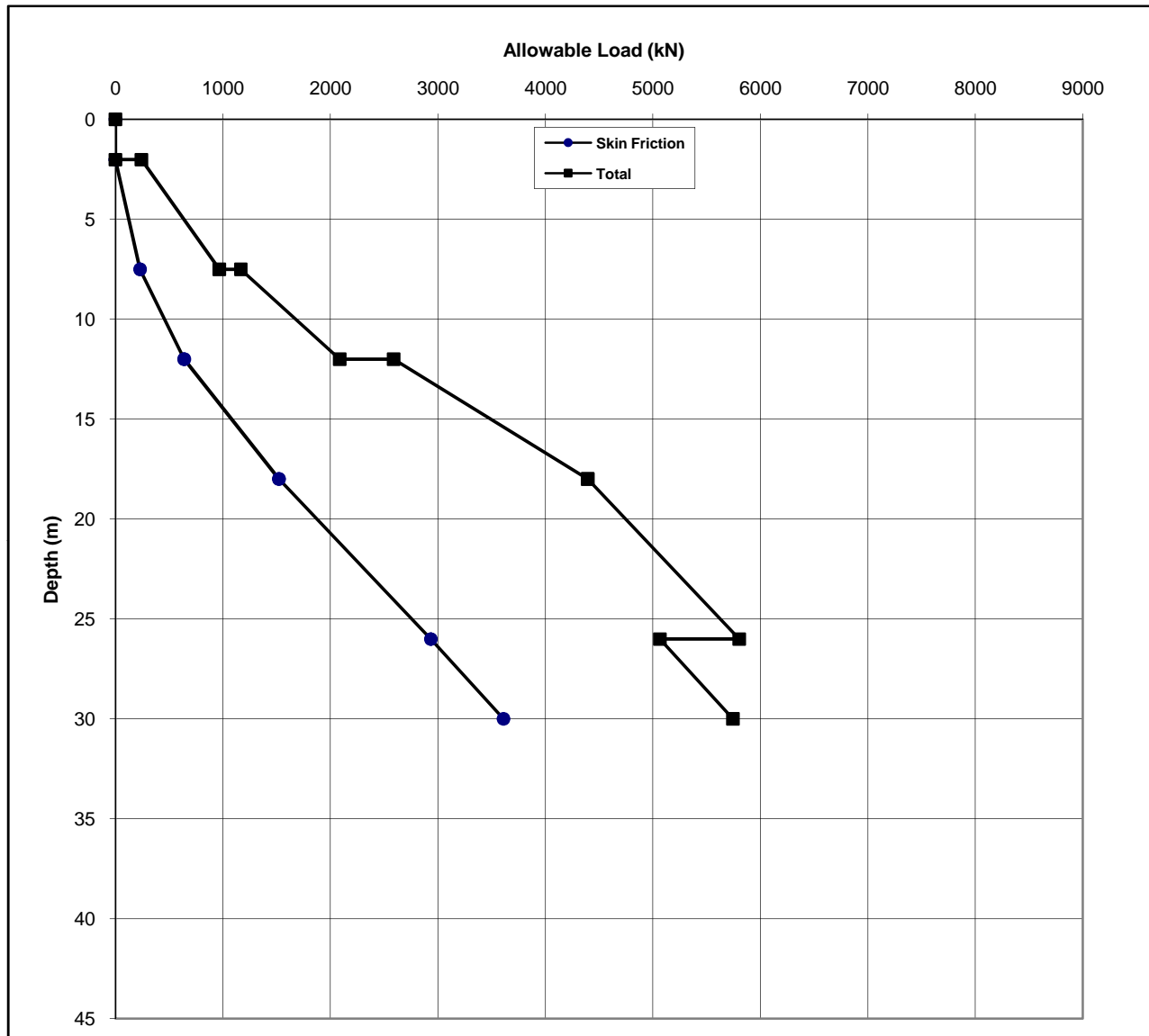
Settlement for Layer-II			
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$		for clay
	$m_v =$	m^2/kN	$\mu_g =$
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$		for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	154.4	$C_{kd}/N =$ 300 kN/m^2
	$p_o =$ 102	$p =$ 100.0	kPa
	Rigidity factor = 0.8		Depth Factor, $d_f =$ 0.88
	δ_2 (mm) =	0.64	
Total settlement	=	21.16 mm	
Allowable Bearing capacity for 25mm settlement =			100.0 KPa



Allowable Pile Capacity at Major Bridge Ch. 54+825

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 54+825

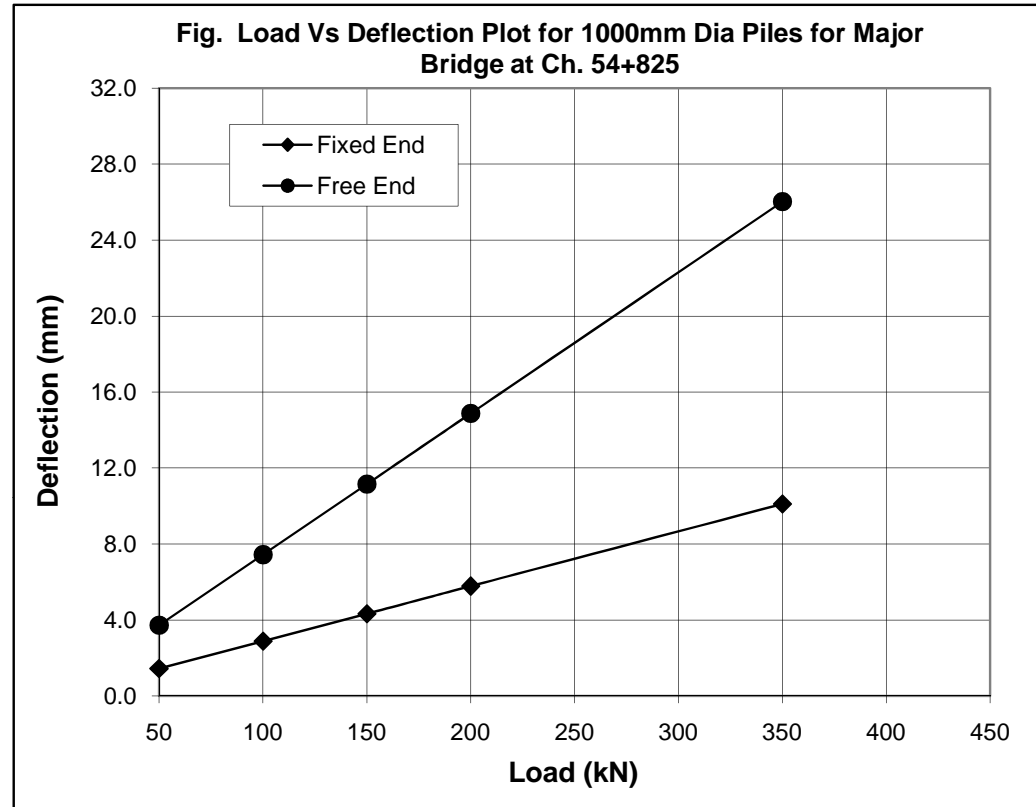
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 η_b = 0.250 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²

T= (EI/ ηh)^{0.2}
 350.65
 L_f/T = 2.2 Fixed
 L_f (Fixed)= 771.44 cm
 L_f/T = 1.9 Free
 L_f (Free)= 666.24 cm
 L_1 = 0 cm
 d= $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03

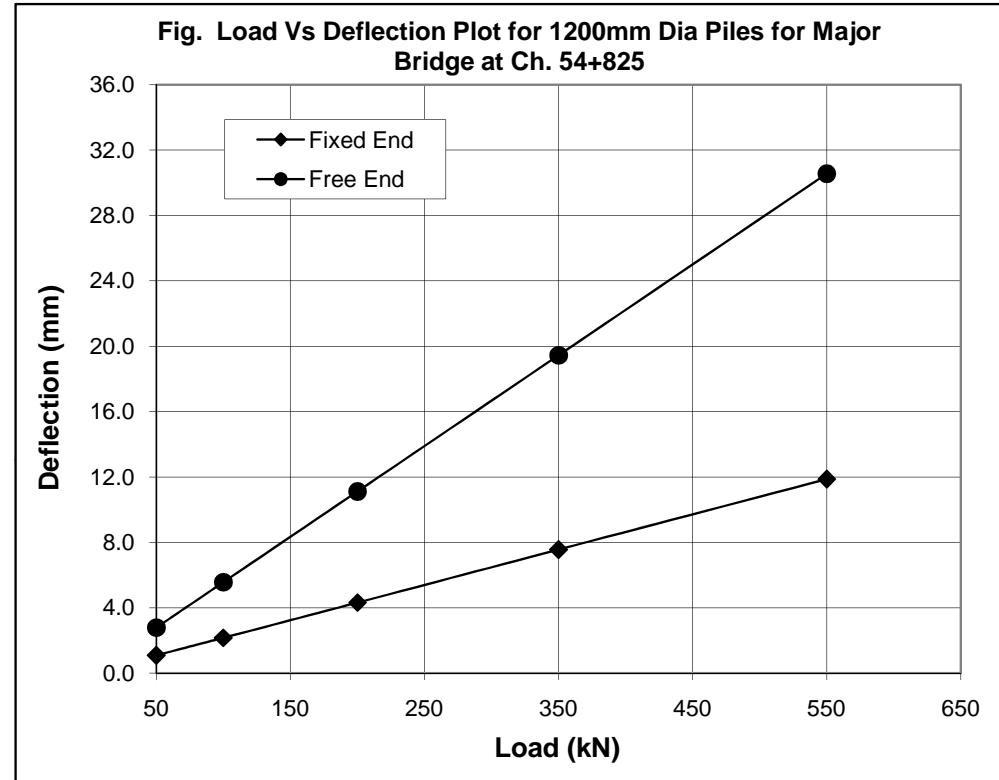


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

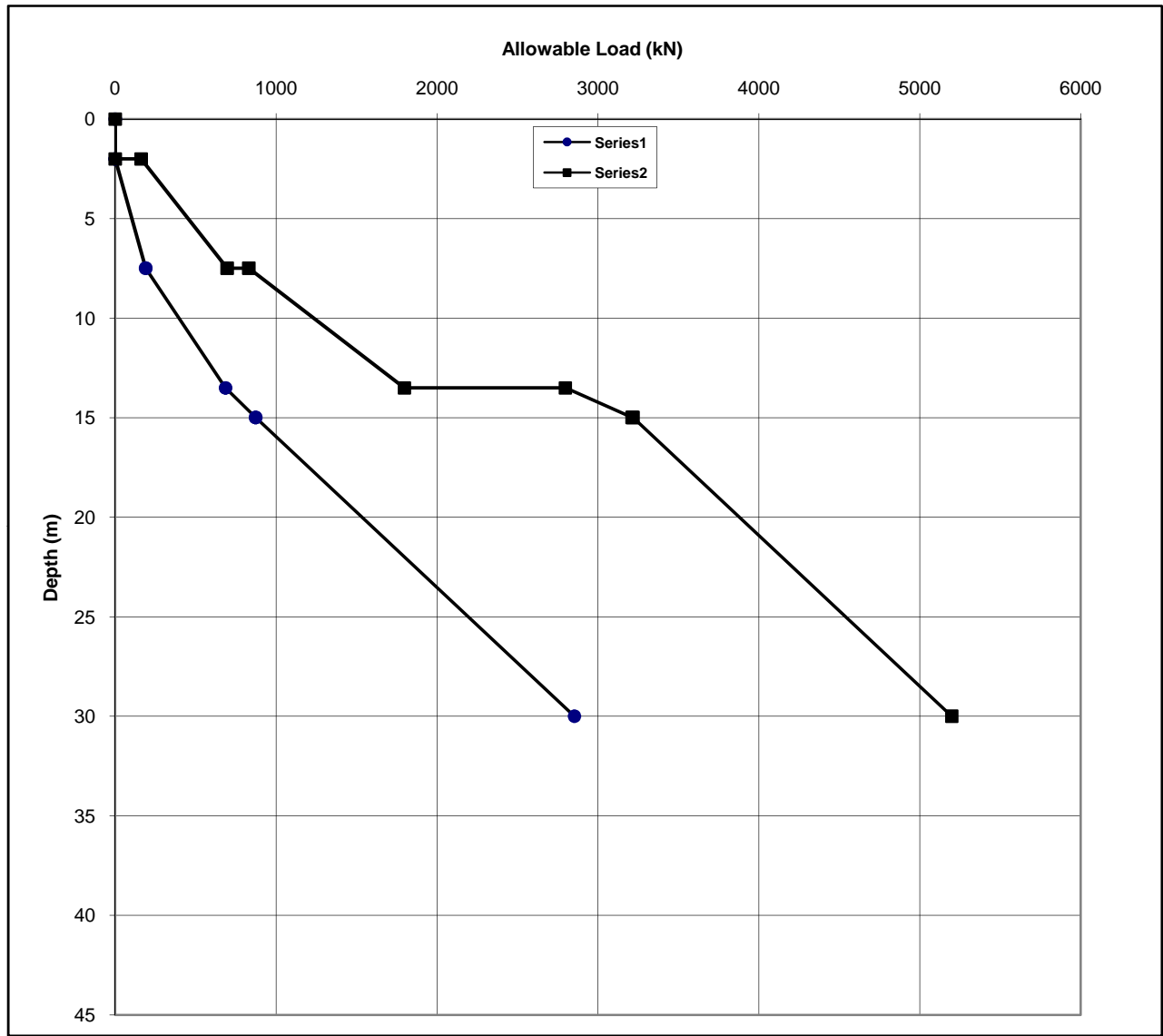
$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

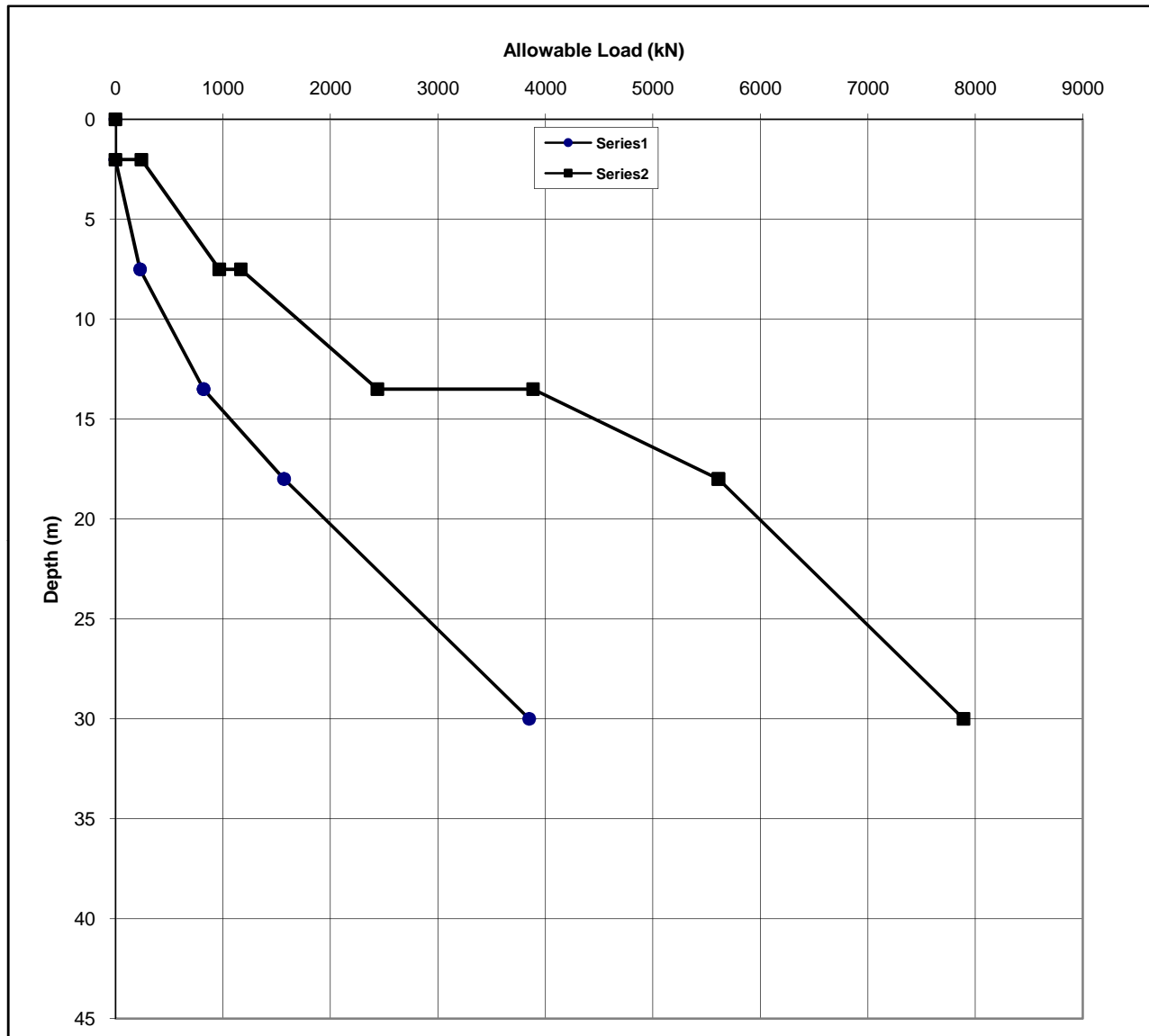
= 550kN (for fixed head condition)
 = 210kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 55+850

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 55+850

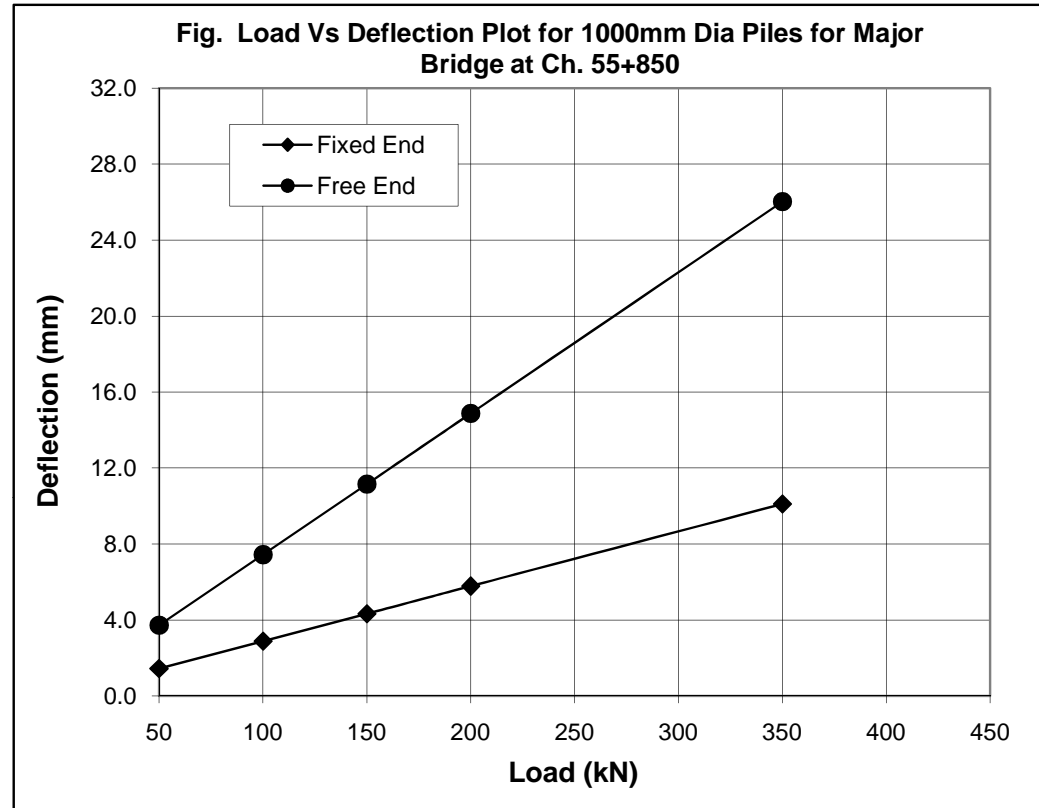
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 $\eta_b = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 350.65$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 771.44$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 666.24$ cm
 $L_1 = 0$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03

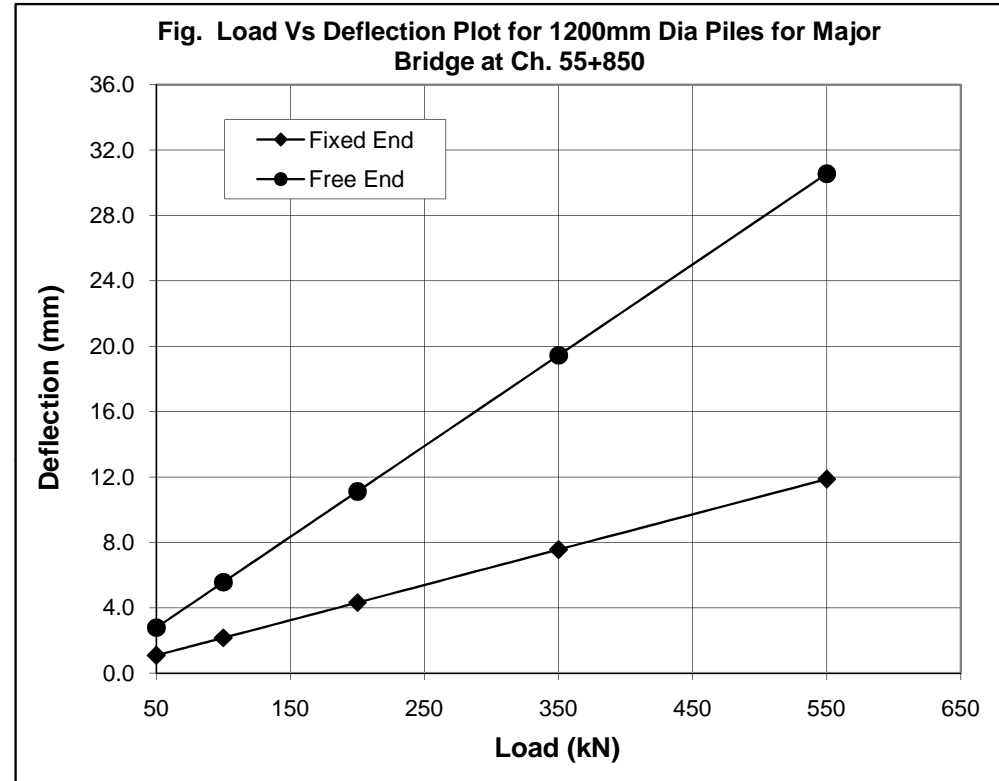


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

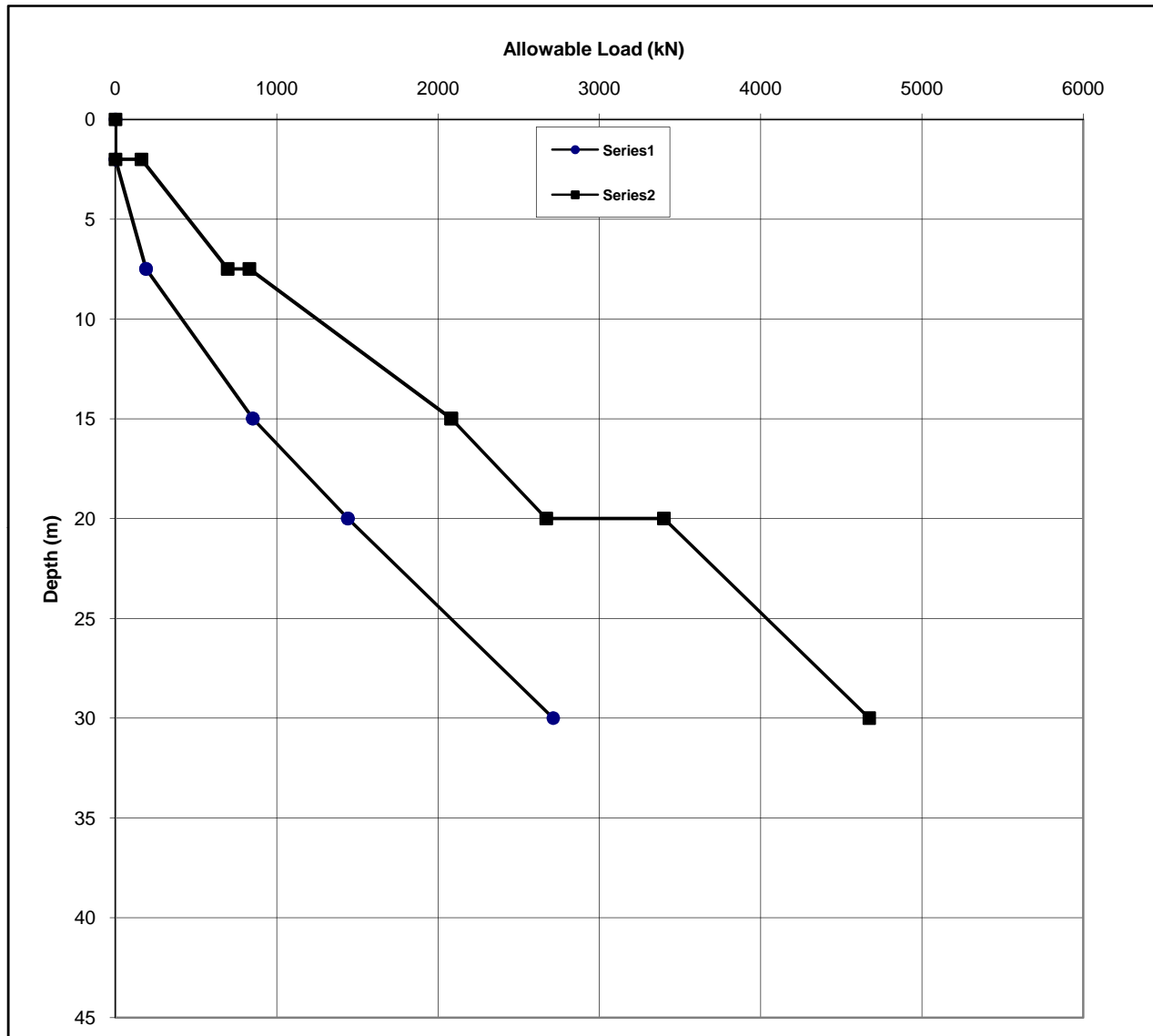
$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

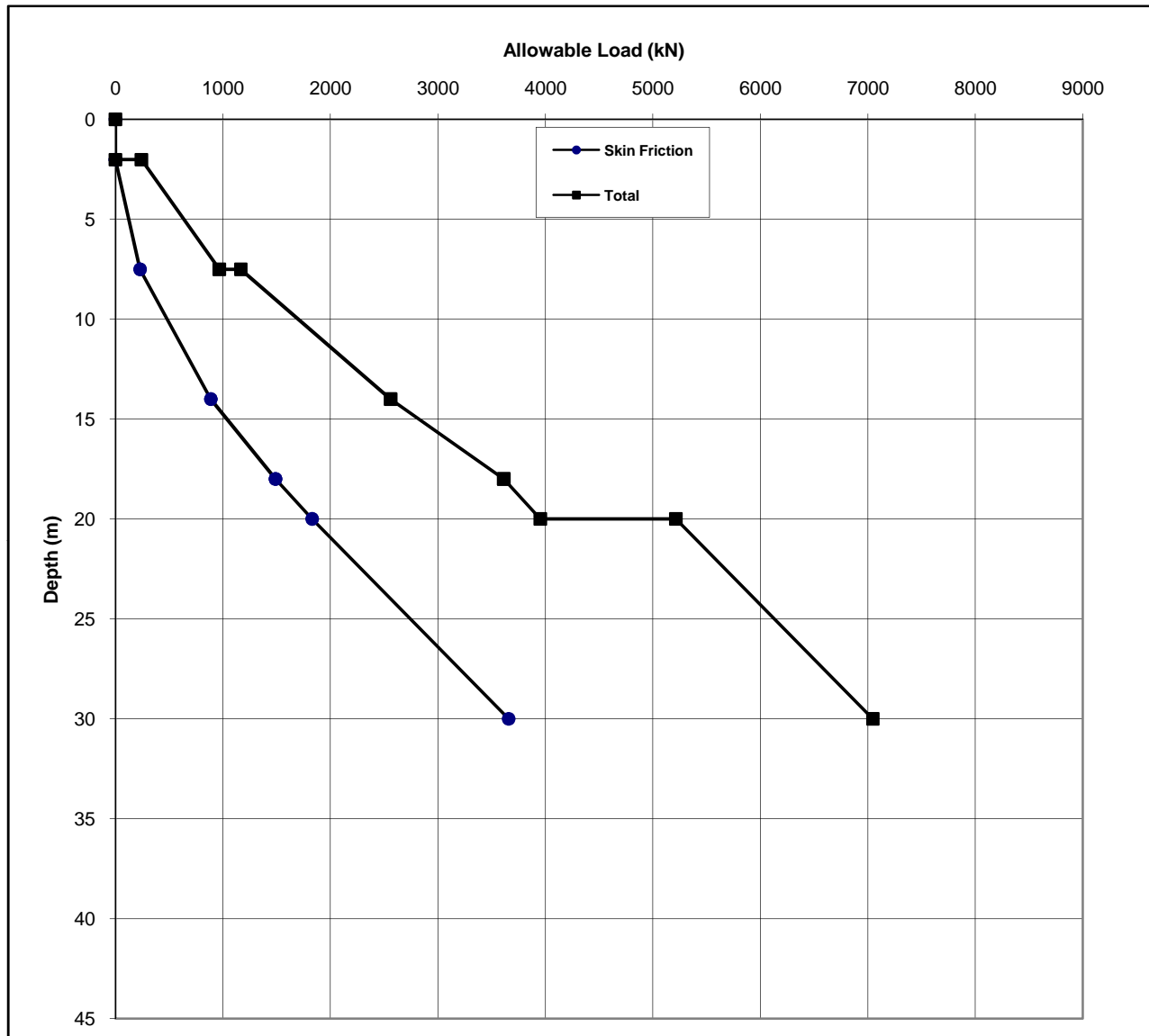
= 550kN (for fixed head condition)
 = 210kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 56+780

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety = 2.5
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 56+780

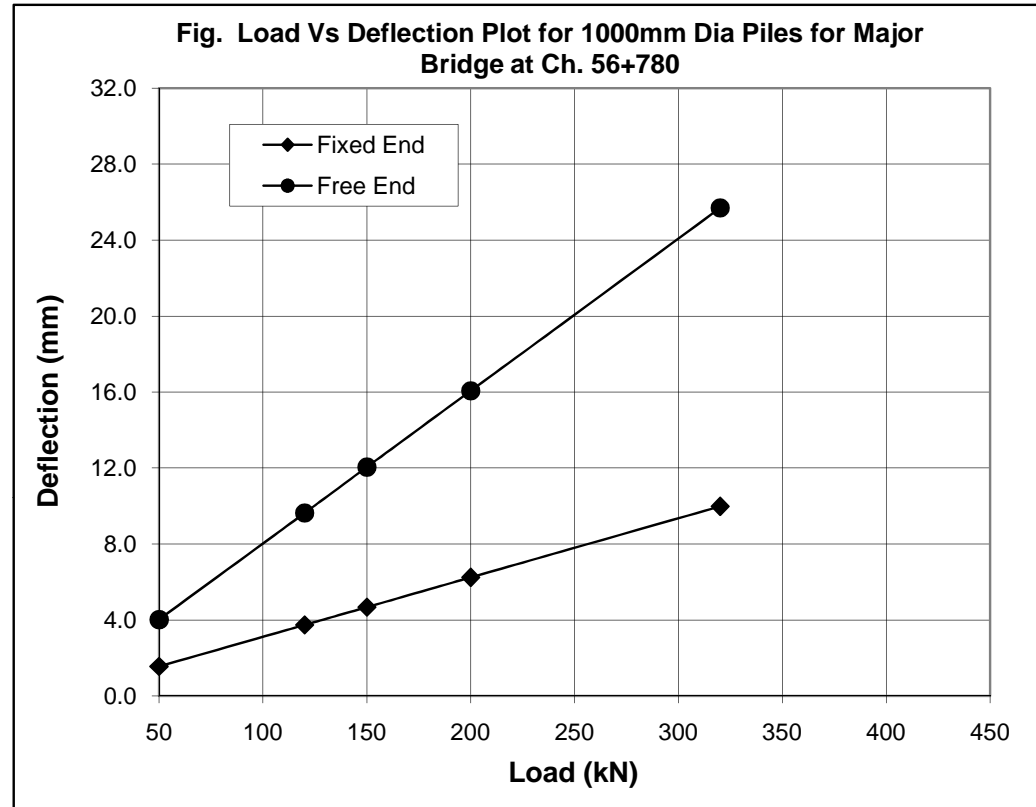
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 η_b = 0.220 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²

T= (EI/ ηh)^{0.2}
 359.73
 L_f/T = 2.2 Fixed
 L_f (Fixed)= 791.42 cm
 L_f/T = 1.9 Free
 L_f (Free)= 683.50 cm
 L_1 = 0 cm
 d= $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.56	4.02
120	3.74	9.64
150	4.68	12.05
200	6.23	16.06
320	9.97	25.70

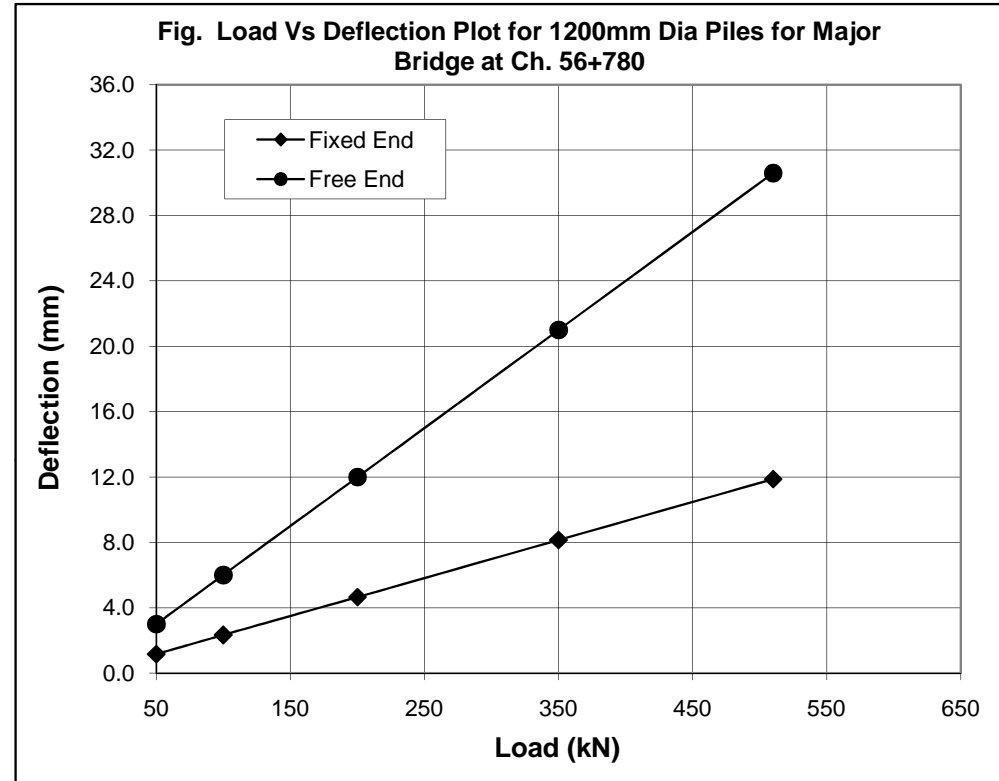


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 320kN (for fixed head condition)
 = 120 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.220 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

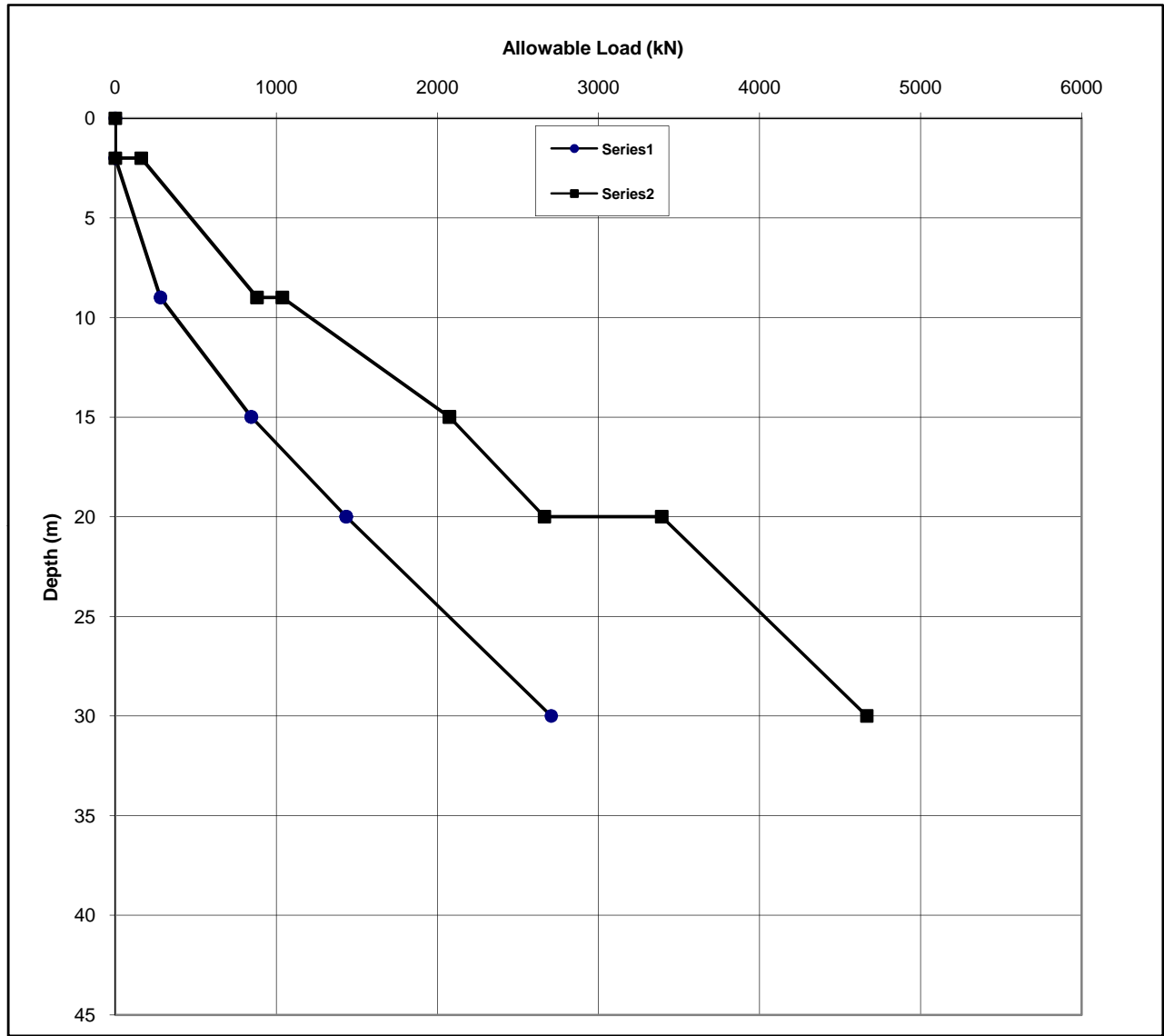
$T = (EI/\eta h)^{0.2} = 416.22$
 $L_f(\text{Fixed})/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 915.69$ cm
 $L_f(\text{Free})/T = 1.9$ Free
 $L_f(\text{Free}) = 790.83$ cm
 $L_1 = 0$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.16	3.00
100	2.33	6.00
200	4.66	12.00
350	8.15	21.00
510	11.87	30.59



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

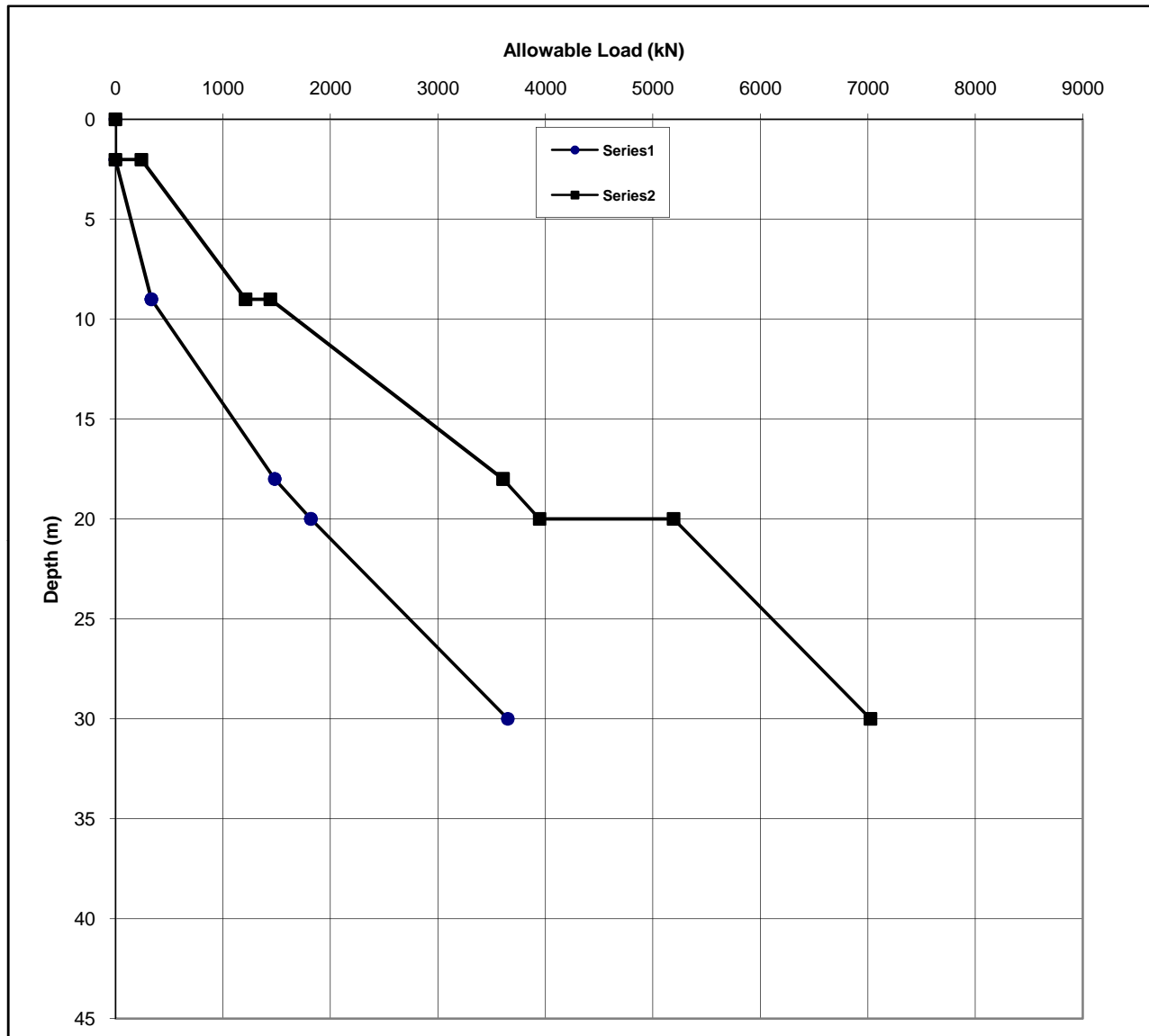
= 510kN (for fixed head condition)
 = 200kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 57+555

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 57+555

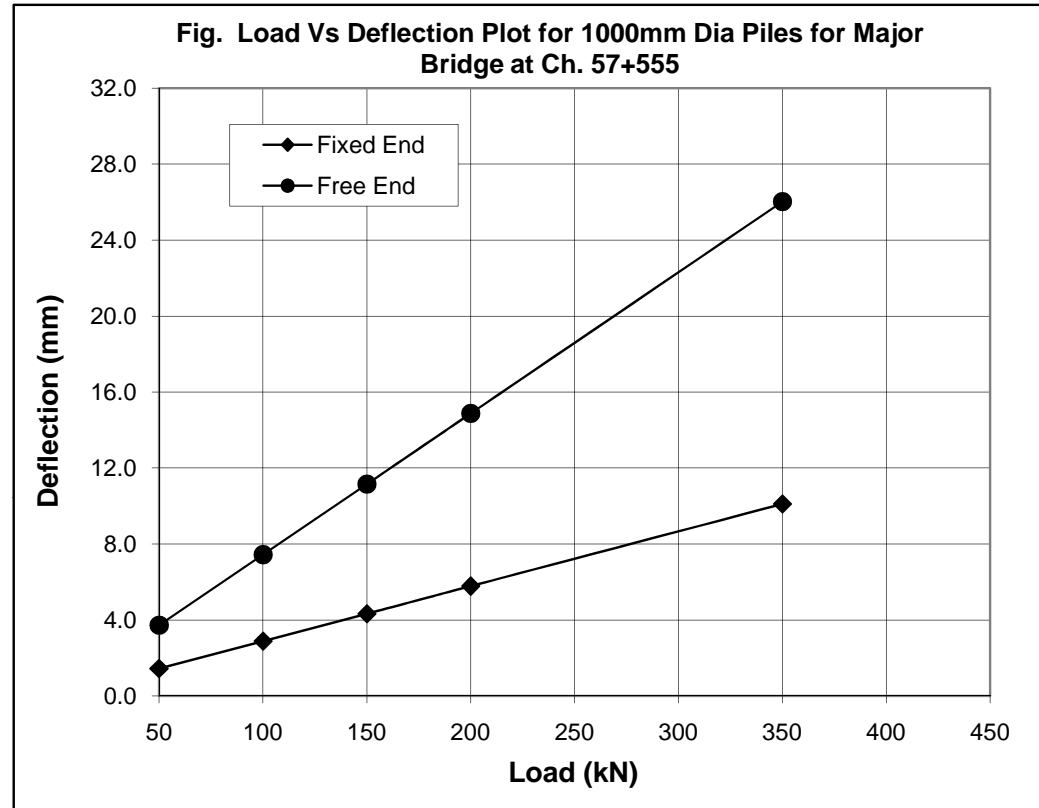
Pile Type= Bored
Pile Dia (mm)= 1200

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5

D= 100 cm
 $\eta_b = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 350.65$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 771.44 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 666.24 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03

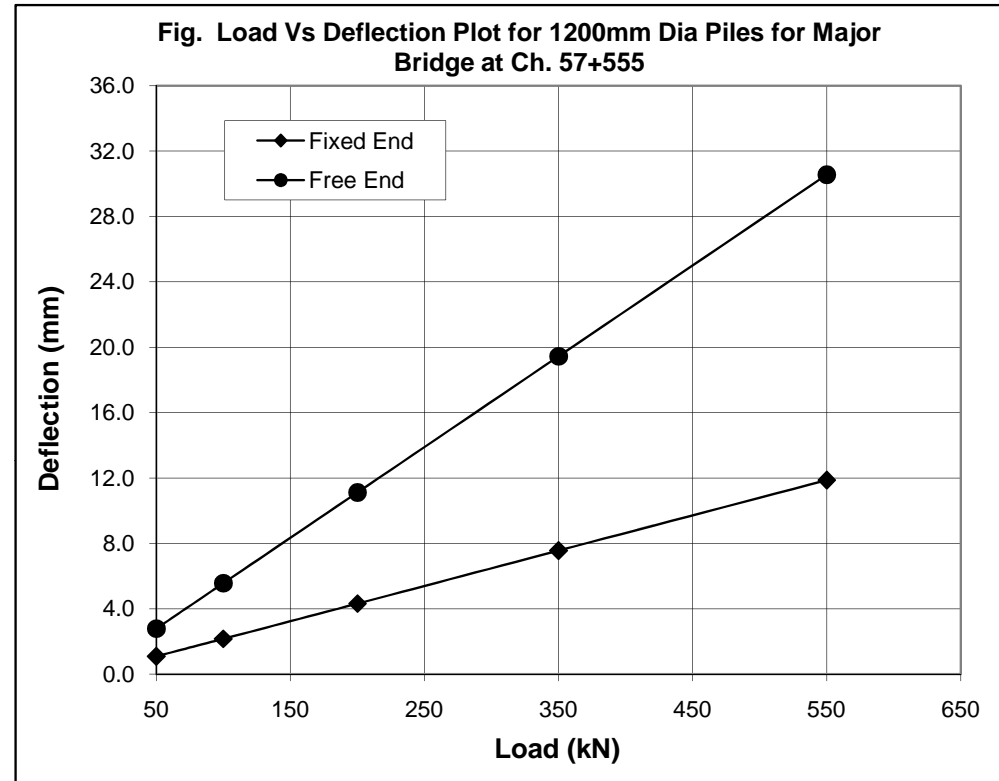


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827\text{E}+12 \text{ kg-cm}^2$

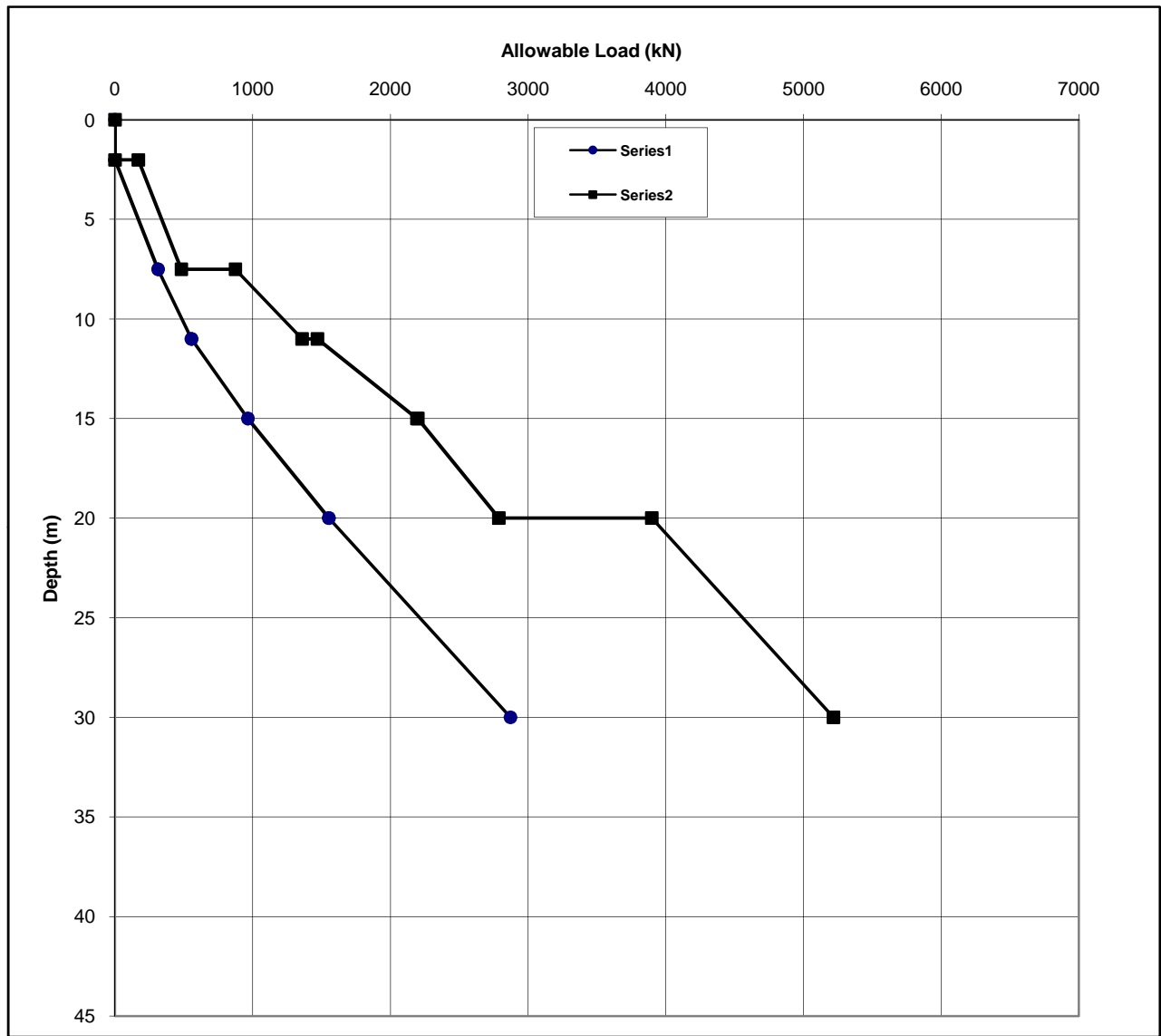
$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

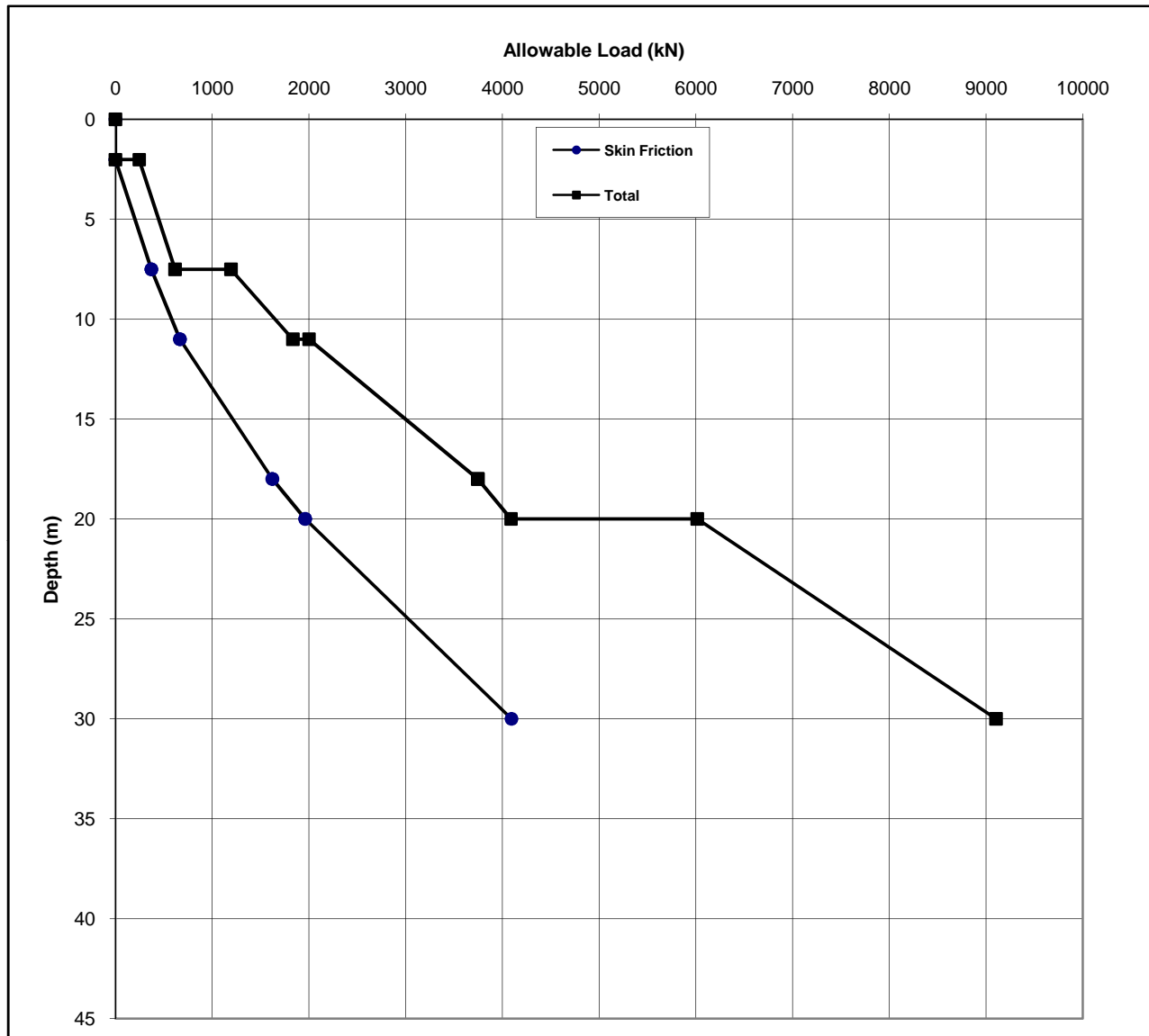
= 550kN (for fixed head condition)
 = 210kN (for free head condition)



Allowable Pile Capacity Major Bridge at Chainage 58+400

Pile Type= Bored
Pile Dia (mm)= 1000

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 58+400

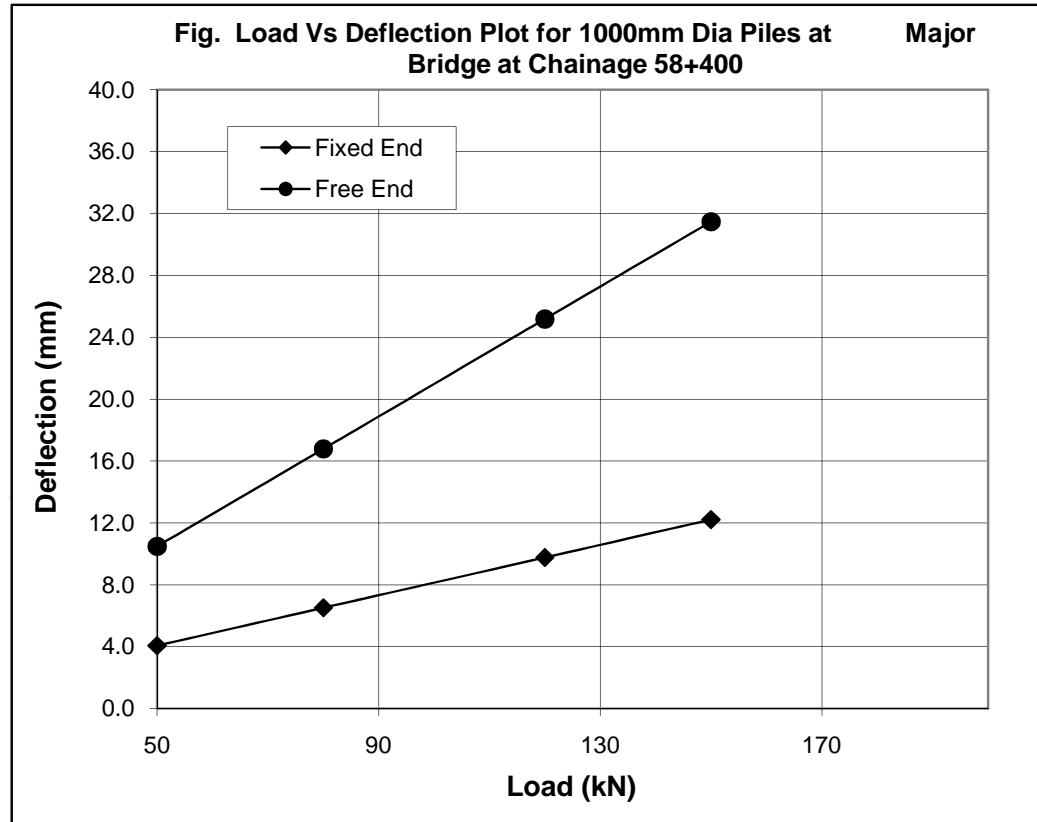
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 K_1 1.100 kg/cm³
 E= 270000 kg/cm²
 I= 4908738.5 cm⁴
 EI= 1.32536E+12 kg-cm²
 $K=(K_1*0.3)/(1.5B)$ 0.22 kg/cm³

T= (EI/KB)^{0.25}
 495.42
 $L_f/T=$ 2.2 Fixed
 L_f (Fixed)= 1089.93 cm
 $L_f/T=$ 1.9 Free
 L_f (Free)= 941.31 cm
 $L_1=$ 0 cm
 $d=$ $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	4.07	10.49
80	6.51	16.78
120	9.77	25.17
150	12.21	31.47

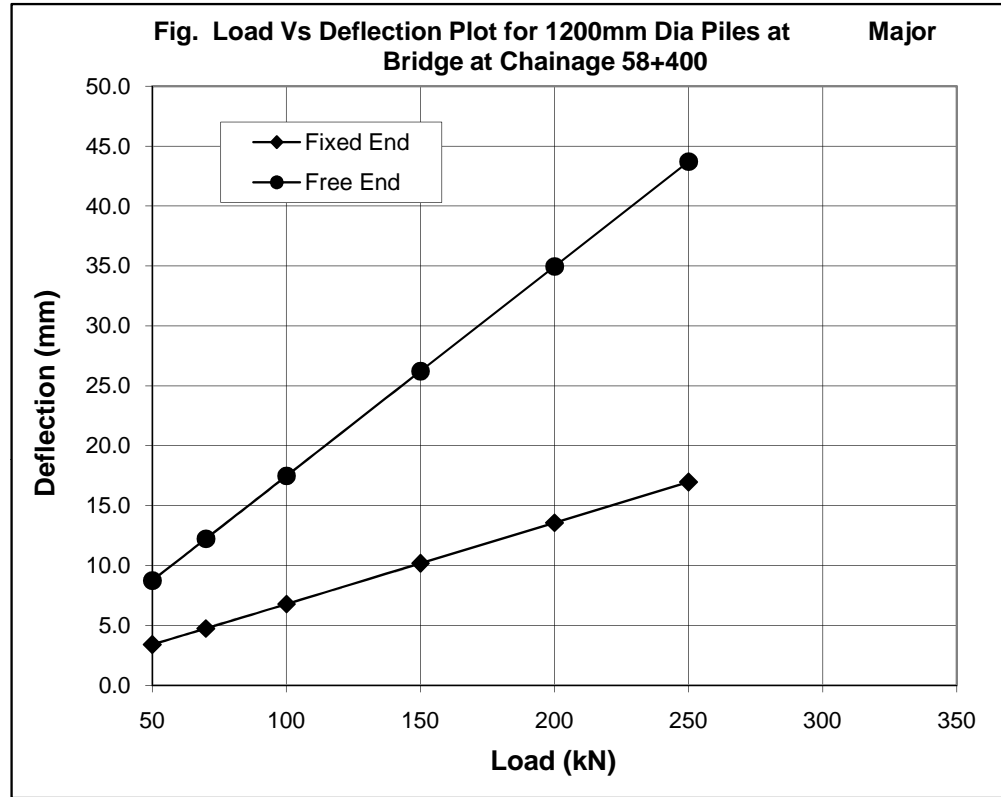


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 120kN (for fixed head condition)
 = 50 kN (for free head condition)

D= 120 cm
 K_1 1.100 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²
 $K=(K_1*0.3)/(1.5B)$ 0.183333333 kg/cm³

T= (EI/KB)^{0.25}
 594.51
 $L_f/T=$ 2.2 Fixed
 L_f (Fixed)= 1307.92 cm
 $L_f/T=$ 1.9 Free
 L_f (Free)= 1129.57 cm
 $L_1=$ 0 cm
 $d=$ $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

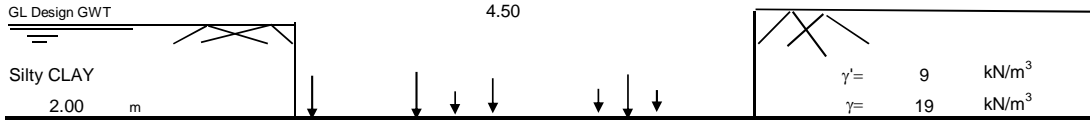
Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.39	8.74
70	4.75	12.24
100	6.78	17.48
150	10.18	26.22
200	13.57	34.96
250	16.96	43.70



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 180kN (for fixed head condition)
 = 70kN (for free head condition)

Calculation for Bearing Capacity at Minor Bridge CH 59+305

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Layer	Soil Type	N_{av}	ϕ_{av}	c_{av}	γ_{av}	Depth (m)
Layer - I	Silty CLAY	10	0 degree	50 kPa	9 kN/m ³	0 to 4.50
Layer - II	Silty SAND	24	31 degree	0 kPa	10 kN/m ³	4.50 to 8.00
Layer - III	silty CLAY	30	0 degree	130 kPa	10 kN/m ³	8.00 to 12.00

Safe Bearing Capacity from Shear Failure

Design $\phi = 0$ degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma \cdot D)(N_q - 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS =	2.5	$N_q =$	1	$w =$	0.5	$N_{\gamma} =$	0.00	Local shear failure
$S_c =$	1.164	$S_q =$	1.164	$S_{\gamma} =$	0.67			
$d_c =$	$1 + 0.2 \cdot (D/B) \cdot \tan(45 + \phi/2) =$				1.09			
$d_q = d_{\gamma} =$	$1 + 0.1 \cdot (D/B) \cdot \tan(45 + \phi/2)$				1.04			
$i_c = i_q =$	$(1 - \alpha/90)^2 =$	1.00		$i_{\gamma} =$	$(1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$	
$Q_{\text{safe-I}} =$		130.3	kPa					

Design Bearing Capacity = **130 kPa**

Settlement for Layer - I

$$\delta (\text{mm}) = m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$m_v = 0.00014$ $\mu_g = 0.7$

for clay

$$\delta (\text{mm}) = [2.303 \cdot (H/C) \cdot \log_{10} (p_o + \Delta p / p_o)] \cdot d_r \cdot \text{Rigidity Factor (0.8)}$$

$C = 1.5 \cdot (C_{kd} / p_o) = 0.0$ $C_{kd} / N =$ $p_o = 13.25$ $p = 130.0$ $\text{Depth Factor, } d_r =$

for sand

1st layer I = 0.92

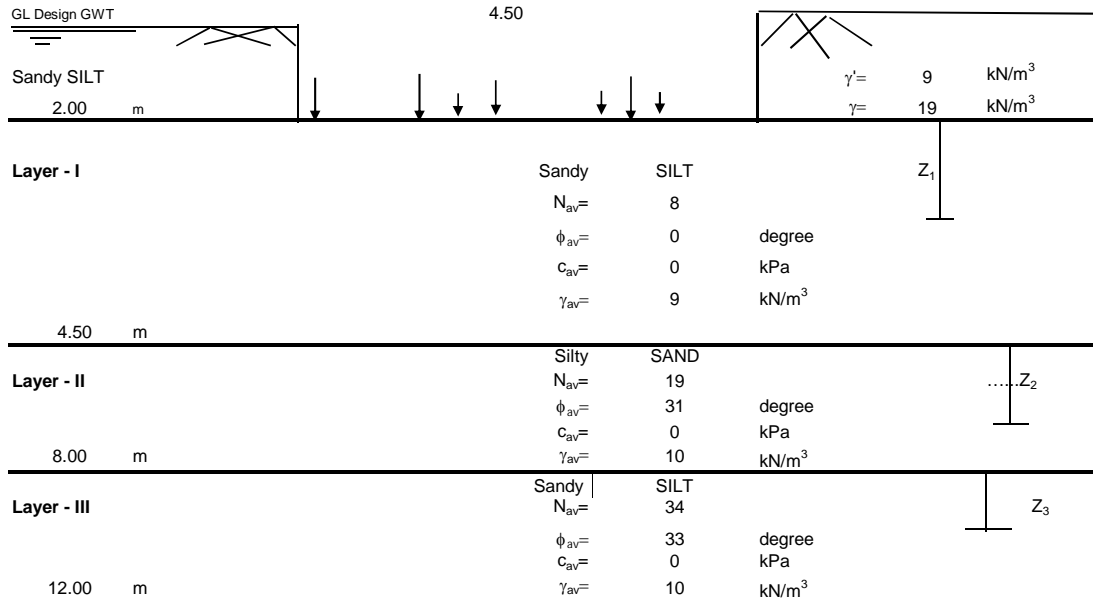
Rigidity factor = 0.8 $\text{Depth Factor, } d_r = 0.88$

$\delta_1 (\text{mm}) = 20.63$

Settlement for Layer-II				
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for clay
	$m_v =$	m^2/kN	$\mu_g =$	
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	186.2	$C_{kd}/N =$	300 KN/m^2
	$p_o =$	58	$p =$	130.0 kPa
	Rigidity factor =	0.8	Depth Factor, $d_f =$	0.88
	δ_2 (mm) =	7.83		
Settlement for Layer-III				
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for clay
	$m_v =$	0.00005	m^2/kN	$\mu_g =$
				0.7
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	0.0	$C_{kd}/N =$	KN/m^2
	$p_o =$	95.5	$p =$	130.0 kPa
	Rigidity factor =	0.8	Depth Factor, $d_f =$	0.88
	δ_3 (mm) =	1.95		
Total settlement	=	30.40 mm		
Allowable Bearing capacity for 25mm settlement =				107.1 KPa

Calculation for Bearing Capacity at Minor Bridge CH 62+160

Footing Size: 5.5x4.5 m
 Depth : 2.00 m



Safe Bearing Capacity from Shear Failure

Design ϕ = 28 degree

For Layer - I

As ϕ is 30° local failure is considered

$$Q(\text{safe}) = (cN_c s_c d_c i_c + (\gamma D)(N_q - 1) s_q d_q i_q + 0.5 B \gamma N_{\gamma} s_{\gamma} d_{\gamma} i_{\gamma}) / FS$$

FS=	2.5	N_q =	6.4	N_{γ} =	5.39	Local shear failure
N_c =	14.83	S_q =	1.164	S_{γ} =	0.67	
S_c =	1.164					
d_c =	$1 + 0.2 * (D/B) * \tan(45 + \phi/2) =$				1.09	
$d_q = d_{\gamma} =$	$1 + 0.1 * (D/B) * \tan(45 + \phi/2)$				1.04	
$i_c = i_q =$	$(1 - \alpha/90)^2 =$	1.00		$i_g = (1 - \alpha/\phi)^2 =$	1.00	$\alpha = 0$
$Q_{\text{safe-I}} =$		79.7	kPa			

Design Bearing Capacity= **80 kPa**

Settlement for Layer - I

δ (mm) = $m_v * H * \Delta p * \mu_g * d_f$ Rigidity Factor (0.8)

$m_v =$ m²/kN $\mu_g =$ for clay

δ (mm) = $[2.303 * (H/C) * \log_{10}((p_o + \Delta p)/p_o)] * d_f$ Rigidity Factor (0.8)

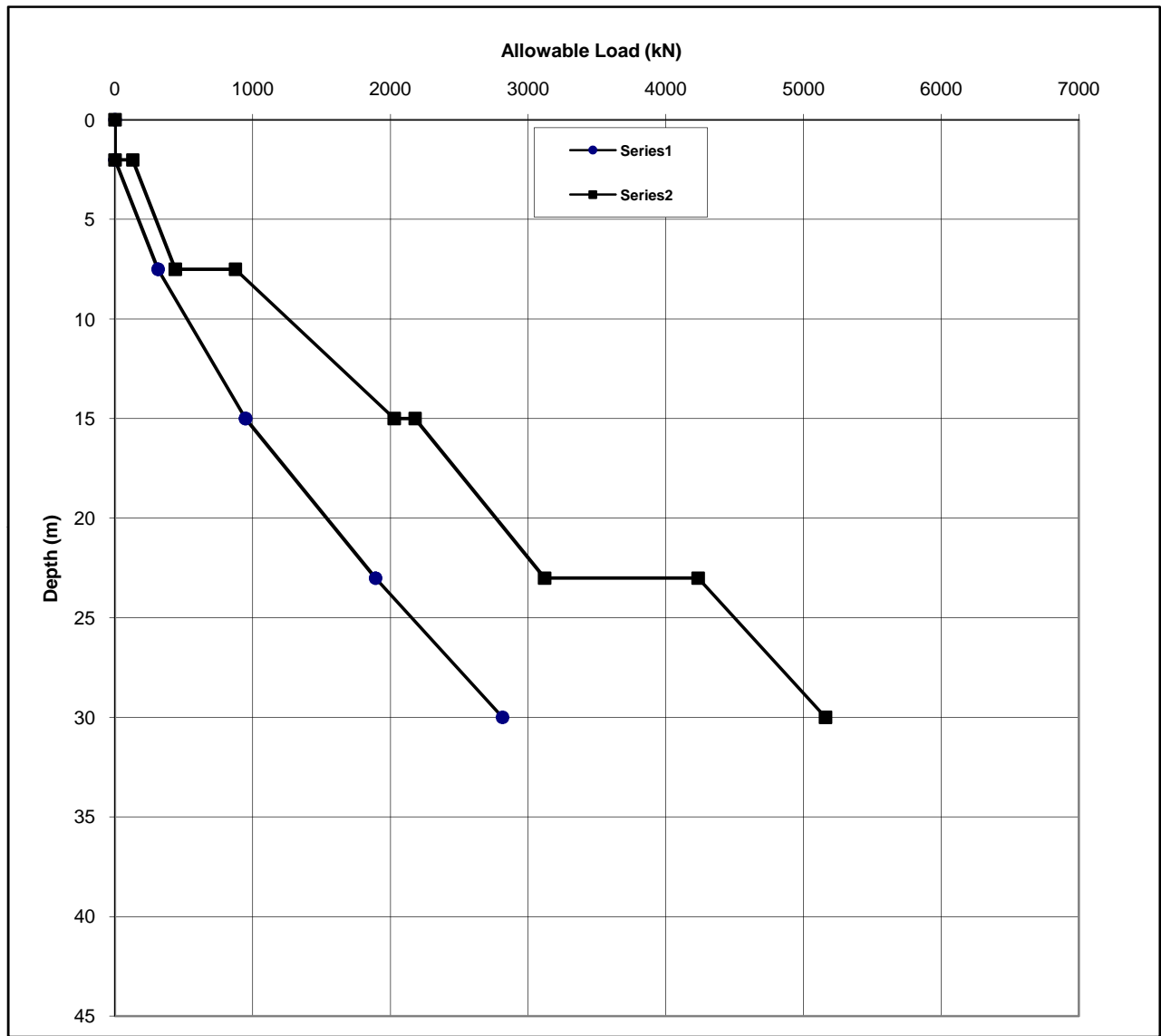
$C = 1.5 * (C_{kd}/p_o) =$ 226.4 $C_{kd}/N =$ 250 KN/m² for sand } 1st layer I = 0.92

$p_o =$ 13.25 $p =$ 80.0 kPa

Rigidity factor = 0.8 Depth Factor, $d_f =$ 0.88

δ_1 (mm) = 14.62

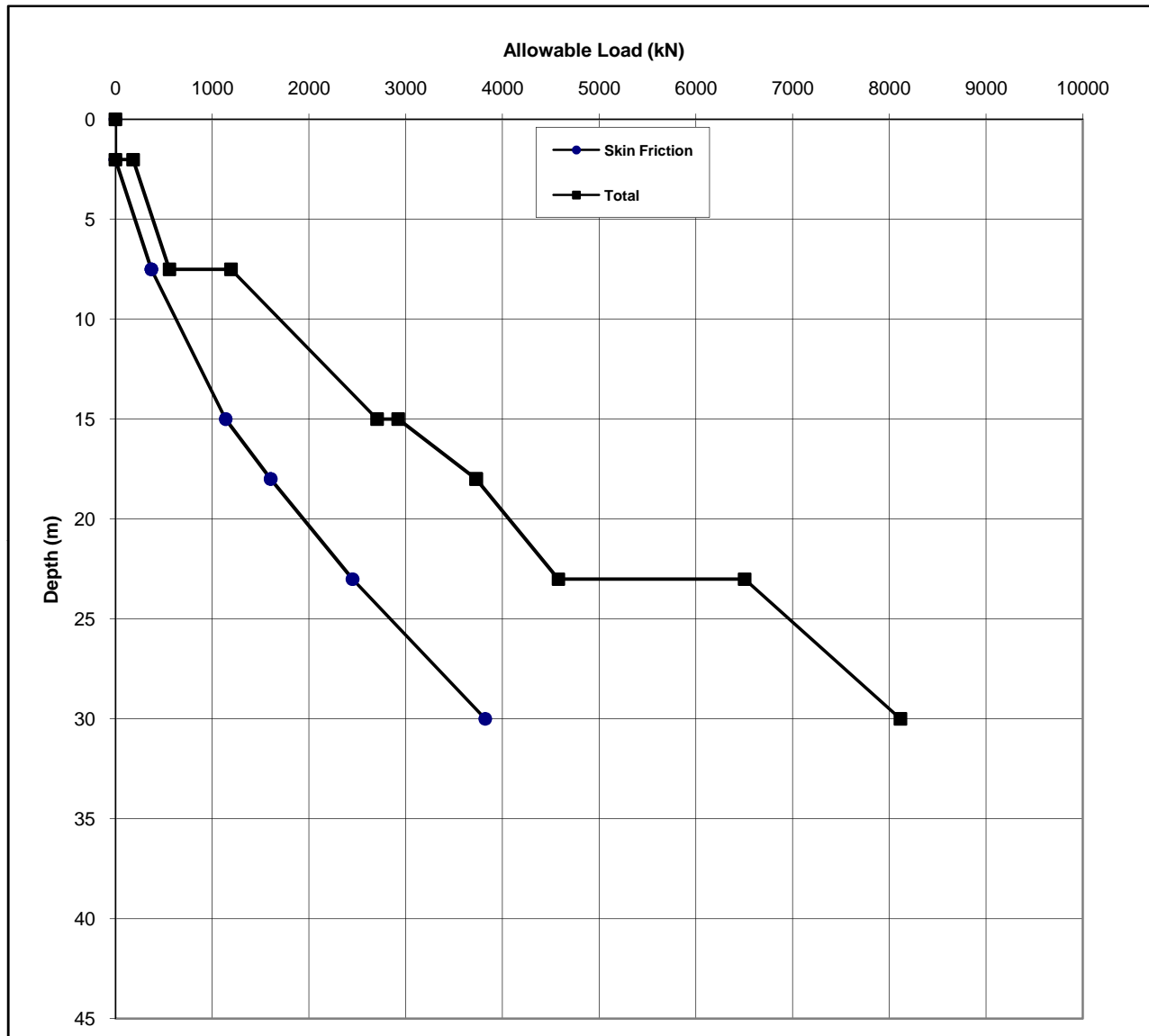
Settlement for Layer-II				
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for clay
	$m_v =$	m^2/kN	$\mu_g =$	
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	147.4	$C_{kd}/N =$	300 kN/m^2
	$p_o =$	58	$p =$	80.0 kPa
	Rigidity factor = 0.8		Depth Factor, $d_f =$	0.88
	δ_2 (mm) = 6.74			
Settlement for Layer-III				
δ (mm) =	$m_v \cdot H \cdot \Delta p \cdot \mu_g \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for clay
	$m_v = 0.00005$	m^2/kN	$\mu_g = 0.7$	
δ (mm) =	$[2.303 \cdot (H/C) \cdot \log_{10}((p_o + \Delta p)/p_o)] \cdot d_f \cdot \text{Rigidity Factor (0.8)}$			for sand
	$C = 1.5 \cdot (C_{kd}/p_o) =$	133.5	$C_{kd}/N =$	250 kN/m^2
	$p_o =$	95.5	$p =$	80.0 kPa
	Rigidity factor = 0.8		Depth Factor, $d_f =$	0.88
	δ_3 (mm) = 2.53			
Total settlement	=	23.88 mm		
Allowable Bearing capacity for 25mm settlement =				80.0 KPa



Allowable Pile Capacity Major Bridge at Chainage 63+570

Pile Type= Bored
Pile Dia (mm)= 1000

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5



Allowable Pile Capacity Major Bridge at Chainage 63+570

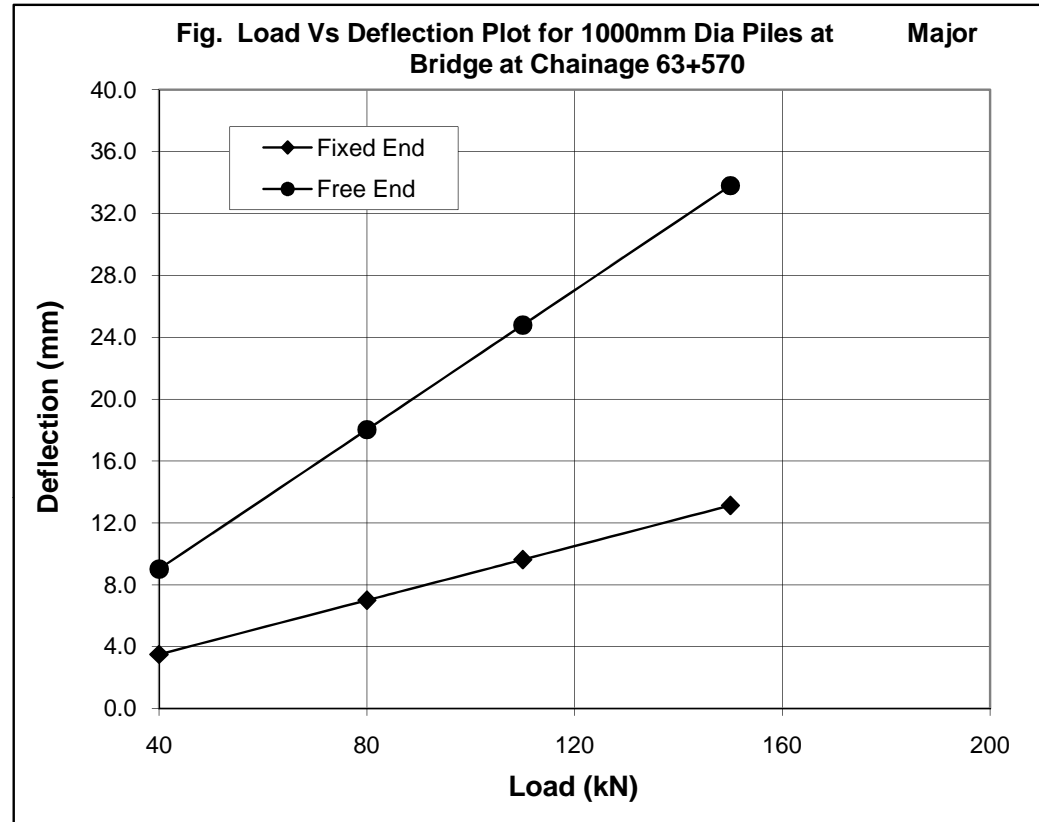
Pile Type= Bored
Pile Dia (mm)= 1200

Factor of Safety
End Bearing = 2.5
Skin Friction = 2.5

D= 100 cm
 $K_1 = 1.000 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$
 $K = (K_1 * 0.3) / (1.5B) = 0.2 \text{ kg/cm}^3$

$T = (EI/KB)^{0.25} = 507.37$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 1116.22$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 964.01$ cm
 $L_1 = 0$ cm
 $d = Q(L_1 + L_f)^3 / 12EI$ Fixed
 $d = Q(L_1 + L_f)^3 / 3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
40	3.50	9.01
80	7.00	18.02
110	9.62	24.78
150	13.12	33.80

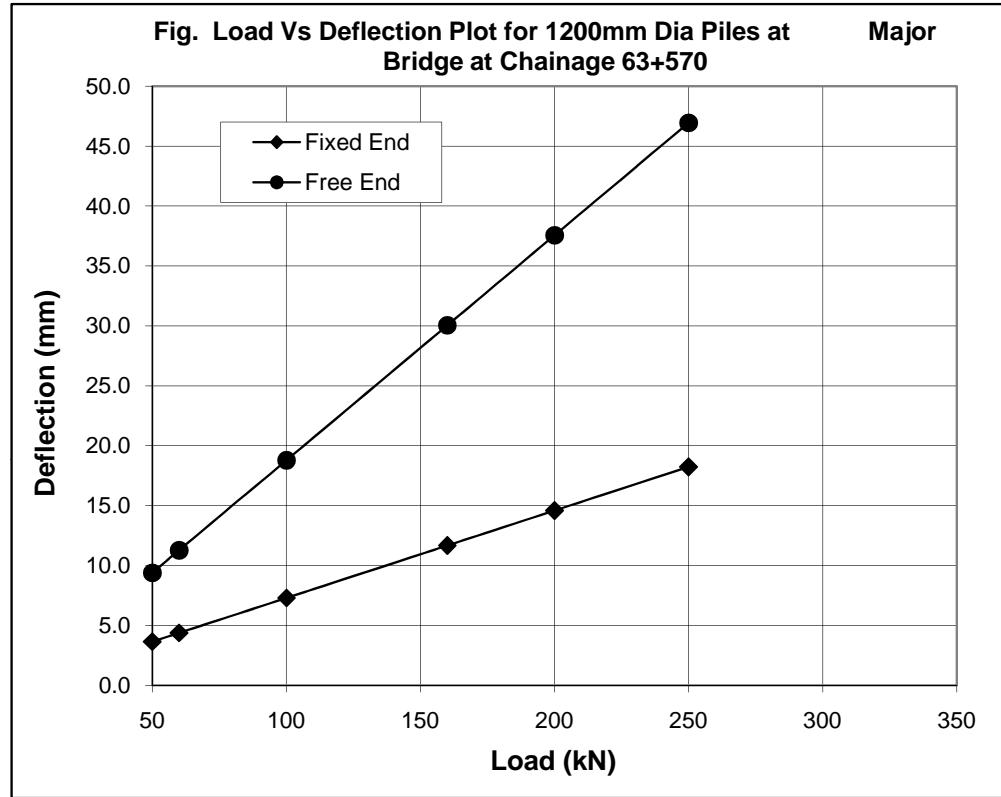


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 110kN (for fixed head condition)
 = 40 kN (for free head condition)

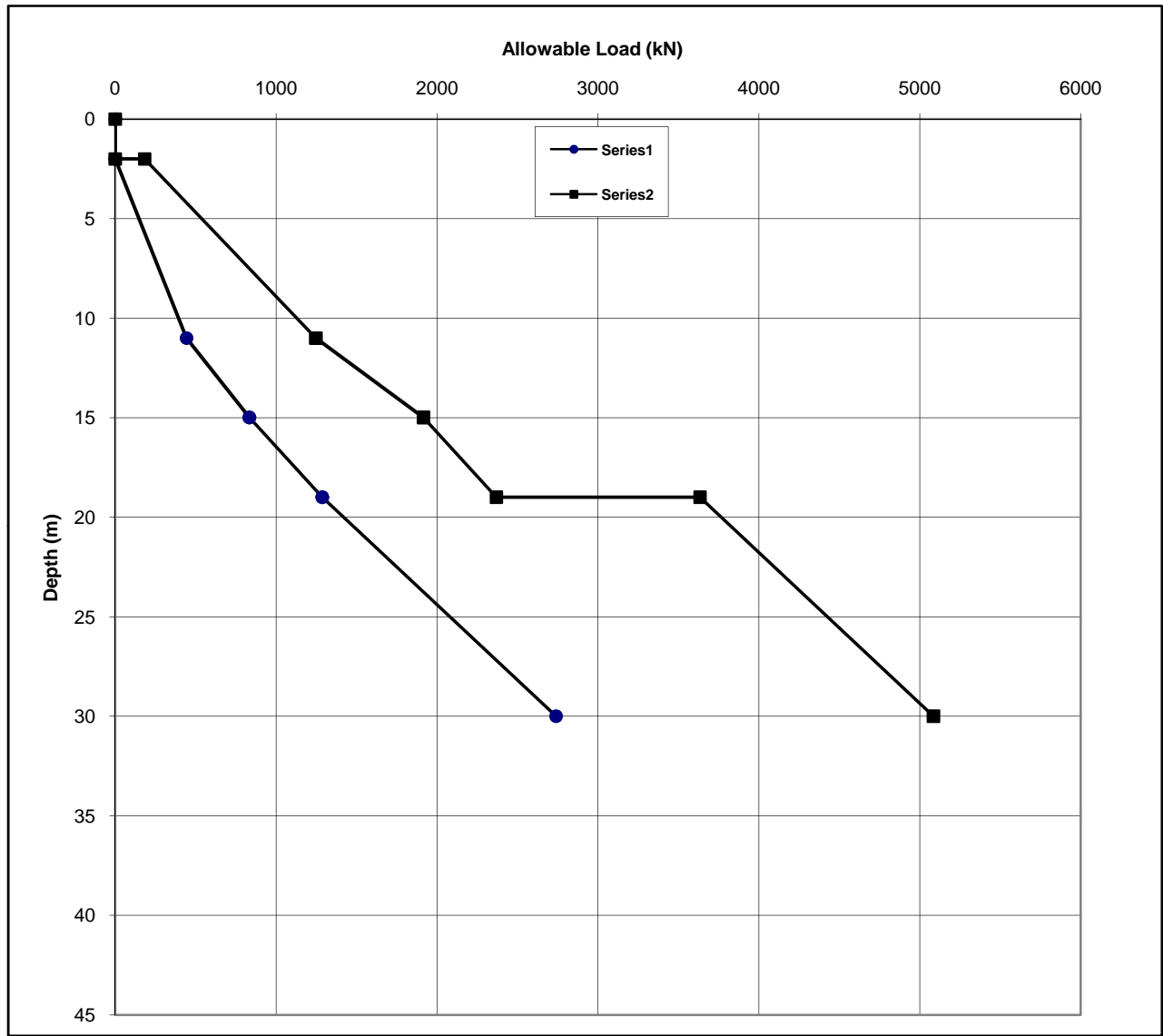
D= 120 cm
 K_1 1.000 kg/cm³
 E= 270000 kg/cm²
 I= 10178760.2 cm⁴
 EI= 2.74827E+12 kg-cm²
 $K=(K_1*0.3)/(1.5B)$ 0.16666667 kg/cm³

T= (EI/KB)^{0.25}
 608.85
 $L_f/T=$ 2.2 Fixed
 L_f (Fixed)= 1339.46 cm
 $L_f/T=$ 1.9 Free
 L_f (Free)= 1156.81 cm
 $L_1=$ 0 cm
 $d=$ $Q(L_1+L_f)^3/12EI$ Fixed
 $Q(L_1+L_f)^3/3EI$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	3.64	9.39
60	4.37	11.27
100	7.29	18.78
160	11.66	30.04
200	14.57	37.55
250	18.22	46.94



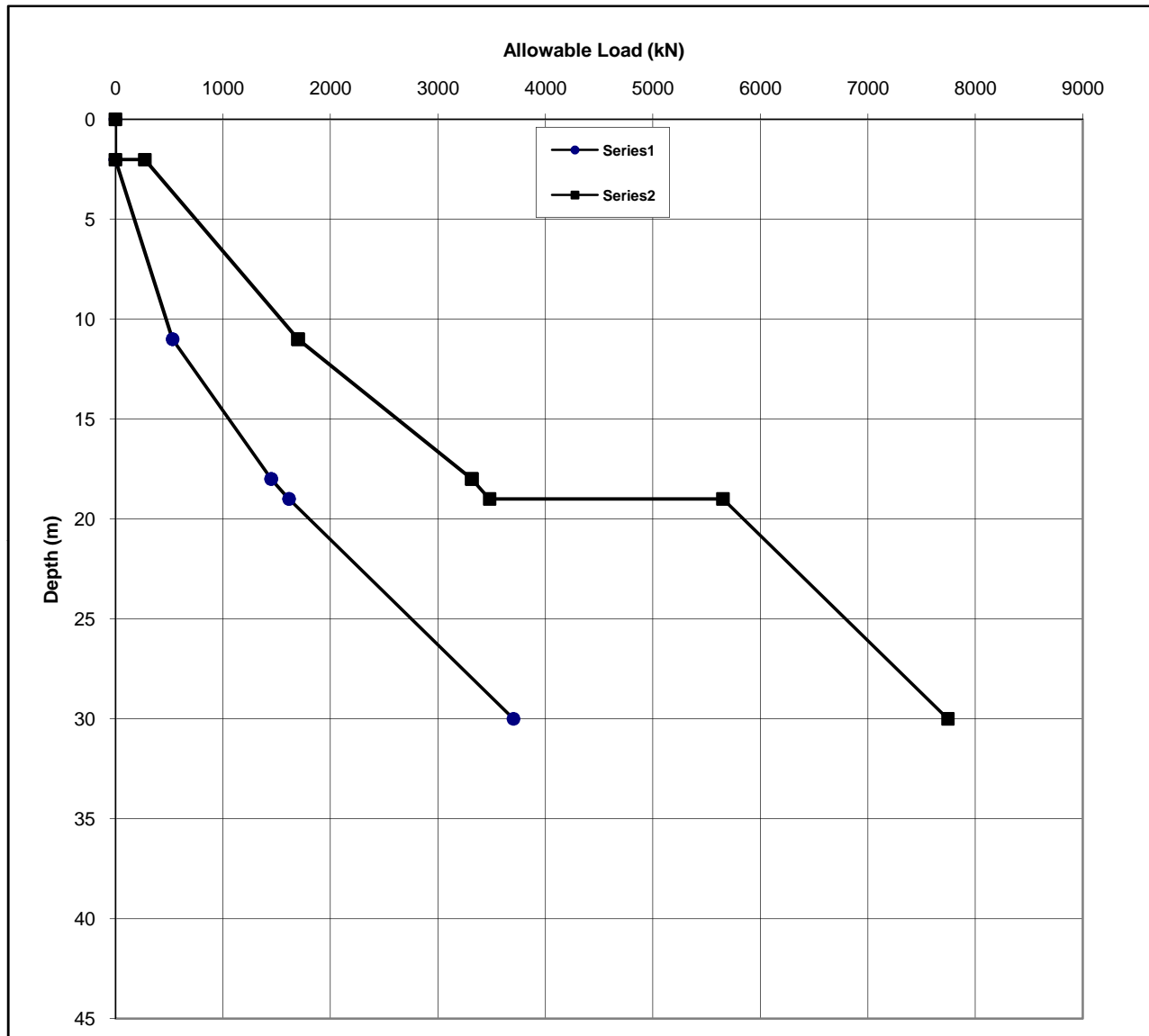
Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)
 = 160kN (for fixed head condition)
 = 60kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 64+270

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 64+270

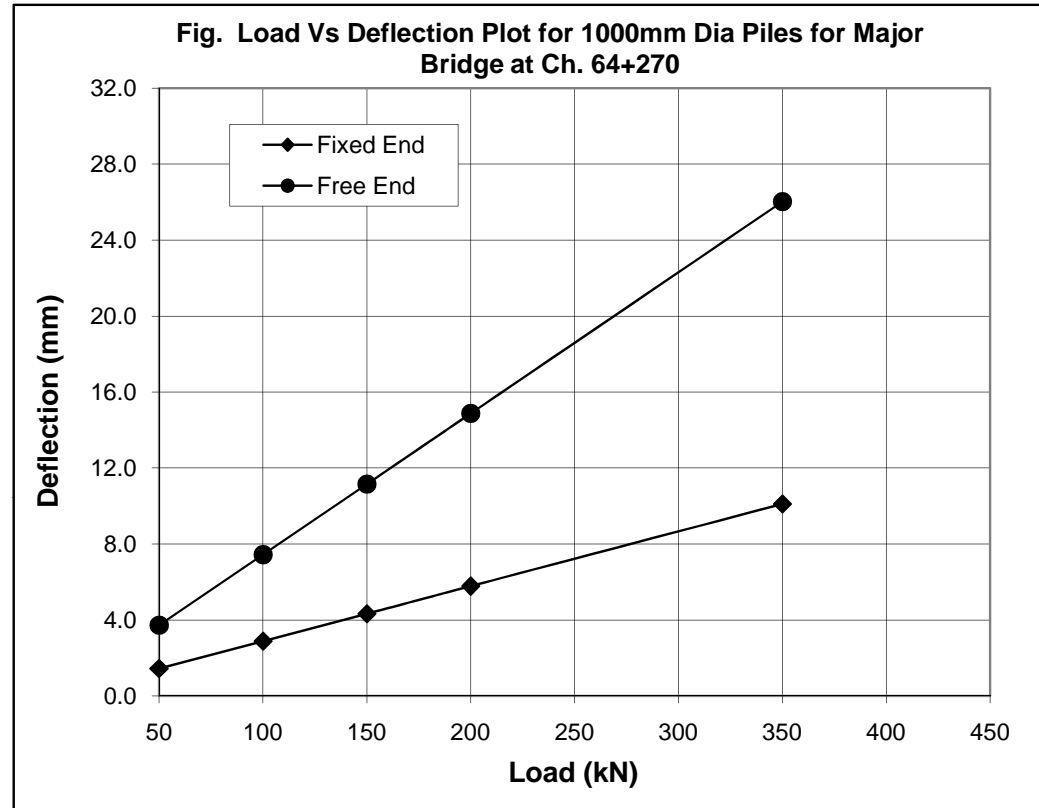
Pile Type= Bored
 Pile Dia (mm)= 1200

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

D= 100 cm
 $\eta_b = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 350.65$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 771.44$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 666.24$ cm
 $L_1 = 0$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03

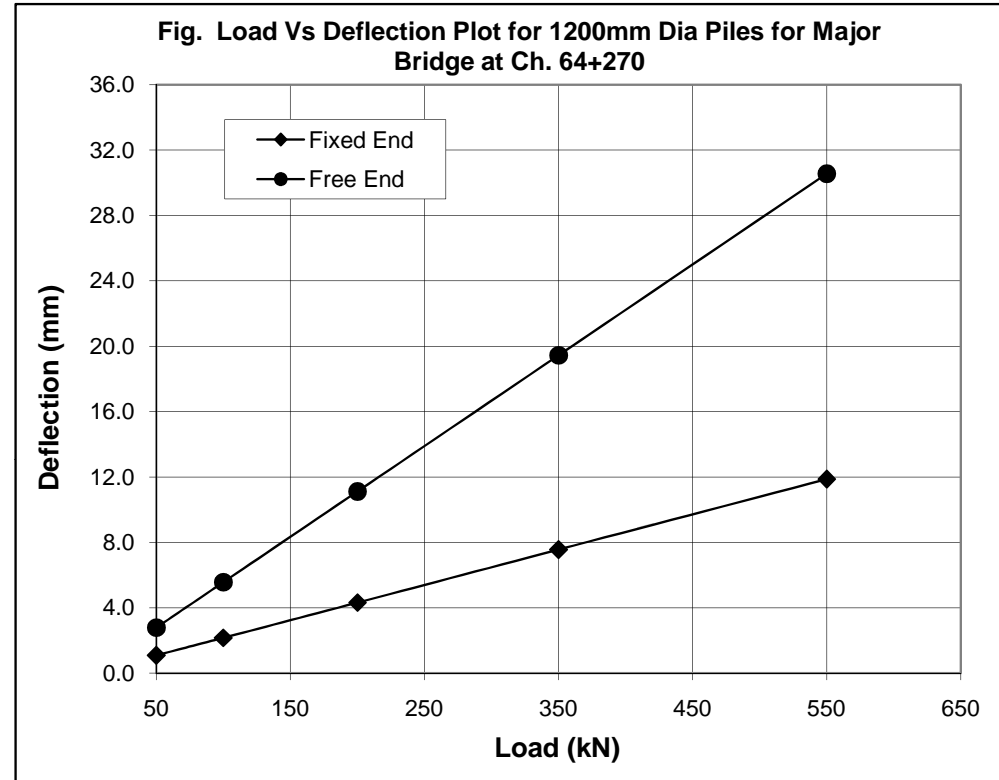


Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

D= 120 cm
 $\eta_h = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

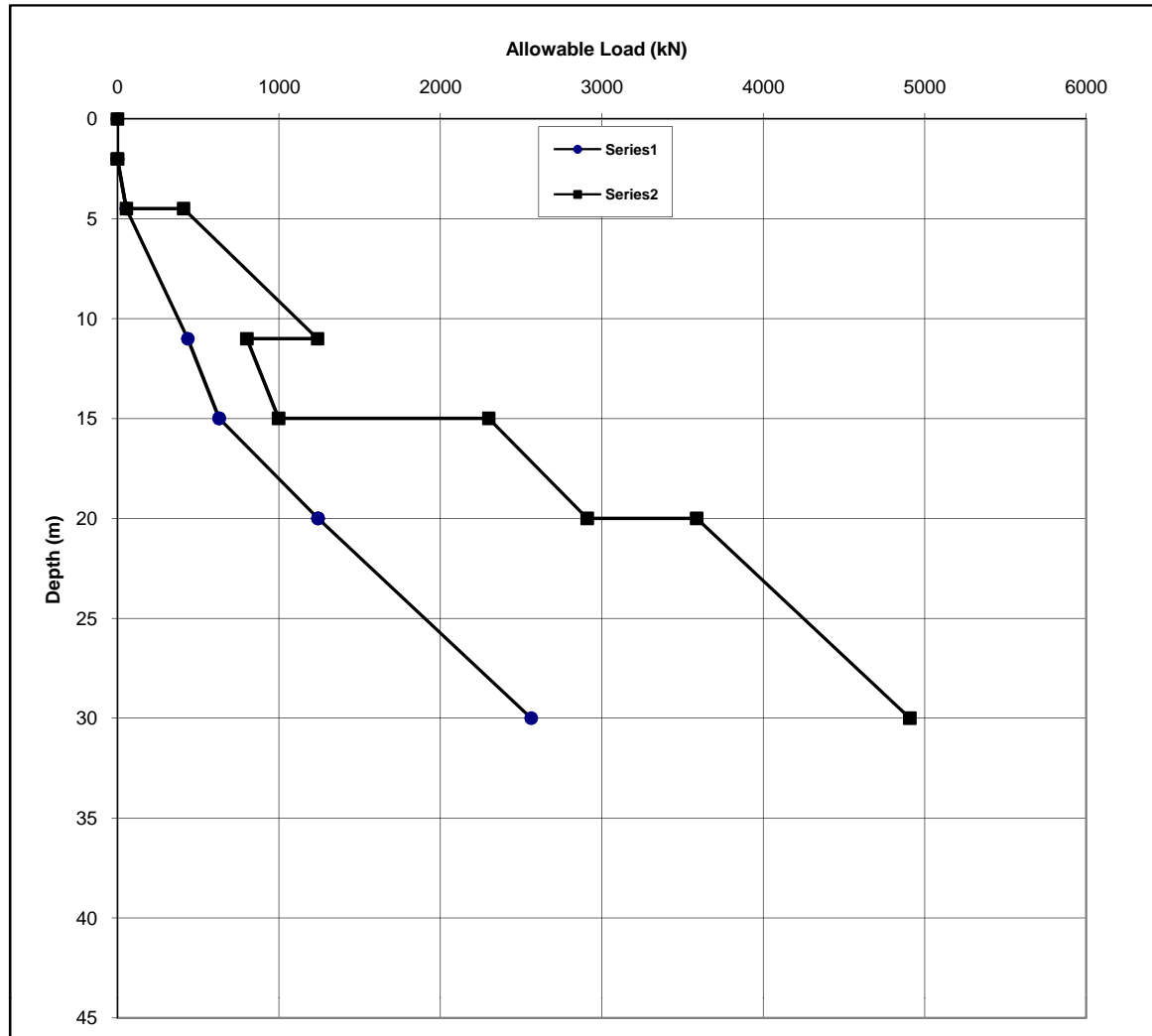
$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

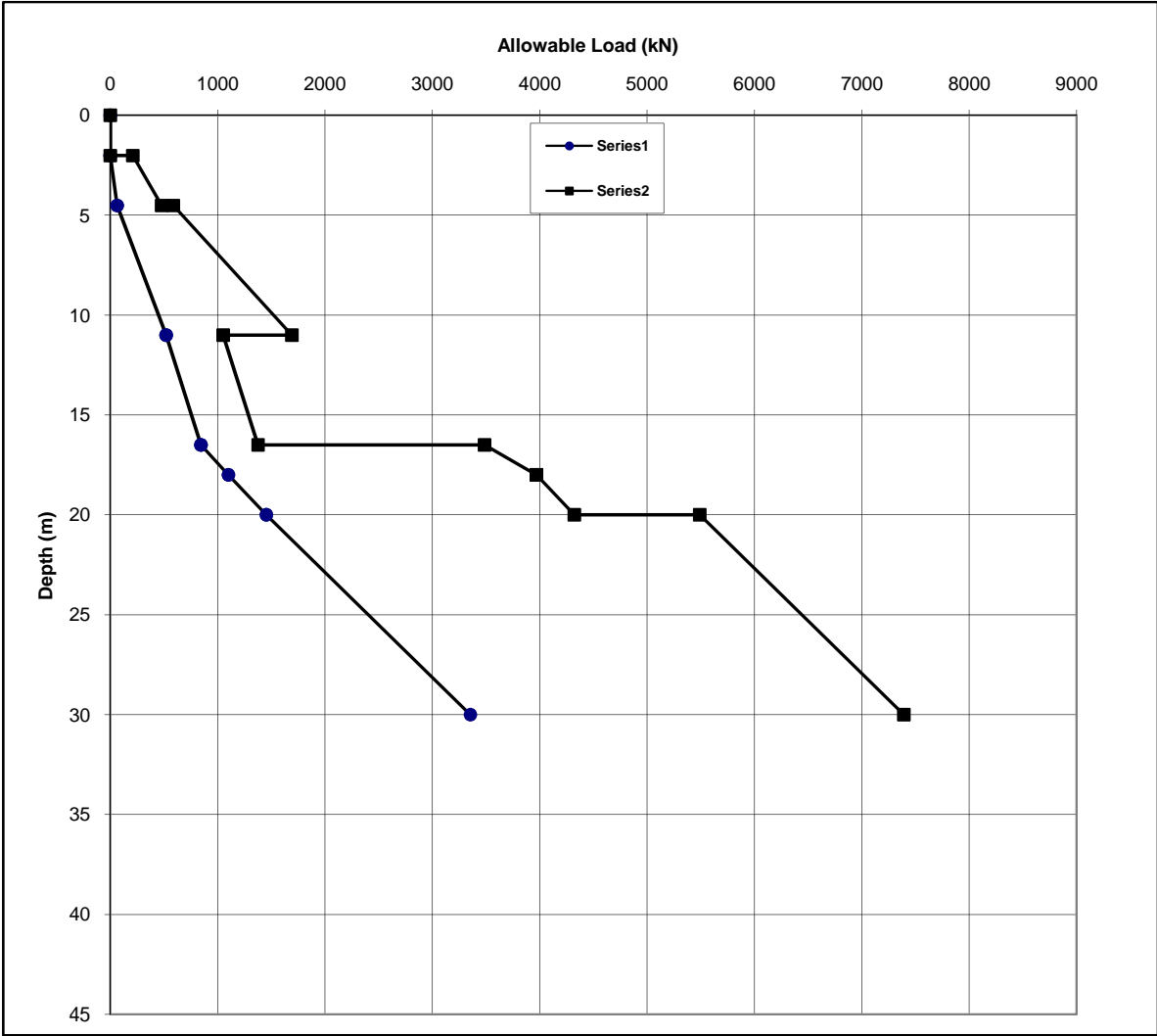
= 550kN (for fixed head condition)
 = 210kN (for free head condition)



Allowable Pile Capacity at Major Bridge Ch. 64+270

Pile Type= Bored
 Pile Dia (mm)= 1000

Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5



Allowable Pile Capacity at Major Bridge Ch. 64+270

Pile Type= Bored
 Pile Dia (mm)= 1200

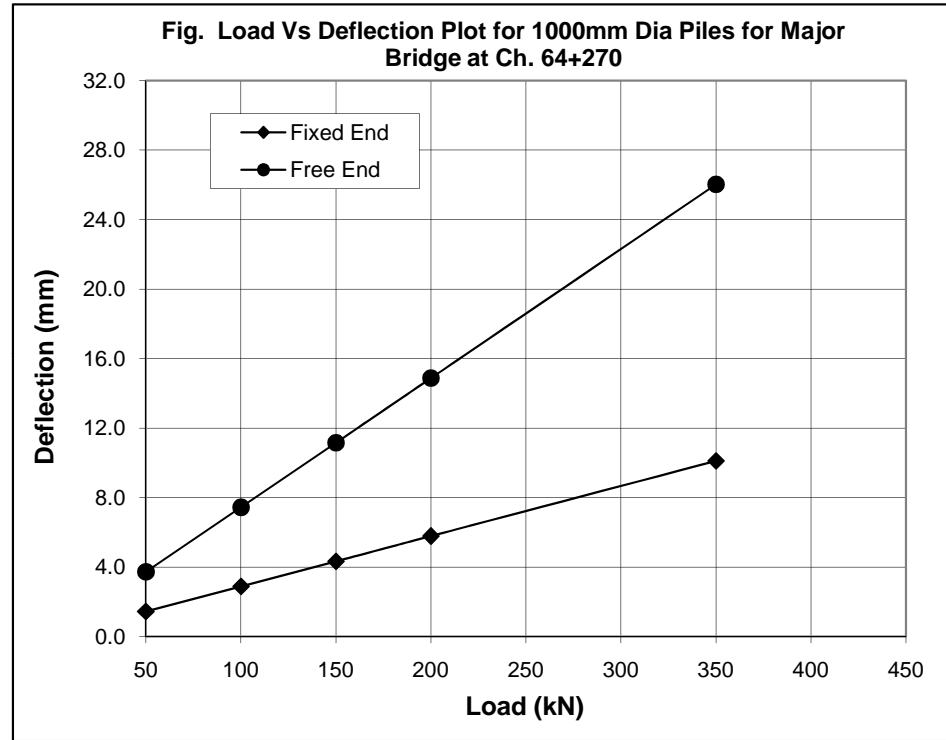
Factor of Safety
 End Bearing = 2.5
 Skin Friction = 2.5

LATERAL CAPACITY OF 1000 MM DIA BORED PILE FOR MAJOR BRIDGE AT CH 64+270 (IS: 2911 - PART-1/SEC-2-2010)

D= 100 cm
 $\eta_b = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 4908738.5 \text{ cm}^4$
 $EI = 1.32536E+12 \text{ kg-cm}^2$

$T = (EI/\eta_b h)^{0.2} = 350.65$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 771.44 \text{ cm}$
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 666.24 \text{ cm}$
 $L_1 = 0 \text{ cm}$
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.44	3.72
100	2.89	7.44
150	4.33	11.16
200	5.77	14.88
350	10.10	26.03



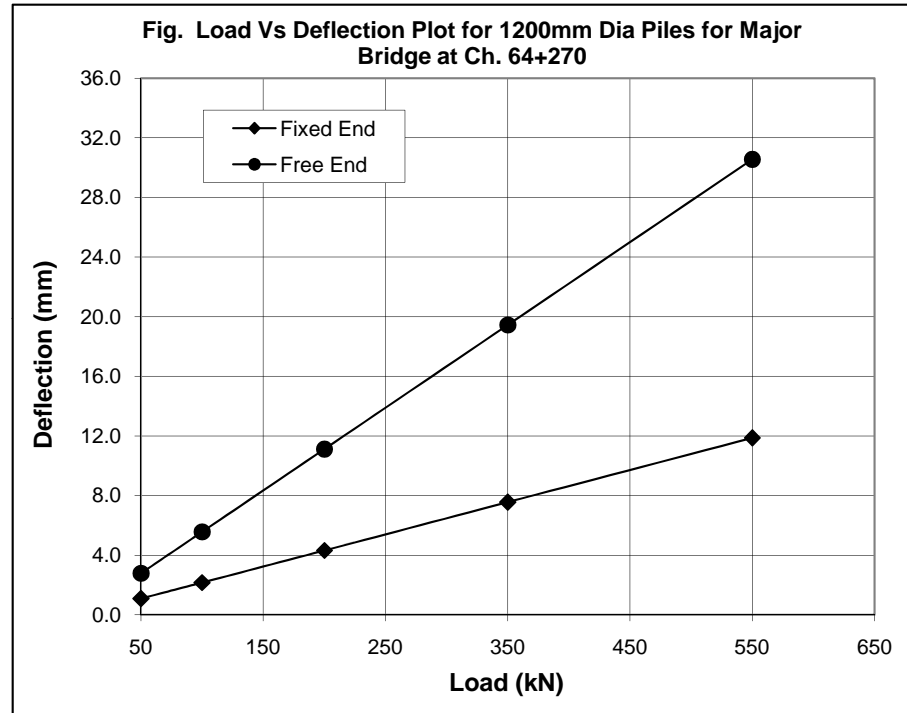
Hence lateral capacity (load corresponding to 1% of pile diameter=10mm deflection)
 = 350kN (for fixed head condition)
 = 130 kN (for free head condition)

LATERAL CAPACITY OF 1200 MM DIA BORED PILE FOR MAJOR BRIDGE CH. 64+270 (IS: 2911 - PART-1/SEC-2-2010)

D= 120 cm
 $\eta_b = 0.250 \text{ kg/cm}^3$
 $E = 270000 \text{ kg/cm}^2$
 $I = 10178760.2 \text{ cm}^4$
 $EI = 2.74827E+12 \text{ kg-cm}^2$

$T = (EI/\eta h)^{0.2} = 405.72$
 $L_f/T = 2.2$ Fixed
 $L_f(\text{Fixed}) = 892.58$ cm
 $L_f/T = 1.9$ Free
 $L_f(\text{Free}) = 770.86$ cm
 $L_1 = 0$ cm
 $d = \frac{Q(L_1+L_f)^3}{12EI}$ Fixed
 $d = \frac{Q(L_1+L_f)^3}{3EI}$ Free

Q (kN)	d (mm) - Fixed	d (mm) - Free
50	1.08	2.78
100	2.16	5.56
200	4.31	11.11
350	7.55	19.45
550	11.86	30.56



Hence lateral capacity (load corresponding to 1% of pile diameter=12mm deflection)

= 550kN (for fixed head condition)
 = 210kN (for free head condition)